

# Final

# Water Quality Management Plan

For:

**18313 Valley Blvd.**

APN(S): 0252-161-09 & 10

Prepared for:

San Bernardino County  
385 N. Arrowhead Ave., 3rd Floor  
San Bernardino, CA 92415  
ATTN: Kenneth Hylin  
PHONE: (909) 387-5000

Prepared by:



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(909) 885-3806

Approval Date: \_\_\_\_\_

## Project Owner's Certification

This Water Quality Management Plan (WQMP) has been prepared for San Bernardino County by Bonadiman & Associates, Inc. The WQMP is intended to comply with the requirements of the San Bernardino County and the NPDES Area wide Stormwater Program requiring the preparation of a WQMP. The undersigned, while it owns the subject property, is responsible for the implementation of the provisions of this plan and will ensure that this plan is amended as appropriate to reflect up-to-date conditions on the site consistent with San Bernardino County's Municipal Storm Water Management Program and the intent of the NPDES Permit for San Bernardino County and the incorporated cities of San Bernardino County within the Santa Ana Region. Once the undersigned transfers its interest in the property, its successors in interest and the city/county shall be notified of the transfer. The new owner will be informed of its responsibility under this WQMP. A copy of the approved WQMP shall be available on the subject site in perpetuity.



"I certify under a penalty of law that the provisions (implementation, operation, maintenance, and funding) of the WQMP have been accepted and that the plan will be transferred to future successors."

Project Data			
Permit/Application Number(s):	WQMP-2024-00002	Grading Permit Number(s):	GRAD-2024-00007
Tract/Parcel Map Number(s):	Marygold Acres	Building Permit Number(s):	NEWNR-2024-00002, 4, 5, 6, & 7
CUP, SUP, and/or APN (Specify Lot Numbers if Portions of Tract):			APNs: 0252-161-09 & 10
Owner's Signature			
<b>Owner:</b> San Bernardino County			
Title	Director, San Bernardino County - Project & Facilities Management Department		
Representative	Don Day		
Address	385 N. Arrowhead Ave., 3 <sup>rd</sup> Floo, San Bernardino, CA 92415-0184		
Email	donald.day@pfm.sbcounty.gov		
Telephone #	(909) 387-5000		
Signature			Date

### Preparer's Certification

Project Data			
Permit/Application Number(s):	WQMP-2024-00002	Grading Permit Number(s):	GRAD-2024-00007
Tract/Parcel Map Number(s):	Marygold Acres	Building Permit Number(s):	NEWNR-2024-00002, 4, 5, 6, & 7
CUP, SUP, and/or APN (Specify Lot Numbers if Portions of Tract):			APN: 0254-021-37 & 41

“The selection, sizing and design of stormwater treatment and other stormwater quality and quantity control measures in this plan were prepared under my oversight and meet the requirements of Regional Water Quality Control Board Order No. R8-2010-0036.”

<b>Engineer:</b> James T. Stanton		PE Stamp Below  
Title	Vice President of Engineering	
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Date	09-26-24	

# Table of Contents

<b>Section 1 Discretionary Permits</b> .....	<b>1-1</b>
<b>Section 2 Project Description</b> .....	<b>2-1</b>
2.1 Project Information.....	2-1
2.2 Property Ownership / Management .....	2-2
2.3 Potential Stormwater Pollutants .....	2-3
2.4 Water Quality Credits .....	2-4
<b>Section 3 Site and Watershed Description</b> .....	<b>3-1</b>
<b>Section 4 Best Management Practices</b> .....	<b>4-1</b>
4.1 Source Control BMP .....	4-1
4.1.1 Pollution Prevention.....	4-1
4.1.2 Preventative LID Site Design Practices .....	4-6
4.2 Project Performance Criteria .....	4-7
4.3 Project Conformance Analysis.....	4-12
4.3.1 Site Design Hydrologic Source Control BMP .....	4-14
4.3.2 Infiltration BMP .....	4-16
4.3.3 Harvest and Use BMP .....	4-18
4.3.4 Biotreatment BMP .....	4-19
4.3.5 Conformance Summary.....	4-23
4.3.6 Hydromodification Control BMP.....	4-24
4.4 Alternative Compliance Plan (if applicable) .....	4-25
<b>Section 5 Inspection &amp; Maintenance Responsibility Post Construction BMPs</b> .....	<b>5-1</b>
<b>Section 6 WQMP Attachments</b> .....	<b>6-1</b>
6.1 Site Plan and Drainage Plan .....	6-1
6.2 Electronic Data Submittal.....	6-2
6.3 Post Construction .....	6-3
6.4 Other Supporting Documentation .....	6-4

# Forms

Form 1-1 Project Information .....	1-1
Form 2.1-1 Description of Proposed Project .....	2-1
Form 2.2-1 Property Ownership/Management .....	2-2
Form 2.3-1 Pollutants of Concern .....	2-3
Form 2.4-1 Water Quality Credits .....	2-4
Form 3-1 Site Location and Hydrologic Features .....	3-1
Form 3-2 Hydrologic Characteristics .....	3-2
Form 3-3 Watershed Description.....	3-3
Form 4.1-1 Non-Structural Source Control BMP .....	4-2
Form 4.1-2 Structural Source Control BMP .....	4-4
Form 4.1-3 Site Design Practices Checklist.....	4-6
Form 4.2-1 LID BMP Performance Criteria for Design Capture Volume .....	4-7
Form 4.2-2 Summary of HCOC Assessment.....	4-8
Form 4.2-3 HCOC Assessment for Runoff Volume .....	4-9
Form 4.2-4 HCOC Assessment for Time of Concentration .....	4-10
Form 4.2-5 HCOC Assessment for Peak Runoff.....	4-11
Form 4.3-1 Infiltration BMP Feasibility .....	4-13
Form 4.3-2 Site Design Hydrologic Source Control BMP .....	4-14
Form 4.3-3 Infiltration LID BMP.....	4-17
Form 4.3-4 Harvest and Use BMP .....	4-18
Form 4.3-5 Selection and Evaluation of Biotreatment BMP .....	4-19
Form 4.3-6 Volume Based Biotreatment – Bioretention and Planter Boxes w/Underdrains	4-20
Form 4.3-7 Volume Based Biotreatment- Constructed Wetlands and Extended Detention	4-21
Form 4.3-8 Flow Based Biotreatment .....	4-22
Form 4.3-9 Conformance Summary and Alternative Compliance Volume Estimate .....	4-23
Form 4.3-10 Hydromodification Control BMP .....	4-24
Form 5-1 BMP Inspection and Maintenance .....	5-1

# Section 1 Discretionary Permit(s)

Form 1-1 Project Information					
Project Name		18313 Valley Blvd.			
Project Owner Contact Name:		Don Day			
Mailing Address:	385 N. Arrowhead Ave., 3rd Floor San Bernardino, CA 92415-0184	E-mail Address:	donald.day@pfm.sbcounty.gov	Telephone:	(909) 387-5000
Permit/Application Number(s):		WQMP-2024-00002	Tract/Parcel Map Number(s):	Marygold Acres	
Additional Information/ Comments:					
Description of Project:		<p>The proposed project consists of the redevelopment of an existing park into a animal care center. This project will include new buildings, landscaping, asphalt/concrete and gravel. Site runoff will be directed to landscape where possible. Site runoff will be drain to catchbasins with filters then to the WQMP infiltration BMP. Overflow for the WQMP basin will be directed to an underground detention basin with an outlet to the southeast corner of the site.</p> <p>DA-1: Roof=70,558 SF Landscaping=54,829 SF Concrete/AC=119,611 SF Gravel=991 SF                      DA-2: Roof=0 SF Landscaping=17,711 SF Concret/AC=4645 SF Gravel=100 SF</p>			
Provide summary of Conceptual WQMP conditions (if previously submitted and approved). Attach complete copy.					

## Section 2 Project Description

### 2.1 Project Information

This section of the WQMP should provide the information listed below. The information provided for Conceptual/ Preliminary WQMP should give sufficient detail to identify the major proposed site design and LID BMPs and other anticipated water quality features that impact site planning. Final Project WQMP must specifically identify all BMP incorporated into the final site design and provide other detailed information as described herein.

The purpose of this information is to help determine the applicable development category, pollutants of concern, watershed description, and long term maintenance responsibilities for the project, and any applicable water quality credits. This information will be used in conjunction with the information in Section 3, Site Description, to establish the performance criteria and to select the LID BMP or other BMP for the project or other alternative programs that the project will participate in, which are described in Section 4.

<b>Form 2.1-1 Description of Proposed Project</b>				
<b>1</b> Development Category (Select all that apply):				
<input checked="" type="checkbox"/> Significant re-development involving the addition or replacement of 5,000 ft <sup>2</sup> or more of impervious surface on an already developed site	<input type="checkbox"/> New development involving the creation of 10,000 ft <sup>2</sup> or more of impervious surface collectively over entire site	<input type="checkbox"/> Automotive repair shops with standard industrial classification (SIC) codes 5013, 5014, 5541, 7532- 7534, 7536-7539	<input type="checkbox"/> Restaurants (with SIC code 5812) where the land area of development is 5,000 ft <sup>2</sup> or more	
<input type="checkbox"/> Hillside developments of 5,000 ft <sup>2</sup> or more which are located on areas with known erosive soil conditions or where the natural slope is 25 percent or more	<input type="checkbox"/> Developments of 2,500 ft <sup>2</sup> of impervious surface or more adjacent to (within 200 ft) or discharging directly into environmentally sensitive areas or waterbodies listed on the CWA Section 303(d) list of impaired waters.	<input checked="" type="checkbox"/> Parking lots of 5,000 ft <sup>2</sup> or more exposed to storm water	<input type="checkbox"/> Retail gasoline outlets that are either 5,000 ft <sup>2</sup> or more, or have a projected average daily traffic of 100 or more vehicles per day	
<input type="checkbox"/> Non-Priority / Non-Category Project <i>May require source control LID BMPs and other LIP requirements. Please consult with local jurisdiction on specific requirements.</i>				
<b>2</b> Project Area (ft <sup>2</sup> ):	264,445	<b>3</b> Number of Dwelling Units:	0	<b>4</b> SIC Code: 0752
<b>5</b> Is Project going to be phased? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> <i>If yes, ensure that the WQMP evaluates each phase as a distinct DA, requiring LID BMPs to address runoff at time of completion.</i>				
<b>6</b> Does Project include roads? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> <i>If yes, ensure that applicable requirements for transportation projects are addressed (see Appendix A of TGD for WQMP)</i>				

## 2.2 Property Ownership/Management

Describe the ownership/management of all portions of the project and site. State whether any infrastructure will transfer to public agencies (City, County, Caltrans, etc.) after project completion. State if a homeowners or property owners association will be formed and be responsible for the long-term maintenance of project stormwater facilities. Describe any lot-level stormwater features that will be the responsibility of individual property owners.

### Form 2.2-1 Property Ownership/Management

Describe property ownership/management responsible for long-term maintenance of WQMP stormwater facilities:

San Bernardino County will be responsible for long-term maintenance of WQMP stormwater facilities.

Director, San Bernardino County - Project & Facilities Management Department

Sr. Project Manager

Contact: Don Day

Email: donald.day@pfm.sbcounty.gov



## 2.3 Potential Stormwater Pollutants

Determine and describe expected stormwater pollutants of concern based on land uses and site activities (refer to Table 3-3 in the TGD for WQMP).

<b>Form 2.3-1 Pollutants of Concern</b>			
Pollutant	Please check: E=Expected, N=Not Expected		Additional Information and Comments
	E <input checked="" type="checkbox"/>	N <input type="checkbox"/>	
Pathogens (Bacterial / Virus)	E <input checked="" type="checkbox"/>	N <input type="checkbox"/>	Expected per Table 3-3 in the TGD for WQMP. Per section 3.3 of the TGD for WQMP, potential sources include animal waste.
Phosphorous	E <input checked="" type="checkbox"/>	N <input type="checkbox"/>	Expected per Table 3-3 in the TGD for WQMP. Per section 3.3 of the TGD for WQMP, potential sources include fertilizers and eroded soils.
Nitrogen	E <input checked="" type="checkbox"/>	N <input type="checkbox"/>	Expected per Table 3-3 in the TGD for WQMP. Per section 3.3 of the TGD for WQMP, potential sources include fertilizers and eroded soils.
Sediment	E <input checked="" type="checkbox"/>	N <input type="checkbox"/>	Expected per Table 3-3 in the TGD for WQMP. Per section 3.3 of the TGD for WQMP, potential sources include eroded soils.
Metals	E <input checked="" type="checkbox"/>	N <input type="checkbox"/>	Expected per Table 3-3 in the TGD for WQMP. Per section 3.3 of the TGD for WQMP, potential sources include brake pad and tire tread wear associated with driving.
Oil and Grease	E <input checked="" type="checkbox"/>	N <input type="checkbox"/>	Expected per Table 3-3 in the TGD for WQMP. Per section 3.3 of the TGD for WQMP, potential sources include petroleum hydrocarbon products, motor products from leaking vehicles, esters, oils, fats, waxes, and high molecular-weight fatty acids.
Trash/Debris	E <input checked="" type="checkbox"/>	N <input type="checkbox"/>	Expected per Table 3-3 in the TGD for WQMP. Per section 3.3 of the TGD for WQMP, potential sources include paper, plastic, polystyrene packing foam, and aluminum materials.
Pesticides / Herbicides	E <input checked="" type="checkbox"/>	N <input type="checkbox"/>	Expected per Table 3-3 in the TGD for WQMP. Per section 3.3 of the TGD for WQMP, potential sources include fertilizers and pest sprays.
Organic Compounds	E <input checked="" type="checkbox"/>	N <input type="checkbox"/>	Expected per Table 3-3 in the TGD for WQMP. Per section 3.3 of the TGD for WQMP, potential sources include solvents and cleaning compounds.
Other:	E <input type="checkbox"/>	N <input type="checkbox"/>	
Other:	E <input type="checkbox"/>	N <input type="checkbox"/>	
Other:	E <input type="checkbox"/>	N <input type="checkbox"/>	
Other:	E <input type="checkbox"/>	N <input type="checkbox"/>	
Other:	E <input type="checkbox"/>	N <input type="checkbox"/>	
Other:	E <input type="checkbox"/>	N <input type="checkbox"/>	

## 2.4 Water Quality Credits

A water quality credit program is applicable for certain types of development projects if it is not feasible to meet the requirements for on-site LID. Proponents for eligible projects, as described below, can apply for water quality credits that would reduce project obligations for selecting and sizing other treatment BMP or participating in other alternative compliance programs. Refer to Section 6.2 in the TGD for WQMP to determine if water quality credits are applicable for the project.

<b>Form 2.4-1 Water Quality Credits</b>			
<b>1</b> Project Types that Qualify for Water Quality Credits: <i>Select all that apply</i>			
<input type="checkbox"/> Redevelopment projects that reduce the overall impervious footprint of the project site. [Credit = % <b>impervious</b> reduced]	Higher density development projects <input type="checkbox"/> Vertical density [20%] <input type="checkbox"/> 7 units/ acre [5%]	<input type="checkbox"/> Mixed use development, (combination of residential, commercial, industrial, office, institutional, or other land uses which incorporate design principles that demonstrate environmental benefits not realized through single use projects) [20%]	<input type="checkbox"/> Brownfield redevelopment (redevelop real property complicated by presence or potential of hazardous contaminants) [25%]
<input type="checkbox"/> Redevelopment projects in established historic district, historic preservation area, or similar significant core city center areas [10%]	<input type="checkbox"/> Transit-oriented developments (mixed use residential or commercial area designed to maximize access to public transportation) [20%]	<input type="checkbox"/> In-fill projects (conversion of empty lots & other underused spaces < 5 acres, substantially surrounded by urban land uses, into more beneficially used spaces, such as residential or commercial areas) [10%]	<input type="checkbox"/> Live-Work developments (variety of developments designed to support residential and vocational needs) [20%]
<b>2</b> Total Credit % 0 <i>(Total all credit percentages up to a maximum allowable credit of 50 percent)</i>			
Description of Water Quality Credit Eligibility (if applicable)	N/A		

## Section 3 Site and Watershed Description

Describe the project site conditions that will facilitate the selection of BMP through an analysis of the physical conditions and limitations of the site and its receiving waters. Identify distinct drainage areas (DA) that collect flow from a portion of the site and describe how runoff from each DA (and sub-watershed DMAs) is conveyed to the site outlet(s). Refer to Section 3.2 in the TGD for WQMP. The form below is provided as an example. Then complete Forms 3.2 and 3.3 for each DA on the project site. ***If the project has more than one drainage area for stormwater management, then complete additional versions of these forms for each DA / outlet.***

RE RE RE RE

Form 3-1 Site Location and Hydrologic Features			
Site coordinates <i>take GPS measurement at approximate center of site</i>	Latitude 34.0694°	Longitude -117.4050°	Thomas Bros Map page Page 605, Grid D6
<p><sup>1</sup> San Bernardino County climatic region: <input checked="" type="checkbox"/> Valley <input type="checkbox"/> Mountain</p>			
<p><sup>2</sup> Does the site have more than one drainage area (DA): Yes <input checked="" type="checkbox"/> No <input type="checkbox"/> <i>If no, proceed to Form 3-2. If yes, then use this form to show a conceptual schematic describing DMAs and hydrologic feature connecting DMAs to the site outlet(s). An example is provided below that can be modified for proposed project or a drawing clearly showing DMA and flow routing may be attached</i></p>			
<pre> graph LR     DA1[DA-1] --&gt; BMP1[BMP-1]     BMP1 --&gt; DA2[DA-2]     DA2 --&gt; BMP2[BMP-2]     BMP2 --&gt; Caltrans[Caltrans Channel]             </pre>			
Conveyance	Briefly describe on-site drainage features to convey runoff that is not retained within a DMA		
DA-1 to BMP-1	Area drains to an infiltration BMP system and the overflow normalizes before discharging south to BMP-2, per exhibit.		
DA-2 to BMP-2	Area drains to a dual purpose infiltration and retarding basin which will normalize flows back to sheet flow before they discharge south to a storm drain channel located in the CalTrans Right-of-Way.		

<b>Form 3-2 Existing Hydrologic Characteristics for Drainage Area 1&amp;2</b>				
For Drainage Area's sub-watershed, provide the following characteristics	DA 1	DA 2		
<b>1</b> DA drainage area (ft <sup>2</sup> )	245,989	18,456		
<b>2</b> Existing site impervious area (ft <sup>2</sup> )	0	0		
<b>3</b> Antecedent moisture condition <i>For desert areas, use <a href="http://www.sbcounty.gov/dpw/floodcontrol/pdf/20100412_map.pdf">http://www.sbcounty.gov/dpw/floodcontrol/pdf/20100412_map.pdf</a></i>	II	II		
<b>4</b> Hydrologic soil group <i>Refer to Watershed Mapping Tool – <a href="http://sbcounty.permitrack.com/WAP">http://sbcounty.permitrack.com/WAP</a></i>	A	A		
<b>5</b> Longest flowpath length (ft)	841	73		
<b>6</b> Longest flowpath slope (ft/ft)	0.013	0.027		
<b>7</b> Current land cover type(s) <i>Select from Fig C-3 of Hydrology Manual</i>	Perennial Grass	Perennial Grass		
<b>8</b> Pre-developed pervious area condition: <i>Based on the extent of wet season vegetated cover good &gt;75%; Fair 50-75%; Poor &lt;50% Attach photos of site to support rating</i>	Fair	Fair		

<b>Form 3-3 Watershed Description for Drainage Area 1</b>	
<p><b>Receiving waters</b>                      Refer to Watershed Mapping Tool - <a href="http://sbcounty.permitrack.com/WAP">http://sbcounty.permitrack.com/WAP</a>                      See "Drainage Facilities" link at this website</p>	<p>San Sevaine Channel                      Santa Ana River, Reach 3                      Santa Ana River, Reach 2                      Santa Ana River, Reach 1                      Pacific Ocean</p>
<p><b>Applicable TMDLs</b>                      Refer to Local Implementation Plan</p>	<p>Santa Ana River, Reach 3 - Indicator Bacteria</p>
<p><b>303(d) listed impairments</b>                      Refer to Local Implementation Plan and Watershed Mapping Tool - <a href="http://sbcounty.permitrack.com/WAP">http://sbcounty.permitrack.com/WAP</a> and State Water Resources Control Board website - <a href="http://www.waterboards.ca.gov/santaana/water_issues/programs/tmdl/index.shtml">http://www.waterboards.ca.gov/santaana/water_issues/programs/tmdl/index.shtml</a></p>	<p>Santa Ana River, Reach 3 - Copper, Lead and Indicator Bacteria</p>
<p><b>Environmentally Sensitive Areas (ESA)</b>                      Refer to Watershed Mapping Tool - <a href="http://sbcounty.permitrack.com/WAP">http://sbcounty.permitrack.com/WAP</a></p>	<p>Areas within 200': None</p>
<p><b>Unlined Downstream Water Bodies</b>                      Refer to Watershed Mapping Tool - <a href="http://sbcounty.permitrack.com/WAP">http://sbcounty.permitrack.com/WAP</a></p>	<p>Santa Ana River</p>
<p><b>Hydrologic Conditions of Concern</b></p>	<p><input type="checkbox"/> Yes Complete Hydrologic Conditions of Concern (HCOC) Assessment. Include Forms 4.2-2 through Form 4.2-5 and Hydromodification BMP Form 4.3-10 in submittal</p> <p><input checked="" type="checkbox"/> No</p>
<p><b>Watershed-based BMP included in a RWQCB approved WAP</b></p>	<p><input type="checkbox"/> Yes Attach verification of regional BMP evaluation criteria in WAP</p> <ul style="list-style-type: none"> <li>• More Effective than On-site LID</li> <li>• Remaining Capacity for Project DCV</li> <li>• Upstream of any Water of the US</li> <li>• Operational at Project Completion</li> <li>• Long-Term Maintenance Plan</li> </ul> <p><input checked="" type="checkbox"/> No</p>

## Section 4 Best Management Practices (BMP)

### 4.1 Source Control BMP

#### 4.1.1 Pollution Prevention

Non-structural and structural source control BMP are required to be incorporated into all new development and significant redevelopment projects. Form 4.1-1 and 4.1-2 are used to describe specific source control BMPs used in the WQMP or to explain why a certain BMP is not applicable. Table 7-3 of the TGD for WQMP provides a list of applicable source control BMP for projects with specific types of potential pollutant sources or activities. The source control BMP in this table must be implemented for projects with these specific types of potential pollutant sources or activities.

The preparers of this WQMP have reviewed the source control BMP requirements for new development and significant redevelopment projects. The preparers have also reviewed the specific BMP required for project as specified in Forms 4.1-1 and 4.1-2. All applicable non-structural and structural source control BMP shall be implemented in the project.

The information provided in Form 4.1.1 and 4.1.2 is based on section 7 of the TGD for WQMP (p.92-105) including table 7-3, CASQA BMP Handbooks and comments from the reviewing agency. The provided description of BMP implementation is a summary and not intended to be an all-inclusive list of actions. Refer to the appendix 6.3 of the approved WQMP for applicable CASQA handouts and manufacturer information.

<b>Form 4.1-1 Non-Structural Source Control BMPs</b>				
Identifier	Name	Check One		Describe BMP Implementation OR, if not applicable, state reason
		Included	Not Applicable	
N1	Education of Property Owners, Tenants and Occupants on Stormwater BMPs	<input checked="" type="checkbox"/>	<input type="checkbox"/>	The Property Owner will provide BMP educational information materials to any employees, tenants (if any), and occupants. These materials will be included in the approved WQMP. Thereafter, such materials will be available through the county stormwater education program. The current website is <a href="http://www.sbcountystormwater.org">www.sbcountystormwater.org</a>
N2	Activity Restrictions	<input checked="" type="checkbox"/>	<input type="checkbox"/>	Activity restrictions will be imposed by the owner to limit exposure of stormwater to potential pollutants. Activity restrictions include limiting the site usage for its intended use and ensuring proper pesticide application.
N3	Landscape Management BMPs	<input checked="" type="checkbox"/>	<input type="checkbox"/>	Owner will ensure landscaping and irrigation is properly maintained.
N4	BMP Maintenance	<input checked="" type="checkbox"/>	<input type="checkbox"/>	The property owner will ensure regular inspection, repair, and maintenance of BMPs as noted in section 5 of this document.
N5	Title 22 CCR Compliance (How development will comply)	<input type="checkbox"/>	<input checked="" type="checkbox"/>	No hazardous waste storage.
N6	Local Water Quality Ordinances	<input checked="" type="checkbox"/>	<input type="checkbox"/>	This project will comply with all local water quality ordinances through this WQMP.
N7	Spill Contingency Plan	<input checked="" type="checkbox"/>	<input type="checkbox"/>	The Spill Contingency Plan developed by the facility operator shall include the following items. Chemical spill kit, similar to a ULine S-18303, with Absorption Capacity equal to or greater than the volume of chemicals stored on site. In the event of a spill call the San Bernardino County Fire Department Hazardous Materials Division 909-386-8401 for proper disposal of contaminated materials. Document the spill noting the time of occurrence, material, volume of spill and completed clean up. Restock spill material as needed.
N8	Underground Storage Tank Compliance	<input type="checkbox"/>	<input checked="" type="checkbox"/>	Not Applicable. No underground storage tanks are proposed
N9	Hazardous Materials Disclosure Compliance	<input type="checkbox"/>	<input checked="" type="checkbox"/>	Not applicable. Per San Bernardino County Fire, Hazardous Materials Division, the basic quantities for disclosure are: hazardous materials at or exceeding 55 gallons, 500 pounds, or 200 cubic feet at any time in the course of a year. The proposed use of this site does not meet this threshold.

**Water Quality Management Plan (WQMP)**

<b>Form 4.1-1 Non-Structural Source Control BMPs</b>				
N10	Uniform Fire Code Implementation	<input type="checkbox"/>	<input checked="" type="checkbox"/>	Not applicable. Per San Bernardino County Fire, Hazardous Materials Division, the basic quantities for disclosure are: hazardous materials at or exceeding 55 gallons, 500 pounds, or 200 cubic feet at any time in the course of a year. The proposed use of this site does not meet this threshold.
N11	Litter/Debris Control Program	<input checked="" type="checkbox"/>	<input type="checkbox"/>	Owner will ensure weekly inspection and clean up for litter and debris.
N12	Employee Training	<input checked="" type="checkbox"/>	<input type="checkbox"/>	Owner will ensure that employees are trained on BMPs.
N13	Housekeeping of Loading Docks	<input type="checkbox"/>	<input checked="" type="checkbox"/>	Not applicable. No loading docks proposed.
N14	Catch Basin Inspection Program	<input checked="" type="checkbox"/>	<input type="checkbox"/>	Owner will ensure 80 percent of drainage facilities inspected, cleaned and maintained on an annual basis with 100 percent of the facilities included in a two-year period. Cleaning will take place in the late summer/early fall prior to the start of the rainy season. Repairs will be done as needed.
N15	Vacuum Sweeping of Private Streets and Parking Lots	<input checked="" type="checkbox"/>	<input type="checkbox"/>	Parking areas shall be vacuumed and swepted monthly.
N16	Other Non-structural Measures for Public Agency Projects	<input checked="" type="checkbox"/>	<input type="checkbox"/>	All NPDES Compliance measures are implemented through this WQMP.
N17	Comply with all other applicable NPDES permits	<input checked="" type="checkbox"/>	<input type="checkbox"/>	The proposed site will comply with all NPDES permit requirements including a Storm Water Pollution Prevention Plan (SWPPP) and Water Quality Management Plan (WQMP).



<b>Form 4.1-2 Structural Source Control BMPs</b>				
Identifier	Name	Check One		Describe BMP Implementation OR, If not applicable, state reason
		Included	Not Applicable	
S1	Provide storm drain system stencilling and signage (CASQA New Development BMP Handbook SD-13)	<input checked="" type="checkbox"/>	<input type="checkbox"/>	All storm drain inlets and catch basins will be labeled. Stenciled labels shall state "No Dumping – Drains to River" or similar message discouraging any litter dumping.
S2	Design and construct outdoor material storage areas to reduce pollution introduction (CASQA New Development BMP Handbook SD-34)	<input type="checkbox"/>	<input checked="" type="checkbox"/>	Not applicable. No outdoor material storage areas proposed.
S3	Design and construct trash and waste storage areas to reduce pollution introduction (CASQA New Development BMP Handbook SD-32)	<input checked="" type="checkbox"/>	<input type="checkbox"/>	Trash storage areas will be designed in accordance with the reviewing jurisdiction development code and will provide secondary trash containment for the trash bins, as required by NPDES Permit No. CAS618036. These areas will provide storage of the state compliant receptacles with attached lids, that are provided by the local refuse service provider. Trash bin lids will be kept closed. Trash area will be paved with impervious surface, designed to not allow run-off or runoff and have permanent roofing over the enclosure.
S4	Use efficient irrigation systems & landscape design, water conservation, smart controllers, and source control (Statewide Model Landscape Ordinance; CASQA New Development BMP Handbook SD-12)	<input checked="" type="checkbox"/>	<input type="checkbox"/>	Owner will ensure landscaping and irrigation is properly maintained. Irrigation controls shall include rain-triggered shutoff devices to prevent irrigation after precipitation.
S5	Finish grade of landscaped areas at a minimum of 1-2 inches below top of curb, sidewalk, or pavement	<input checked="" type="checkbox"/>	<input type="checkbox"/>	Landscape areas will be a minimum of 1 inch below adjacent impervious areas.
S6	Protect slopes and channels and provide energy dissipation (CASQA New Development BMP Handbook SD-10)	<input checked="" type="checkbox"/>	<input type="checkbox"/>	Rip-Rap added to discharge location of underground system. Inspect annual and after major storm events. Repair as needed.
S7	Covered dock areas (CASQA New Development BMP Handbook SD-31)	<input type="checkbox"/>	<input checked="" type="checkbox"/>	Not applicable. No dock proposed.
S8	Covered maintenance bays with spill containment plans (CASQA New Development BMP Handbook SD-31)	<input type="checkbox"/>	<input checked="" type="checkbox"/>	Not applicable. No maintenance proposed.
S9	Vehicle wash areas with spill containment plans (CASQA New Development BMP Handbook SD-33)	<input type="checkbox"/>	<input checked="" type="checkbox"/>	Not applicable. No vehicle wash proposed.

**Water Quality Management Plan (WQMP)**

<b>Form 4.1-2 Structural Source Control BMPs</b>				
S10	Covered outdoor processing areas (CASQA New Development BMP Handbook SD-36)	<input type="checkbox"/>	<input checked="" type="checkbox"/>	Not applicable. No processing proposed.
S11	Equipment wash areas with spill containment plans (CASQA New Development BMP Handbook SD-33)	<input type="checkbox"/>	<input checked="" type="checkbox"/>	Not applicable. No equipment wash proposed.
S12	Fueling areas (CASQA New Development BMP Handbook SD-30)	<input type="checkbox"/>	<input checked="" type="checkbox"/>	Not applicable. No fueling proposed.
S13	Hillside landscaping (CASQA New Development BMP Handbook SD-10)	<input type="checkbox"/>	<input checked="" type="checkbox"/>	Not applicable. No hillside landscaping proposed.
S14	Wash water control for food preparation areas	<input type="checkbox"/>	<input checked="" type="checkbox"/>	Not applicable. No food preparation proposed.
S15	Community car wash racks (CASQA New Development BMP Handbook SD-33)	<input type="checkbox"/>	<input checked="" type="checkbox"/>	Not applicable. No car wash racks proposed.

### 4.1.2 Preventative LID Site Design Practices

Site design practices associated with new LID requirements in the MS4 Permit should be considered in the earliest phases of a project. Preventative site design practices can result in smaller DCV for LID BMP and hydromodification control BMP by reducing runoff generation. Describe site design and drainage plan including:

- A narrative of site design practices utilized or rationale for not using practices
- A narrative of how site plan incorporates preventive site design practices
- Include an attached Site Plan layout which shows how preventative site design practices are included in WQMP

Refer to Section 5.2 of the TGD for WQMP for more details.

<b>Form 4.1-3 Preventative LID Site Design Practices Checklist</b>
<p>Site Design Practices <i>If yes, explain how preventative site design practice is addressed in project site plan. If no, other LID BMPs must be selected to meet targets</i></p>
<p>Minimize impervious areas: Yes <input checked="" type="checkbox"/> No <input type="checkbox"/>                      Explanation: Impervious area has been minimized by designing drive isles to a minimum width and using pervious cover in the center courtyard area. Existing site 100% pervious, proposed site 27.43% pervious.</p>
<p>Maximize natural infiltration capacity: Yes <input checked="" type="checkbox"/> No <input type="checkbox"/>                      Explanation: Maximized natural infiltration capacity by incorporating a design that promotes water retention through placement of proposed landscape and infiltration BMPs.</p>
<p>Preserve existing drainage patterns and time of concentration: Yes <input checked="" type="checkbox"/> No <input type="checkbox"/>                      Explanation: Existing drainage patterns and time of concentration have been generally preserved by normalising flows in the southern basin prior to discharge from the site. Post construction mitigation was done per the county hydrology manual.</p>
<p>Disconnect impervious areas: Yes <input checked="" type="checkbox"/> No <input type="checkbox"/>                      Explanation: Site is approximately 70% impervious overall. LID BMP selected to meet WQMP requirements is an underground infiltration system.</p>
<p>Protect existing vegetation and sensitive areas: Yes <input type="checkbox"/> No <input checked="" type="checkbox"/>                      Explanation: No sensitive areas exist on site. The existing perennial grass is not in a condition worth saving. See exhibit for proposed landscaping locations. Infiltration LID BMP are proposed to meet the WQMP targets for this site.</p>
<p>Re-vegetate disturbed areas: Yes <input checked="" type="checkbox"/> No <input type="checkbox"/>                      Explanation: Disturbed areas will be re-vegetated with proposed landscape per plans.</p>
<p>Minimize unnecessary compaction in stormwater retention/infiltration basin/trench areas: Yes <input checked="" type="checkbox"/> No <input type="checkbox"/>                      Explanation: Stormwater BMP areas will be marked with flagging tape to minimize compaction and maximize natural infiltration capacity. See manufacturer's specifications for compaction requirements regarding the underground infiltration system.</p>
<p>Utilize vegetated drainage swales in place of underground piping or imperviously lined swales: Yes <input type="checkbox"/> No <input checked="" type="checkbox"/>                      Explanation: Vegetated swales are not proposed due to limited landscape area and small planters. The majority of landscape area is within the infiltration basin. Underground piping is needed collect the overflow and drain it to the underground BMP. LID BMP selected to meet target is underground infiltration system.</p>
<p>Stake off areas that will be used for landscaping to minimize compaction during construction : Yes <input checked="" type="checkbox"/> No <input type="checkbox"/>                      Explanation: Landscape areas will be marked with flagging to minimize compaction and maximize natural infiltration capacity.</p>

## 4.2 Project Performance Criteria

The purpose of this section of the Project WQMP is to establish targets for post-development hydrology based on performance criteria specified in the MS4 Permit. These targets include runoff volume for water quality control (referred to as LID design capture volume), and runoff volume, time of concentration, and peak runoff for protection of any downstream waterbody segments with a HCOC. ***If the project has more than one outlet for stormwater runoff, then complete additional versions of these forms for each DA / outlet.***

Methods applied in the following forms include:

- For LID BMP Design Capture Volume (DCV), the San Bernardino County Stormwater Program requires use of the P<sub>6</sub> method (MS4 Permit Section XI.D.6a.ii) – Form 4.2-1
- For HCOC pre- and post-development hydrologic calculation, the San Bernardino County Stormwater Program requires the use of the Rational Method (San Bernardino County Hydrology Manual Section D). Forms 4.2-2 through Form 4.2-5 calculate hydrologic variables including runoff volume, time of concentration, and peak runoff from the project site pre- and post-development using the Hydrology Manual Rational Method approach. For projects greater than 640 acres (1.0 mi<sup>2</sup>), the Rational Method and these forms should not be used. For such projects, the Unit Hydrograph Method (San Bernardino County Hydrology Manual Section E) shall be applied for hydrologic calculations for HCOC performance criteria.

Refer to Section 4 in the TGD for WQMP for detailed guidance and instructions.

<b>Form 4.2-1 LID BMP Performance Criteria for Design Capture Volume (DA 1)</b>		
<b>1</b> Project area DA 1 (ft <sup>2</sup> ): 245,989	<b>2</b> Imperviousness after applying preventative site design practices (Imp%): 77.46	<b>3</b> Runoff Coefficient (Rc): 0.570 $R_c = 0.858(\text{Imp}\%)^3 - 0.78(\text{Imp}\%)^2 + 0.774(\text{Imp}\%) + 0.04$
<b>4</b> Determine 1-hour rainfall depth for a 2-year return period P <sub>2yr-1hr</sub> (in): 0.543 <a href="http://hdsc.nws.noaa.gov/hdsc/pfds/so/sca_pfds.html">http://hdsc.nws.noaa.gov/hdsc/pfds/so/sca_pfds.html</a>		
<b>5</b> Compute P <sub>6</sub> , Mean 6-hr Precipitation (inches): 0.804 <i>P<sub>6</sub> = Item 4 * C<sub>1</sub>, where C<sub>1</sub> is a function of site climatic region specified in Form 3-1 Item 1 (Valley = 1.4807; Mountain = 1.909; Desert = 1.2371)</i>		
<b>6</b> Drawdown Rate <i>Use 48 hours as the default condition. Selection and use of the 24 hour drawdown time condition is subject to approval by the local jurisdiction. The necessary BMP footprint is a function of drawdown time. While shorter drawdown times reduce the performance criteria for LID BMP design capture volume, the depth of water that can be stored is also reduced.</i>		24-hrs <input type="checkbox"/> 48-hrs <input checked="" type="checkbox"/>
<b>7</b> Compute design capture volume, DCV (ft <sup>3</sup> ): 18,451 $DCV = 1/12 * [\text{Item 1} * \text{Item 3} * \text{Item 5} * C_2]$ , where C <sub>2</sub> is a function of drawdown rate (24-hr = 1.582; 48-hr = 1.963) <i>Compute separate DCV for each outlet from the project site per schematic drawn in Form 3-1 Item 2</i>		

Form 4.2-1 LID BMP Performance Criteria for Design Capture Volume (DA 2)		
<b>1</b> Project area DA 2 (ft <sup>2</sup> ): 18,456	<b>2</b> Imperviousness after applying preventative site design practices (Imp%): 2.36	<b>3</b> Runoff Coefficient (Rc): 0.058 $R_c = 0.858(\text{Imp}\%)^{0.3} - 0.78(\text{Imp}\%)^{0.2} + 0.774(\text{Imp}\%) + 0.04$
<b>4</b> Determine 1-hour rainfall depth for a 2-year return period $P_{2\text{yr-1hr}}$ (in): 0.543 <a href="http://hdsc.nws.noaa.gov/hdsc/pfds/so/sca_pfds.html">http://hdsc.nws.noaa.gov/hdsc/pfds/so/sca_pfds.html</a>		
<b>5</b> Compute $P_6$ , Mean 6-hr Precipitation (inches): 0.804 $P_6 = \text{Item 4} * C_1$ , where $C_1$ is a function of site climatic region specified in Form 3-1 Item 1 (Valley = 1.4807; Mountain = 1.909; Desert = 1.2371)		
<b>6</b> Drawdown Rate Use 48 hours as the default condition. Selection and use of the 24 hour drawdown time condition is subject to approval by the local jurisdiction. The necessary BMP footprint is a function of drawdown time. While shorter drawdown times reduce the performance criteria for LID BMP design capture volume, the depth of water that can be stored is also reduced.		24-hrs <input type="checkbox"/> 48-hrs <input checked="" type="checkbox"/>
<b>7</b> Compute design capture volume, DCV (ft <sup>3</sup> ): 140 $DCV = 1/12 * [\text{Item 1} * \text{Item 3} * \text{Item 5} * C_2]$ , where $C_2$ is a function of drawdown rate (24-hr = 1.582; 48-hr = 1.963) Compute separate DCV for each outlet from the project site per schematic drawn in Form 3-1 Item 2		

Form 4.2-2 Summary of HCOC Assessment (DA 1-2)			
Does project have the potential to cause or contribute to an HCOC in a downstream channel: Yes <input type="checkbox"/> No <input checked="" type="checkbox"/>			
Go to: <a href="http://sbcounty.permitrack.com/WAP">http://sbcounty.permitrack.com/WAP</a>			
If "Yes", then complete HCOC assessment of site hydrology for 2yr storm event using Forms 4.2-3 through 4.2-5 and insert results below (Forms 4.2-3 through 4.2-5 may be replaced by computer software analysis based on the San Bernardino County Hydrology Manual)			
If "No," then proceed to Section 4.3 Project Conformance Analysis			
Condition	Runoff Volume (ft <sup>3</sup> )	Time of Concentration (min)	Peak Runoff (cfs)
Pre-developed	<b>1</b> <i>Form 4.2-3 Item 12</i>	<b>2</b> <i>Form 4.2-4 Item 13</i>	<b>3</b> <i>Form 4.2-5 Item 10</i>
Post-developed	<b>4</b> <i>Form 4.2-3 Item 13</i>	<b>5</b> <i>Form 4.2-4 Item 14</i>	<b>6</b> <i>Form 4.2-5 Item 14</i>
Difference	<b>7</b> <i>Item 4 – Item 1</i>	<b>8</b> <i>Item 2– Item 5</i>	<b>9</b> <i>Item 6 – Item 3</i>
Difference (as % of pre-developed)	<b>10</b> 0% <i>Item 7 / Item 1</i>	<b>11</b> 0% <i>Item 8 / Item 2</i>	<b>12</b> 0% <i>Item 9 / Item 3</i>

**Form 4.2-3 HCOC Assessment for Runoff Volume (DA 1-2)**

<b>Weighted Curve Number Determination for: Pre-developed DA</b>	DMA A	DMA B	DMA C	DMA D	DMA E	DMA F	DMA G	DMA H
<b>1a</b> Land Cover type								
<b>2a</b> Hydrologic Soil Group (HSG)								
<b>3a</b> DMA Area, ft <sup>2</sup> sum of areas of DMA should equal area of DA								
<b>4a</b> Curve Number (CN) use Items 1 and 2 to select the appropriate CN from Appendix C-2 of the TGD for WQMP								
<b>Weighted Curve Number Determination for: Post-developed DA</b>	DMA A	DMA B	DMA C	DMA D	DMA E	DMA F	DMA G	DMA H
<b>1b</b> Land Cover type								
<b>2b</b> Hydrologic Soil Group (HSG)								
<b>3b</b> DMA Area, ft <sup>2</sup> sum of areas of DMA should equal area of DA								
<b>4b</b> Curve Number (CN) use Items 5 and 6 to select the appropriate CN from Appendix C-2 of the TGD for WQMP								
<b>5</b> Pre-Developed area-weighted CN:	<b>7</b> Pre-developed soil storage capacity, S (in): $S = (1000 / \text{Item } 5) - 10$				<b>9</b> Initial abstraction, I <sub>a</sub> (in): $I_a = 0.2 * \text{Item } 7$			
<b>6</b> Post-Developed area-weighted CN:	<b>8</b> Post-developed soil storage capacity, S (in): $S = (1000 / \text{Item } 6) - 10$				<b>10</b> Initial abstraction, I <sub>a</sub> (in): $I_a = 0.2 * \text{Item } 8$			
<b>11</b> Precipitation for 2 yr, 24 hr storm (in): Go to: <a href="http://hdsc.nws.noaa.gov/hdsc/pfds/sa/sca_pfds.html">http://hdsc.nws.noaa.gov/hdsc/pfds/sa/sca_pfds.html</a>								
<b>12</b> Pre-developed Volume (ft <sup>3</sup> ): $V_{pre} = (1 / 12) * (\text{Item sum of Item } 3) * [(\text{Item } 11 - \text{Item } 9)^2 / ((\text{Item } 11 - \text{Item } 9 + \text{Item } 7))$								
<b>13</b> Post-developed Volume (ft <sup>3</sup> ): $V_{pre} = (1 / 12) * (\text{Item sum of Item } 3) * [(\text{Item } 11 - \text{Item } 10)^2 / ((\text{Item } 11 - \text{Item } 10 + \text{Item } 8))$								
<b>14</b> Volume Reduction needed to meet HCOC Requirement, (ft <sup>3</sup> ): $V_{HCOC} = (\text{Item } 13 * 0.95) - \text{Item } 12$								

<b>Form 4.2-4 HCOC Assessment for Time of Concentration (DA 1-2)</b>								
Compute time of concentration for pre and post developed conditions for each DA ( <i>For projects using the Hydrology Manual complete the form below</i> )								
Variables	Pre-developed DA1 <i>Use additional forms if there are more than 4 DMA</i>				Post-developed DA1 <i>Use additional forms if there are more than 4 DMA</i>			
	DMA A	DMA B	DMA C	DMA D	DMA A	DMA B	DMA C	DMA D
<b>1</b> Length of flowpath (ft) <i>Use Form 3-2 Item 5 for pre-developed condition</i>								
<b>2</b> Change in elevation (ft)								
<b>3</b> Slope (ft/ft), $S_o = \text{Item 2} / \text{Item 1}$								
<b>4</b> Land cover								
<b>5</b> Initial DMA Time of Concentration (min) <i>Appendix C-1 of the TGD for WQMP</i>								
<b>6</b> Length of conveyance from DMA outlet to project site outlet (ft) <i>May be zero if DMA outlet is at project site outlet</i>								
<b>7</b> Cross-sectional area of channel (ft <sup>2</sup> )								
<b>8</b> Wetted perimeter of channel (ft)								
<b>9</b> Manning's roughness of channel (n)								
<b>10</b> Channel flow velocity (ft/sec) $V_{fps} = (1.49 / \text{Item 9}) * (\text{Item 7}/\text{Item 8})^{0.67} * (\text{Item 3})^{0.5}$								
<b>11</b> Travel time to outlet (min) $T_t = \text{Item 6} / (\text{Item 10} * 60)$								
<b>12</b> Total time of concentration (min) $T_c = \text{Item 5} + \text{Item 11}$								
<b>13</b> Pre-developed time of concentration (min):	<i>Minimum of Item 12 pre-developed DMA</i>							
<b>14</b> Post-developed time of concentration (min):	<i>Minimum of Item 12 post-developed DMA</i>							
<b>15</b> Additional time of concentration needed to meet HCOC requirement (min):	$T_{C-HCOC} = (\text{Item 14} * 0.95) - \text{Item 13}$							

## Form 4.2-5 HCOC Assessment for Peak Runoff (DA 1-2)

Compute peak runoff for pre- and post-developed conditions						
Variables	Pre-developed DA to Project Outlet <i>(Use additional forms if more than 3 DMA)</i>			Post-developed DA to Project Outlet <i>(Use additional forms if more than 3 DMA)</i>		
	DMA A	DMA B	DMA C	DMA A	DMA B	DMA C
<b>1</b> Rainfall Intensity for storm duration equal to time of concentration <i><math>I_{peak} = 10^{(LOG Form 4.2-1 Item 4 - 0.6 LOG Form 4.2-4 Item 5 / 60)}</math></i>						
<b>2</b> Drainage Area of each DMA (ft <sup>2</sup> ) <i>For DMA with outlet at project site outlet, include upstream DMA (Using example schematic in Form 3-1, DMA A will include drainage from DMA C)</i>						
<b>3</b> Ratio of pervious area to total area <i>For DMA with outlet at project site outlet, include upstream DMA (Using example schematic in Form 3-1, DMA A will include drainage from DMA C)</i>						
<b>4</b> Pervious area infiltration rate (in/hr) <i>Use pervious area CN and antecedent moisture condition with Appendix C-3 of the TGD for WQMP</i>						
<b>5</b> Maximum loss rate (in/hr) <i><math>F_m = Item 3 * Item 4</math>                      Use area-weighted <math>F_m</math> from DMA with outlet at project site outlet, include upstream DMA (Using example schematic in Form 3-1, DMA A will include drainage from DMA C)</i>						
<b>6</b> Peak Flow from DMA (cfs) <i><math>Q_p = Item 2 * 0.9 * (Item 1 - Item 5)</math></i>						
<b>7</b> Time of concentration adjustment factor for other DMA to site discharge point <i>Form 4.2-4 Item 12 DMA / Other DMA upstream of site discharge point (If ratio is greater than 1.0, then use maximum value of 1.0)</i>	DMA A	<i>n/a</i>		DMA A	<i>n/a</i>	
	DMA B		<i>n/a</i>		<i>n/a</i>	
	DMA C			<i>n/a</i>		<i>n/a</i>
<b>8</b> Pre-developed $Q_p$ at $T_c$ for DMA A: <i><math>Q_p = Item 6_{DMAA} + [Item 6_{DMAB} * (Item 1_{DMAA} - Item 5_{DMAB}) / (Item 1_{DMAB} - Item 5_{DMAB}) * Item 7_{DMAA/2}] + [Item 6_{DMAC} * (Item 1_{DMAA} - Item 5_{DMAC}) / (Item 1_{DMAC} - Item 5_{DMAC}) * Item 7_{DMAA/3}]</math></i>	<b>9</b> Pre-developed $Q_p$ at $T_c$ for DMA B: <i><math>Q_p = Item 6_{DMAB} + [Item 6_{DMAA} * (Item 1_{DMAB} - Item 5_{DMAA}) / (Item 1_{DMAA} - Item 5_{DMAA}) * Item 7_{DMAB/1}] + [Item 6_{DMAC} * (Item 1_{DMAB} - Item 5_{DMAC}) / (Item 1_{DMAC} - Item 5_{DMAC}) * Item 7_{DMAB/3}]</math></i>		<b>10</b> Pre-developed $Q_p$ at $T_c$ for DMA C: <i><math>Q_p = Item 6_{DMAC} + [Item 6_{DMAA} * (Item 1_{DMAC} - Item 5_{DMAA}) / (Item 1_{DMAA} - Item 5_{DMAA}) * Item 7_{DMAC/1}] + [Item 6_{DMAB} * (Item 1_{DMAC} - Item 5_{DMAB}) / (Item 1_{DMAB} - Item 5_{DMAB}) * Item 7_{DMAC/2}]</math></i>			
<b>10</b> Peak runoff from pre-developed condition confluence analysis (cfs): <span style="float: right;"><i>Maximum of Item 8, 9, and 10 (including additional forms as needed)</i></span>						
<b>11</b> Post-developed $Q_p$ at $T_c$ for DMA A: <i>Same as Item 8 for post-developed values</i>	<b>12</b> Post-developed $Q_p$ at $T_c$ for DMA B: <i>Same as Item 9 for post-developed values</i>		<b>13</b> Post-developed $Q_p$ at $T_c$ for DMA C: <i>Same as Item 10 for post-developed values</i>			
<b>14</b> Peak runoff from post-developed condition confluence analysis (cfs): <span style="float: right;"><i>Maximum of Item 11, 12, and 13 (including additional forms as needed)</i></span>						
<b>15</b> Peak runoff reduction needed to meet HCOC Requirement (cfs): <span style="float: right;"><i><math>Q_{p-HCOC} = (Item 14 * 0.95) - Item 10</math></i></span>						



## 4.3 Project Conformance Analysis

Complete the following forms for each project site DA to document that the proposed LID BMPs conform to the project DCV developed to meet performance criteria specified in the MS<sub>4</sub> Permit (WQMP Template Section 4.2). For the LID DCV, the forms are ordered according to hierarchy of BMP selection as required by the MS<sub>4</sub> Permit (see Section 5.3.1 in the TGD for WQMP). The forms compute the following for on-site LID BMP:

- Site Design and Hydrologic Source Controls (Form 4.3-2)
- Retention and Infiltration (Form 4.3-3)
- Harvested and Use (Form 4.3-4) or
- Biotreatment (Form 4.3-5).

At the end of each form, additional fields facilitate the determination of the extent of mitigation provided by the specific BMP category, allowing for use of the next category of BMP in the hierarchy, if necessary.

The first step in the analysis, using Section 5.3.2.1 of the TGD for WQMP, is to complete Forms 4.3-1 and 4.3-3) to determine if retention and infiltration BMPs are infeasible for the project. For each feasibility criterion in Form 4.3-1, if the answer is “Yes,” provide all study findings that includes relevant calculations, maps, data sources, etc. used to make the determination of infeasibility.

Next, complete Forms 4.3-2 and 4.3-4 to determine the feasibility of applicable HSC and harvest and use BMPs, and, if their implementation is feasible, the extent of mitigation of the DCV.

If no site constraints exist that would limit the type of BMP to be implemented in a DA, evaluate the use of combinations of LID BMPs, including all applicable HSC BMPs to maximize on-site retention of the DCV. If no combination of BMP can mitigate the entire DCV, implement the single BMP type, or combination of BMP types, that maximizes on-site retention of the DCV within the minimum effective area.

If the combination of LID HSC, retention and infiltration, and harvest and use BMPs are unable to mitigate the entire DCV, then biotreatment BMPs may be implemented by the project proponent. If biotreatment BMPs are used, then they must be sized to provide sufficient capacity for effective treatment of the remainder of the volume-based performance criteria that cannot be achieved with LID BMPs (TGD for WQMP Section 5.4.4.2). **Under no circumstances shall any portion of the DCV be released from the site without effective mitigation and/or treatment.**

### Form 4.3-1 Infiltration BMP Feasibility (DA 1-2)

Feasibility Criterion – Complete evaluation for each DA on the Project Site

**1** Would infiltration BMP pose significant risk for groundwater related concerns? Yes  No

*Refer to Section 5.3.2.1 of the TGD for WQMP*

If Yes, Provide basis: (attach)

**2** Would installation of infiltration BMP significantly increase the risk of geotechnical hazards? Yes  No

(Yes, if the answer to any of the following questions is yes, as established by a geotechnical expert):

- The location is less than 50 feet away from slopes steeper than 15 percent
- The location is less than eight feet from building foundations or an alternative setback.
- A study certified by a geotechnical professional or an available watershed study determines that stormwater infiltration would result in significantly increased risks of geotechnical hazards.

If Yes, Provide basis: (attach)

**3** Would infiltration of runoff on a Project site violate downstream water rights? Yes  No

If Yes, Provide basis: (attach)

**4** Is proposed infiltration facility located on hydrologic soil group (HSG) D soils or does the site geotechnical investigation indicate presence of soil characteristics, which support categorization as D soils? Yes  No

If Yes, Provide basis: (attach)

**5** Is the design infiltration rate, after accounting for safety factor of 2.0, below proposed facility less than 0.3 in/hr (accounting for soil amendments)? Yes  No

If Yes, Provide basis: (attach)

**6** Would on-site infiltration or reduction of runoff over pre-developed conditions be partially or fully inconsistent with watershed management strategies as defined in the WAP, or impair beneficial uses? Yes  No

*See Section 3.5 of the TGD for WQMP and WAP*

If Yes, Provide basis: (attach)

**7** Any answer from Item 1 through Item 3 is “Yes”: Yes  No

*If yes, infiltration of any volume is not feasible onsite. Proceed to Form 4.3-4, Harvest and Use BMP. If no, then proceed to Item 9 below.*

**8** Any answer from Item 4 through Item 6 is “Yes”: Yes  No

*If yes, infiltration is permissible but is not required to be considered. Proceed to Form 4.3-2, Hydrologic Source Control BMP.*

*If no, then proceed to Item 9, below.*

**9** All answers to Item 1 through Item 6 are “No”: Yes  No

*Infiltration of the full DCV is potentially feasible, LID infiltration BMP must be designed to infiltrate the full DCV to the MEP.*

*Proceed to Form 4.3-2, Hydrologic Source Control BMP.*

### 4.3.1 Site Design Hydrologic Source Control BMP

Section XI.E. of the Permit emphasizes the use of LID preventative measures; and the use of LID HSC BMPs reduces the portion of the DCV that must be addressed in downstream BMPs. Therefore, all applicable HSC shall be provided except where they are mutually exclusive with each other, or with other BMPs. Mutual exclusivity may result from overlapping BMP footprints such that either would be potentially feasible by itself, but both could not be implemented. Please note that while there are no numeric standards regarding the use of HSC, if a project cannot feasibly meet BMP sizing requirements or cannot fully address HCOCs, feasibility of all applicable HSC must be part of demonstrating that the BMP system has been designed to retain the maximum feasible portion of the DCV. Complete Form 4.3-2 to identify and calculate estimated retention volume from implementing site design HSC BMP. Refer to Section 5.4.1 in the TGD for more detailed guidance.

<b>Form 4.3-2 Site Design Hydrologic Source Control BMPs (DA 1-2)</b>			
<b>1</b> Implementation of Impervious Area Dispersion BMP (i.e. routing runoff from impervious to pervious areas), excluding impervious areas planned for routing to on-lot infiltration BMP: Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> <i>If yes, complete Items 2-5; if no, proceed to Item 6</i>	DA DMA BMP Type	DA DMA BMP Type	DA DMA BMP Type <i>(Use additional forms for more BMPs)</i>
<b>2</b> Total impervious area draining to pervious area (ft <sup>2</sup> )			
<b>3</b> Ratio of pervious area receiving runoff to impervious area			
<b>4</b> Retention volume achieved from impervious area dispersion (ft <sup>3</sup> ) $V = \text{Item 2} * \text{Item 3} * (0.5/12)$ , assuming retention of 0.5 inches of runoff	0	0	0
<b>5</b> Sum of retention volume achieved from impervious area dispersion (ft <sup>3</sup> ): 0 $V_{\text{retention}} = \text{Sum of Item 4 for all BMPs}$			
<b>6</b> Implementation of Localized On-lot Infiltration BMPs (e.g. on-lot rain gardens): Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> <i>If yes, complete Items 7-13 for aggregate of all on-lot infiltration BMP in each DA; if no, proceed to Item 14</i>	DA DMA BMP Type	DA DMA BMP Type	DA DMA BMP Type <i>(Use additional forms for more BMPs)</i>
<b>7</b> Ponding surface area (ft <sup>2</sup> )			
<b>8</b> Ponding depth (ft)			
<b>9</b> Surface area of amended soil/gravel (ft <sup>2</sup> )			
<b>10</b> Average depth of amended soil/gravel (ft)			
<b>11</b> Average porosity of amended soil/gravel			
<b>12</b> Retention volume achieved from on-lot infiltration (ft <sup>3</sup> ) $V_{\text{retention}} = (\text{Item 7} * \text{Item 8}) + (\text{Item 9} * \text{Item 10} * \text{Item 11})$	0	0	0
<b>13</b> Runoff volume retention from on-lot infiltration (ft <sup>3</sup> ): 0 $V_{\text{retention}} = \text{Sum of Item 12 for all BMPs}$			

<b>Form 4.3-2 Site Design Hydrologic Source Control BMPs (DA 1-2)</b>			
<b>14</b> Implementation of evapotranspiration BMP (green, brown, or blue roofs): Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> <i>If yes, complete Items 15-20. If no, proceed to Item 21</i>	DA DMA BMP Type	DA DMA BMP Type	DA DMA BMP Type (Use additional forms for more BMPs)
<b>15</b> Rooftop area planned for ET BMP (ft <sup>2</sup> )			
<b>16</b> Average wet season ET demand (in/day) <i>Use local values, typical ~ 0.1</i>			
<b>17</b> Daily ET demand (ft <sup>3</sup> /day) <i>Item 15 * (Item 16 / 12)</i>			
<b>18</b> Drawdown time (hrs) <i>Copy Item 6 in Form 4.2-1</i>			
<b>19</b> Retention Volume (ft <sup>3</sup> ) <i>V<sub>retention</sub> = Item 17 * (Item 18 / 24)</i>	0	0	0
<b>20</b> Runoff volume retention from evapotranspiration BMPs (ft <sup>3</sup> ): 0 <i>V<sub>retention</sub> = Sum of Item 19 for all BMPs</i>			
<b>21</b> Implementation of Street Trees: Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> <i>If yes, complete Items 20-2. If no, proceed to Item 24</i>	DA DMA BMP Type	DA DMA BMP Type	DA DMA BMP Type (Use additional forms for more BMPs)
<b>22</b> Number of Street Trees			
<b>23</b> Average canopy cover over impervious area (ft <sup>2</sup> )			
<b>24</b> Runoff volume retention from street trees (ft <sup>3</sup> ) <i>V<sub>retention</sub> = Item 22 * Item 23 * (0.05/12) assume runoff retention of 0.05 inches</i>	0	0	0
<b>25</b> Runoff volume retention from street tree BMPs (ft <sup>3</sup> ): 0 <i>V<sub>retention</sub> = Sum of Item 24 for all BMPs</i>			
<b>26</b> Implementation of residential rain barrels/cisterns: Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> <i>If yes, complete Items 27-28; If no, proceed to Item 29</i>	DA DMA BMP Type	DA DMA BMP Type	DA DMA BMP Type (Use additional forms for more BMPs)
<b>27</b> Number of rain barrels/cisterns			
<b>28</b> Runoff volume retention from rain barrels/cisterns (ft <sup>3</sup> ) <i>V<sub>retention</sub> = Item 27 * 3</i>	0	0	0
<b>29</b> Runoff volume retention from residential rain barrels/Cisterns (ft <sup>3</sup> ): 0 <i>V<sub>retention</sub> = Sum of Item 28 for all BMPs</i>			
<b>30</b> Total Retention Volume from Site Design Hydrologic Source Control BMPs: 0 <i>Sum of Items 5, 13, 20, 25 and 29</i>			

### 4.3.2 Infiltration BMPs

Use Form 4.3-3 to compute on-site retention of runoff from proposed retention and infiltration BMPs. Volume retention estimates are sensitive to the percolation rate used, which determines the amount of runoff that can be infiltrated within the specified drawdown time. The infiltration safety factor reduces field measured percolation to account for potential inaccuracy associated with field measurements, declining BMP performance over time, and compaction during construction. Appendix D of the TGD for WQMP provides guidance on estimating an appropriate safety factor to use in Form 4.3-3.

If site constraints limit the use of BMPs to a single type and implementation of retention and infiltration BMPs mitigate no more than 40% of the DCV, then they are considered infeasible and the Project Proponent may evaluate the effectiveness of BMPs lower in the LID hierarchy of use (Section 5.5.1 of the TGD for WQMP)

If implementation of infiltrations BMPs is feasible as determined using Form 4.3-1, then LID infiltration BMPs shall be implemented to the MEP (section 4.1 of the TGD for WQMP).

**Form 4.3-3 Infiltration LID BMP - including underground BMPs (DA 1)**

<p><b>1</b> Remaining LID DCV not met by site design HSC BMP (ft<sup>3</sup>): 18451 <math>V_{unmet} = \text{Form 4.2-1 Item 7} - \text{Form 4.3-2 Item 30}</math></p>			
<p>BMP Type Use columns to the right to compute runoff volume retention from proposed infiltration BMP (select BMP from Table 5-4 in TGD for WQMP) - Use additional forms for more BMPs</p>	<p>DA 1 DMA BMP Type Underground Infiltration System</p>	<p>DA 0 DMA BMP Type</p>	<p>DA 0 DMA BMP Type (Use additional forms for more BMPs)</p>
<p><b>2</b> Infiltration rate of underlying soils (in/hr) See Section 5.4.2 and Appendix D of the TGD for WQMP for minimum requirements for assessment methods</p>	1.82		
<p><b>3</b> Infiltration safety factor See TGD Section 5.4.2 and Appendix D</p>	2.81		
<p><b>4</b> Design percolation rate (in/hr) <math>P_{design} = \text{Item 2} / \text{Item 3}</math></p>	0.65		
<p><b>5</b> Pondered water drawdown time (hr) Copy Item 6 in Form 4.2-1</p>	48		
<p><b>6</b> Maximum ponding depth (ft) BMP specific, see Table 5-4 of the TGD for WQMP for BMP design details</p>	2.6		
<p><b>7</b> Ponding Depth (ft) <math>d_{BMP} = \text{Minimum of } (1/12 * \text{Item 4} * \text{Item 5}) \text{ or Item 6}</math></p>	2.6		
<p><b>8</b> Infiltrating surface area, <math>SA_{BMP}</math> (ft<sup>2</sup>) the lesser of the area needed for infiltration of full DCV or minimum space requirements from Table 5.7 of the TGD for WQMP</p>	9,750		
<p><b>9</b> Amended soil depth, <math>d_{media}</math> (ft) Only included in certain BMP types, see Table 5-4 in the TGD for WQMP for reference to BMP design details</p>	N/A		
<p><b>10</b> Amended soil porosity</p>	N/A		
<p><b>11</b> Gravel depth, <math>d_{media}</math> (ft) Only included in certain BMP types, see Table 5-4 of the TGD for WQMP for BMP design details</p>	N/A		
<p><b>12</b> Gravel porosity</p>	N/A		
<p><b>13</b> Duration of storm as basin is filling (hrs) Typical ~ 3hrs</p>	3		
<p><b>14</b> Above Ground Retention Volume (ft<sup>3</sup>) <math>V_{retention} = \text{Item 8} * [\text{Item 7} + (\text{Item 9} * \text{Item 10}) + (\text{Item 11} * \text{Item 12}) + (\text{Item 13} * (\text{Item 4} / 12))]</math></p>	0		
<p><b>15</b> Underground Retention Volume (ft<sup>3</sup>) Volume determined using manufacturer's specifications and calculations</p>	18,897		
<p><b>16</b> Total Retention Volume from LID Infiltration BMPs: 18,897 (Sum of Items 14 and 15 for all infiltration BMP included in plan)</p>			
<p><b>17</b> Fraction of DCV achieved with infiltration BMP: 102% <math>\text{Retention\%} = \text{Item 16} / \text{Form 4.2-1 Item 7}</math></p>			
<p><b>18</b> Is full LID DCV retained on-site with combination of hydrologic source control and LID retention and infiltration BMPs? Yes <input checked="" type="checkbox"/> No <input type="checkbox"/>                  If yes, demonstrate conformance using Form 4.3-10; if no, then reduce Item 3, Factor of Safety to 2.0 and increase Item 8, Infiltrating Surface Area, such that the portion of the site area used for retention and infiltration BMPs equals or exceeds the minimum effective area thresholds (Table 5-7 of the TGD for WQMP) for the applicable category of development and repeat all above calculations.</p>			

**Form 4.3-3 Infiltration LID BMP - including underground BMPs (DA 2)**

<p><b>1</b> Remaining LID DCV not met by site design HSC BMP (ft<sup>3</sup>): 140 <math>V_{unmet} = \text{Form 4.2-1 Item 7} - \text{Form 4.3-2 Item 30}</math></p>			
<p>BMP Type Use columns to the right to compute runoff volume retention from proposed infiltration BMP (select BMP from Table 5-4 in TGD for WQMP) - Use additional forms for more BMPs</p>	<p>DA 2 DMA BMP Type Infiltration Basin</p>	<p>DA 0 DMA BMP Type</p>	<p>DA 0 DMA BMP Type (Use additional forms for more BMPs)</p>
<p><b>2</b> Infiltration rate of underlying soils (in/hr) See Section 5.4.2 and Appendix D of the TGD for WQMP for minimum requirements for assessment methods</p>	1.82		
<p><b>3</b> Infiltration safety factor See TGD Section 5.4.2 and Appendix D</p>	2.81		
<p><b>4</b> Design percolation rate (in/hr) <math>P_{design} = \text{Item 2} / \text{Item 3}</math></p>	0.65		
<p><b>5</b> Pondered water drawdown time (hr) Copy Item 6 in Form 4.2-1</p>	48		
<p><b>6</b> Maximum ponding depth (ft) BMP specific, see Table 5-4 of the TGD for WQMP for BMP design details</p>	1.5		
<p><b>7</b> Ponding Depth (ft) <math>d_{BMP} = \text{Minimum of } (1/12 * \text{Item 4} * \text{Item 5}) \text{ or Item 6}</math></p>	2.7		
<p><b>8</b> Infiltrating surface area, <math>SA_{BMP}</math> (ft<sup>2</sup>) the lesser of the area needed for infiltration of full DCV or minimum space requirements from Table 5.7 of the TGD for WQMP</p>	4,358		
<p><b>9</b> Amended soil depth, <math>d_{media}</math> (ft) Only included in certain BMP types, see Table 5-4 in the TGD for WQMP for reference to BMP design details</p>	N/A		
<p><b>10</b> Amended soil porosity</p>	N/A		
<p><b>11</b> Gravel depth, <math>d_{media}</math> (ft) Only included in certain BMP types, see Table 5-4 of the TGD for WQMP for BMP design details</p>	N/A		
<p><b>12</b> Gravel porosity</p>	N/A		
<p><b>13</b> Duration of storm as basin is filling (hrs) Typical ~ 3hrs</p>	3		
<p><b>14</b> Above Ground Retention Volume (ft<sup>3</sup>) <math>V_{retention} = \text{Item 8} * [\text{Item 7} + (\text{Item 9} * \text{Item 10}) + (\text{Item 11} * \text{Item 12}) + (\text{Item 13} * (\text{Item 4} / 12))]</math></p>	6537		
<p><b>15</b> Underground Retention Volume (ft<sup>3</sup>) Volume determined using manufacturer's specifications and calculations</p>	0		
<p><b>16</b> Total Retention Volume from LID Infiltration BMPs: 6,537 (Sum of Items 14 and 15 for all infiltration BMP included in plan)</p>			
<p><b>17</b> Fraction of DCV achieved with infiltration BMP: 4654% <math>\text{Retention\%} = \text{Item 16} / \text{Form 4.2-1 Item 7}</math></p>			
<p><b>18</b> Is full LID DCV retained on-site with combination of hydrologic source control and LID retention and infiltration BMPs? Yes <input checked="" type="checkbox"/> No <input type="checkbox"/>                  If yes, demonstrate conformance using Form 4.3-10; if no, then reduce Item 3, Factor of Safety to 2.0 and increase Item 8, Infiltrating Surface Area, such that the portion of the site area used for retention and infiltration BMPs equals or exceeds the minimum effective area thresholds (Table 5-7 of the TGD for WQMP) for the applicable category of development and repeat all above calculations.</p>			

### 4.3.3 Harvest and Use BMP

Harvest and use BMP may be considered if the full LID DCV cannot be met by maximizing infiltration BMPs. Use Form 4.3-4 to compute on-site retention of runoff from proposed harvest and use BMPs.

Volume retention estimates for harvest and use BMPs are sensitive to the on-site demand for captured stormwater. Since irrigation water demand is low in the wet season, when most rainfall events occur in San Bernardino County, the volume of water that can be used within a specified drawdown period is relatively low. The bottom portion of Form 4.3-4 facilitates the necessary computations to show infeasibility if a minimum incremental benefit of 40 percent of the LID DCV would not be achievable with MEP implementation of on-site harvest and use of stormwater (Section 5.5.4 of the TGD for WQMP).

<b>Form 4.3-4 Harvest and Use BMPs (DA 1-2)</b>			
<b>1</b> Remaining LID DCV not met by site design HSC or infiltration BMP (ft <sup>3</sup> ): 0 <i>V<sub>unmet</sub> = Form 4.2-1 Item 7 - Form 4.3-2 Item 30 - Form 4.3-3 Item 16</i>			
BMP Type(s) <i>Compute runoff volume retention from proposed harvest and use BMP (Select BMPs from Table 5-4 of the TGD for WQMP) - Use additional forms for more BMPs</i>	DA DMA BMP Type	DA DMA BMP Type	DA DMA BMP Type <i>(Use additional forms for more BMPs)</i>
<b>2</b> Describe cistern or runoff detention facility			
<b>3</b> Storage volume for proposed detention type (ft <sup>3</sup> ) <i>Volume of cistern</i>			
<b>4</b> Landscaped area planned for use of harvested stormwater (ft <sup>2</sup> )			
<b>5</b> Average wet season daily irrigation demand (in/day) <i>Use local values, typical ~ 0.1 in/day</i>			
<b>6</b> Daily water demand (ft <sup>3</sup> /day) <i>Item 4 * (Item 5 / 12)</i>			
<b>7</b> Drawdown time (hrs) <i>Copy Item 6 from Form 4.2-1</i>			
<b>8</b> Retention Volume (ft <sup>3</sup> ) <i>V<sub>retention</sub> = Minimum of (Item 3) or (Item 6 * (Item 7 / 24))</i>	0	0	0
<b>9</b> Total Retention Volume (ft <sup>3</sup> ) from Harvest and Use BMP: 0 <i>Sum of Item 8 for all harvest and use BMP included in plan</i>			
<b>10</b> Is the full DCV retained with a combination of LID HSC, retention and infiltration, and harvest and use BMPs? Yes <input type="checkbox"/> No <input type="checkbox"/> <i>If yes, demonstrate conformance using Form 4.3-10. If no, then re-evaluate combinations of all LID BMP and optimize their implementation such that the maximum portion of the DCV is retained on-site (using a single BMP type or combination of BMP types). If the full DCV cannot be mitigated after this optimization process, proceed to Section 4.3.4.</i>			



### 4.3.4 Biotreatment BMP

Biotreatment BMPs may be considered if the full LID DCV cannot be met by maximizing retention and infiltration, and harvest and use BMPs. A key consideration when using biotreatment BMP is the effectiveness of the proposed BMP in addressing the pollutants of concern for the project (see Table 5-5 of the TGD for WQMP).

Use Form 4.3-5 to summarize the potential for volume based and/or flow based biotreatment options to biotreat the remaining unmet LID DCV w. Biotreatment computations are included as follows:

- Use Form 4.3-6 to compute biotreatment in small volume based biotreatment BMP (e.g. bioretention w/underdrains);
- Use Form 4.3-7 to compute biotreatment in large volume based biotreatment BMP (e.g. constructed wetlands);
- Use Form 4.3-8 to compute sizing criteria for flow-based biotreatment BMP (e.g. bioswales)

<b>Form 4.3-5 Selection and Evaluation of Biotreatment BMP (DA 1-2)</b>		
<b>1</b> Remaining LID DCV not met by site design HSC, infiltration, or harvest and use BMP for potential biotreatment (ft <sup>3</sup> ): 0 Form 4.2-1 Item 7 - Form 4.3-2 Item 30 – Form 4.3-3 Item 16- Form 4.3-4 Item 9		List pollutants of concern Copy from Form 2.3-1.
<b>2</b> Biotreatment BMP Selected  (Select biotreatment BMP(s) necessary to ensure all pollutants of concern are addressed through Unit Operations and Processes, described in Table 5-5 of the TGD for WQMP)	<b>Volume-based biotreatment</b> Use Forms 4.3-6 and 4.3-7 to compute treated volume	<b>Flow-based biotreatment</b> Use Form 4.3-8 to compute treated volume
	<input type="checkbox"/> Bioretention with underdrain <input type="checkbox"/> Planter box with underdrain <input type="checkbox"/> Constructed wetlands <input type="checkbox"/> Wet extended detention <input type="checkbox"/> Dry extended detention	<input type="checkbox"/> Vegetated swale <input type="checkbox"/> Vegetated filter strip <input type="checkbox"/> Proprietary biotreatment
<b>3</b> Volume biotreated in volume based biotreatment BMP (ft <sup>3</sup> ): 0 Form 4.3-6 Item 15 + Form 4.3-7 Item 13	<b>4</b> Compute remaining LID DCV with implementation of volume based biotreatment BMP (ft <sup>3</sup> ): 0 Item 1 – Item 3	<b>5</b> Remaining fraction of LID DCV for sizing flow based biotreatment BMP: % Item 4 / Item 1
<b>6</b> Flow-based biotreatment BMP capacity provided (cfs): Use Figure 5-2 of the TGD for WQMP to determine flow capacity required to provide biotreatment of remaining percentage of unmet LID DCV (Item 5), for the project’s precipitation zone (Form 3-1 Item 1)		
<b>7</b> Metrics for MEP determination: <ul style="list-style-type: none"> <li>• Provided a WQMP with the portion of site area used for suite of LID BMP equal to minimum thresholds in Table 5-7 of the TGD for WQMP for the proposed category of development: <input type="checkbox"/> If maximized on-site retention BMPs is feasible for partial capture, then LID BMP implementation must be optimized to retain and infiltrate the maximum portion of the DCV possible within the prescribed minimum effective area. The remaining portion of the DCV shall then be mitigated using biotreatment BMP.</li> </ul>		

<b>Form 4.3-6 Volume Based Biotreatment (DA 1-2) – Bioretention and Planter Boxes with Underdrains</b>			
Biotreatment BMP Type <i>(Bioretention w/underdrain, planter box w/underdrain, other comparable BMP)</i>	DA DMA BMP Type	DA DMA BMP Type	DA DMA BMP Type <i>(Use additional forms for more BMPs)</i>
<b>1</b> Pollutants addressed with BMP <i>List all pollutant of concern that will be effectively reduced through specific Unit Operations and Processes described in Table 5-5 of the TGD for WQMP</i>			
<b>2</b> Amended soil infiltration rate <i>Typical ~ 5.0</i>			
<b>3</b> Amended soil infiltration safety factor <i>Typical ~ 2.0</i>			
<b>4</b> Amended soil design percolation rate (in/hr) $P_{design} = \text{Item 2} / \text{Item 3}$			
<b>5</b> Ponded water drawdown time (hr) <i>Copy Item 6 from Form 4.2-1</i>			
<b>6</b> Maximum ponding depth (ft) <i>see Table 5-6 of the TGD for WQMP for reference to BMP design details</i>			
<b>7</b> Ponding Depth (ft) $d_{BMP} = \text{Minimum of } (1/12 * \text{Item 4} * \text{Item 5}) \text{ or Item 6}$			
<b>8</b> Amended soil surface area (ft <sup>2</sup> )			
<b>9</b> Amended soil depth (ft) <i>see Table 5-6 of the TGD for WQMP for reference to BMP design details</i>			
<b>10</b> Amended soil porosity, <i>n</i>			
<b>11</b> Gravel depth (ft) <i>see Table 5-6 of the TGD for WQMP for reference to BMP design details</i>			
<b>12</b> Gravel porosity, <i>n</i>			
<b>13</b> Duration of storm as basin is filling (hrs) <i>Typical ~ 3hrs</i>			
<b>14</b> Biotreated Volume (ft <sup>3</sup> ) $V_{biotreated} = \text{Item 8} * [(\text{Item 7}/2) + (\text{Item 9} * \text{Item 10}) + (\text{Item 11} * \text{Item 12}) + (\text{Item 13} * (\text{Item 4} / 12))]$	0	0	0
<b>15</b> Total biotreated volume from bioretention and/or planter box with underdrains BMP: 0 <i>Sum of Item 14 for all volume-based BMPs included in this form</i>			

## Form 4.3-7 Volume Based Biotreatment (DA 1-2) – Constructed Wetlands and Extended Detention

Biotreatment BMP Type <i>Constructed wetlands, extended wet detention, extended dry detention, or other comparable proprietary BMP. If BMP includes multiple modules (e.g. forebay and main basin), provide separate estimates for storage and pollutants treated in each module.</i>	DA    DMA BMP Type		DA    DMA BMP Type <i>(Use additional forms for more BMPs)</i>	
	Forebay	Basin	Forebay	Basin
<b>1</b> Pollutants addressed with BMP forebay and basin <i>List all pollutant of concern that will be effectively reduced through specific Unit Operations and Processes described in Table 5-5 of the TGD for WQMP</i>				
<b>2</b> Bottom width (ft)				
<b>3</b> Bottom length (ft)				
<b>4</b> Bottom area (ft <sup>2</sup> ) <i>A<sub>bottom</sub> = Item 2 * Item 3</i>				
<b>5</b> Side slope (ft/ft)				
<b>6</b> Depth of storage (ft)				
<b>7</b> Water surface area (ft <sup>2</sup> ) <i>A<sub>surface</sub> = (Item 2 + (2 * Item 5 * Item 6)) * (Item 3 + (2 * Item 5 * Item 6))</i>				
<b>8</b> Storage volume (ft <sup>3</sup> ) <i>For BMP with a forebay, ensure fraction of total storage is within ranges specified in BMP specific fact sheets, see Table 5-6 of the TGD for WQMP for reference to BMP design details</i> <i>V = Item 6 / 3 * [Item 4 + Item 7 + (Item 4 * Item 7)<sup>0.5</sup>]</i>				
<b>9</b> Drawdown Time (hrs) <i>Copy Item 6 from Form 2.1</i>				
<b>10</b> Outflow rate (cfs) <i>Q<sub>BMP</sub> = (Item 8<sub>forebay</sub> + Item 8<sub>basin</sub>) / (Item 9 * 3600)</i>				
<b>11</b> Duration of design storm event (hrs)				
<b>12</b> Biotreated Volume (ft <sup>3</sup> ) <i>V<sub>biotreated</sub> = (Item 8<sub>forebay</sub> + Item 8<sub>basin</sub>) + (Item 10 * Item 11 * 3600)</i>	0		0	
<b>13</b> Total biotreated volume from constructed wetlands, extended dry detention, or extended wet detention : 0 <i>(Sum of Item 12 for all BMP included in plan)</i>				

<b>Form 4.3-8 Flow Based Biotreatment (DA 1-2)</b>			
Biotreatment BMP Type <i>Vegetated swale, vegetated filter strip, or other comparable proprietary BMP</i>	DA DMA BMP Type	DA DMA BMP Type	DA DMA BMP Type <i>(Use additional forms for more BMPs)</i>
<b>1</b> Pollutants addressed with BMP <i>List all pollutant of concern that will be effectively reduced through specific Unit Operations and Processes described in TGD Table 5-5</i>			
<b>2</b> Flow depth for water quality treatment (ft) <i>BMP specific, see Table 5-6 of the TGD for WQMP for reference to BMP design details</i>			
<b>3</b> Bed slope (ft/ft) <i>BMP specific, see Table 5-6 of the TGD for WQMP for reference to BMP design details</i>			
<b>4</b> Manning's roughness coefficient			
<b>5</b> Bottom width (ft) <i><math>b_w = (\text{Form 4.3-5 Item 6} * \text{Item 4}) / (1.49 * \text{Item 2}^{1.67} * \text{Item 3}^{0.5})</math></i>			
<b>6</b> Side Slope (ft/ft) <i>BMP specific, see Table 5-6 of the TGD for WQMP for reference to BMP design details</i>			
<b>7</b> Cross sectional area (ft <sup>2</sup> ) <i><math>A = (\text{Item 5} * \text{Item 2}) + (\text{Item 6} * \text{Item 2}^{0.2})</math></i>			
<b>8</b> Water quality flow velocity (ft/sec) <i><math>V = \text{Form 4.3-5 Item 6} / \text{Item 7}</math></i>			
<b>9</b> Hydraulic residence time (min) <i>Pollutant specific, see Table 5-6 of the TGD for WQMP for reference to BMP design details</i>			
<b>10</b> Length of flow based BMP (ft) <i><math>L = \text{Item 8} * \text{Item 9} * 60</math></i>			
<b>11</b> Water surface area at water quality flow depth (ft <sup>2</sup> ) <i><math>SA_{top} = (\text{Item 5} + (2 * \text{Item 2} * \text{Item 6})) * \text{Item 10}</math></i>			

### 4.3.5 Conformance Summary

Complete Form 4.3-9 to demonstrate how on-site LID DCV is met with proposed site design hydrologic source control, infiltration, harvest and use, and/or biotreatment BMP. The bottom line of the form is used to describe the basis for infeasibility determination for on-site LID BMP to achieve full LID DCV, and provides methods for computing remaining volume to be addressed in an alternative compliance plan. If the project has more than one outlet, then complete additional versions of this form for each outlet.

<b>Form 4.3-9 Conformance Summary and Alternative Compliance Volume Estimate (DA 1)</b>	
<b>1</b>	Total LID DCV for the Project DA-1 (ft <sup>3</sup> ): 18,451 <i>Copy Item 7 in Form 4.2-1</i>
<b>2</b>	On-site retention with site design hydrologic source control LID BMP (ft <sup>3</sup> ): 0 <i>Copy Item 30 in Form 4.3-2</i>
<b>3</b>	On-site retention with LID infiltration BMP (ft <sup>3</sup> ): 18,897 <i>Copy Item 16 in Form 4.3-3</i>
<b>4</b>	On-site retention with LID harvest and use BMP (ft <sup>3</sup> ): 0 <i>Copy Item 9 in Form 4.3-4</i>
<b>5</b>	On-site biotreatment with volume based biotreatment BMP (ft <sup>3</sup> ): 0 <i>Copy Item 3 in Form 4.3-5</i>
<b>6</b>	Flow capacity provided by flow based biotreatment BMP (cfs): 0 <i>Copy Item 6 in Form 4.3-5</i>
<b>7</b>	<p>LID BMP performance criteria are achieved if answer to any of the following is "Yes":</p> <ul style="list-style-type: none"> <li>• Full retention of LID DCV with site design HSC, infiltration, or harvest and use BMP: Yes <input checked="" type="checkbox"/> No <input type="checkbox"/> <i>If yes, sum of Items 2, 3, and 4 is greater than Item 1</i></li> <li>• Combination of on-site retention BMPs for a portion of the LID DCV and volume-based biotreatment BMP that address all pollutants of concern for the remaining LID DCV: Yes <input type="checkbox"/> No <input type="checkbox"/> <i>If yes, a) sum of Items 2, 3, 4, and 5 is greater than Item 1, and Items 2, 3 and 4 are maximized; or b) Item 6 is greater than Form 4.3--5 Item 6 and Items 2, 3 and 4 are maximized</i></li> <li>▪ On-site retention and infiltration is determined to be infeasible and biotreatment BMP provide biotreatment for all pollutants of concern for full LID DCV: Yes <input type="checkbox"/> No <input type="checkbox"/> <i>If yes, Form 4.3-1 Items 7 and 8 were both checked yes</i></li> </ul>
<b>8</b>	<p>If the LID DCV is not achieved by any of these means, then the project may be allowed to develop an alternative compliance plan. Check box that describes the scenario which caused the need for alternative compliance:</p> <ul style="list-style-type: none"> <li>• Combination of HSC, retention and infiltration, harvest and use, and biotreatment BMPs provide less than full LID DCV capture: <input type="checkbox"/> <i>Checked yes for Form 4.3-5 Item 7, Item 6 is zero, and sum of Items 2, 3, 4, and 5 is less than Item 1. If so, apply water quality credits and calculate volume for alternative compliance, <math>V_{alt} = (Item\ 1 - Item\ 2 - Item\ 3 - Item\ 4 - Item\ 5) * (100 - Form\ 2.4-1\ Item\ 2)\%</math></i></li> <li>• An approved Watershed Action Plan (WAP) demonstrates that water quality and hydrologic impacts of urbanization are more effective when managed in at an off-site facility: <input type="checkbox"/> <i>Attach appropriate WAP section, including technical documentation, showing effectiveness comparisons for the project site and regional watershed</i></li> </ul>

## Form 4.3-9 Conformance Summary and Alternative Compliance Volume Estimate (DA 2)

**1** Total LID DCV for the Project DA-2 (ft<sup>3</sup>): 140 *Copy Item 7 in Form 4.2-1*

**2** On-site retention with site design hydrologic source control LID BMP (ft<sup>3</sup>): 0 *Copy Item 30 in Form 4.3-2*

**3** On-site retention with LID infiltration BMP (ft<sup>3</sup>): 6,537 *Copy Item 16 in Form 4.3-3*

**4** On-site retention with LID harvest and use BMP (ft<sup>3</sup>): 0 *Copy Item 9 in Form 4.3-4*

**5** On-site biotreatment with volume based biotreatment BMP (ft<sup>3</sup>): 0 *Copy Item 3 in Form 4.3-5*

**6** Flow capacity provided by flow based biotreatment BMP (cfs): 0 *Copy Item 6 in Form 4.3-5*

**7** LID BMP performance criteria are achieved if answer to any of the following is "Yes":

- Full retention of LID DCV with site design HSC, infiltration, or harvest and use BMP: Yes  No   
*If yes, sum of Items 2, 3, and 4 is greater than Item 1*
- Combination of on-site retention BMPs for a portion of the LID DCV and volume-based biotreatment BMP that address all pollutants of concern for the remaining LID DCV: Yes  No   
*If yes, a) sum of Items 2, 3, 4, and 5 is greater than Item 1, and Items 2, 3 and 4 are maximized; or b) Item 6 is greater than Form 4.3-5 Item 6 and Items 2, 3 and 4 are maximized*
- On-site retention and infiltration is determined to be infeasible and biotreatment BMP provide biotreatment for all pollutants of concern for full LID DCV: Yes  No   
*If yes, Form 4.3-1 Items 7 and 8 were both checked yes*

**8** If the LID DCV is not achieved by any of these means, then the project may be allowed to develop an alternative compliance plan. Check box that describes the scenario which caused the need for alternative compliance:

- Combination of HSC, retention and infiltration, harvest and use, and biotreatment BMPs provide less than full LID DCV capture:   
*Checked yes for Form 4.3-5 Item 7, Item 6 is zero, and sum of Items 2, 3, 4, and 5 is less than Item 1. If so, apply water quality credits and calculate volume for alternative compliance,  $V_{alt} = (Item\ 1 - Item\ 2 - Item\ 3 - Item\ 4 - Item\ 5) * (100 - Form\ 2.4-1\ Item\ 2)\%$*
- An approved Watershed Action Plan (WAP) demonstrates that water quality and hydrologic impacts of urbanization are more effective when managed in at an off-site facility:   
*Attach appropriate WAP section, including technical documentation, showing effectiveness comparisons for the project site and regional watershed*

### 4.3.6 Hydromodification Control BMP

Use Form 4.3-10 to compute the remaining runoff volume retention, after LID BMP are implemented, needed to address HCOC, and the increase in time of concentration and decrease in peak runoff necessary to meet targets for protection of waterbodies with a potential HCOC. Describe hydromodification control BMP that address HCOC, which may include off-site BMP and/or in-stream controls. Section 5.6 of the TGD for WQMP provides additional details on selection and evaluation of hydromodification control BMP.

Form 4.3-10 Hydromodification Control BMPs (DA 1-2)	
<p><b>1</b> Volume reduction needed for HCOC performance criteria (ft<sup>3</sup>): 0 (Form 4.2-2 Item 4 * 0.95) – Form 4.2-2 Item 1</p>	<p><b>2</b> On-site retention with site design hydrologic source control, infiltration, and harvest and use LID BMP (ft<sup>3</sup>): <i>Sum of Form 4.3-9 Items 2, 3, and 4 Evaluate option to increase implementation of on-site retention in Forms 4.3-2, 4.3-3, and 4.3-4 in excess of LID DCV toward achieving HCOC volume reduction</i></p>
<p><b>3</b> Remaining volume for HCOC volume capture (ft<sup>3</sup>): <i>Item 1 – Item 2</i></p>	<p><b>4</b> Volume capture provided by incorporating additional on-site or off-site retention BMPs (ft<sup>3</sup>): <i>Existing downstream BMP may be used to demonstrate additional volume capture (if so, attach to this WQMP a hydrologic analysis showing how the additional volume would be retained during a 2-yr storm event for the regional watershed)</i></p>
<p><b>5</b> If Item 4 is less than Item 3, incorporate in-stream controls on downstream waterbody segment to prevent impacts due to hydromodification <input type="checkbox"/> <i>Attach in-stream control BMP selection and evaluation to this WQMP</i></p>	
<p><b>6</b> Is Form 4.2-2 Item 11 less than or equal to 5%: Yes <input type="checkbox"/> No <input type="checkbox"/>  <i>If yes, HCOC performance criteria is achieved. If no, select one or more mitigation options below:</i></p> <ul style="list-style-type: none"> <li>• Demonstrate increase in time of concentration achieved by proposed LID site design, LID BMP, and additional on-site or off-site retention BMP <input type="checkbox"/>  <i>BMP upstream of a waterbody segment with a potential HCOC may be used to demonstrate increased time of concentration through hydrograph attenuation (if so, show that the hydraulic residence time provided in BMP for a 2-year storm event is equal or greater than the addition time of concentration requirement in Form 4.2-4 Item 15)</i></li> <li>• Increase time of concentration by preserving pre-developed flow path and/or increase travel time by reducing slope and increasing cross-sectional area and roughness for proposed on-site conveyance facilities <input type="checkbox"/></li> <li>• Incorporate appropriate in-stream controls for downstream waterbody segment to prevent impacts due to hydromodification, in a plan approved and signed by a licensed engineer in the State of California <input type="checkbox"/></li> </ul>	
<p><b>7</b> Form 4.2-2 Item 12 less than or equal to 5%: Yes <input type="checkbox"/> No <input type="checkbox"/>  <i>If yes, HCOC performance criteria is achieved. If no, select one or more mitigation options below:</i></p> <ul style="list-style-type: none"> <li>• Demonstrate reduction in peak runoff achieved by proposed LID site design, LID BMPs, and additional on-site or off-site retention BMPs <input type="checkbox"/>  <i>BMPs upstream of a waterbody segment with a potential HCOC may be used to demonstrate additional peak runoff reduction through hydrograph attenuation (if so, attach to this WQMP, a hydrograph analysis showing how the peak runoff would be reduced during a 2-yr storm event)</i></li> <li>• Incorporate appropriate in-stream controls for downstream waterbody segment to prevent impacts due to hydromodification, in a plan approved and signed by a licensed engineer in the State of California <input type="checkbox"/></li> </ul>	

## 4.4 Alternative Compliance Plan (if applicable)

Describe an alternative compliance plan (if applicable) for projects not fully able to infiltrate, harvest and use, or biotreat the DCV via on-site LID practices. A project proponent must develop an alternative compliance plan to address the remainder of the LID DCV. Depending on project type some projects may qualify for water quality credits that can be applied to reduce the DCV that must be treated prior to development of an alternative compliance plan (see Form 2.4-1, Water Quality Credits). Form 4.3-9 Item 8 includes instructions on how to apply water quality credits when computing the DCV that must be met through alternative compliance. Alternative compliance plans may include one or more of the following elements:

- On-site structural treatment control BMP - All treatment control BMP should be located as close to possible to the pollutant sources and should not be located within receiving waters;
- Off-site structural treatment control BMP - Pollutant removal should occur prior to discharge of runoff to receiving waters;
- Urban runoff fund or In-lieu program, if available

Depending upon the proposed alternative compliance plan, approval by the executive officer may or may not be required (see Section 6 of the TGD for WQMP).



## Section 5 Inspection and Maintenance Responsibility for Post Construction BMP

All BMP included as part of the project WQMP are required to be maintained through regular scheduled inspection and maintenance (refer to Section 8, Post Construction BMP Requirements, in the TGD for WQMP). Fully complete Form 5-1 summarizing all BMP included in the WQMP. Attach additional forms as needed. The WQMP shall also include a detailed Operation and Maintenance Plan for all BMP and may require a Maintenance Agreement (consult the jurisdiction's LIP). If a Maintenance Agreement is required, it must also be attached to the WQMP.

<b>Form 5-1 BMP Inspection and Maintenance (use additional forms as necessary)</b>			
BMP	Responsible Party(s)	Inspection/Maintenance Activities Required	Minimum Frequency of Activities
Education of Property Owners, Tenants & Occupants on Stormwater BMPs	Property Owner	The Property Owner will provide BMP educational information materials to all occupants of site.	Annually
Activity Restrictions	Property Owner	Inspect for proper site usage and pesticide application.	Daily
Landscape Management BMPs	Property Owner	Owner will ensure landscaping and irrigation is properly maintained.	Bi-weekly
Underground Chambers	Property Owner	Inspect for trash, debris, sediment and damage. Clean as needed and repair per manufacturer's recommendations.	Annually (prior to October 1st) and after major storm event
Infiltration Basin	Property Owner	Inspect for trash, debris, sediment and damage. Clean trash, debris, sediment and repair damage as needed.	Annually (prior to October 1st) and after major storm events
Litter/Debris Control Program	Property Owner	Inspect and clean site for trash and debris	Weekly
Employee Training	Property Owner	Educational materials on general housekeeping practices for the protection of storm water quality shall be provided to employees.	Within 3 months of hire and annually thereafter

**Water Quality Management Plan (WQMP)**

Catch Basin Inserts	Property Owner	Inspect for trash, debris and damage	Monthly
Vacuum Sweeping	Property Owner	Parking lots shall be swept and vacuumed	Monthly
Underground Chambers	Property Owner	Inspect for trash, debris, sediment and damage. Clean as needed and repair per manufacturer's recommendations.	Annually (prior to October 1st) and after major storm events
Provide storm drain system stenciling and signage	Property Owner	Inspect storm drain system stenciling and signage for clarity and legibility. Relabel as needed.	Annually, repair as needed
Trash Storage Area	Property Owner	Inspect, clean, and repair as needed.	Bi-weekly
Use Efficient Irrigation System and Landscape Design	Property Owner	Install irrigation systems with timing devices to avoid overwatering. Repair as needed	Bi-weekly
Finish grade of landscaped areas at a minimum of 1-2 inches below top of curb, sidewalk, or pavement	Property Owner	Landscape areas will be a minimum of 1 inch below adjacent impervious areas.	Bi-weekly

## Section 6 WQMP Attachments

### 6.1. Site Plan and Drainage Plan

Include a site plan and drainage plan sheet set containing the following minimum information:

- Project location
- Site boundary
- Land uses and land covers, as applicable
- Suitability/feasibility constraints
- Structural Source Control BMP locations
- Site Design Hydrologic Source Control BMP locations
- LID BMP details
- Drainage delineations and flow information
- Drainage connections

### 6.2 Electronic Data Submittal

Minimum requirements include submittal of PDF exhibits in addition to hard copies. Format must not require specialized software to open. If the local jurisdiction requires specialized electronic document formats (as described in their local Local Implementation Plan), this section will describe the contents (e.g., layering, nomenclature, geo-referencing, etc.) of these documents so that they may be interpreted efficiently and accurately.

### 6.3 Post Construction

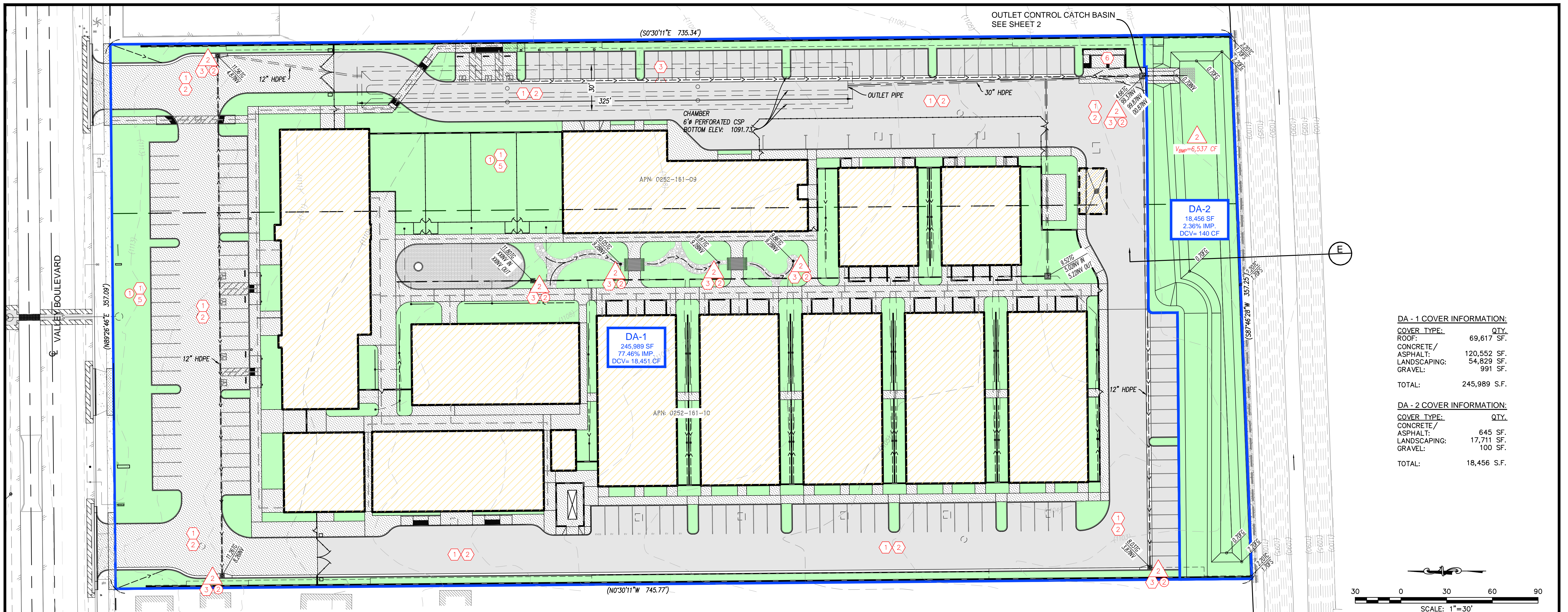
Attach all O&M Plans and Maintenance Agreements for BMP to the WQMP.

- O&M Plan
  - BMP Educational Materials
- Maintenance Agreement(s)
- Activity Restriction – C, C&R's & Lease Agreements

### 6.4 Other Supporting Documentation

- San Bernardino County Watershed Mapping Tool Data
- NOAA Rainfall Data
- Soils information

# Appendix 6.1 – Site Plan and Drainage Plan

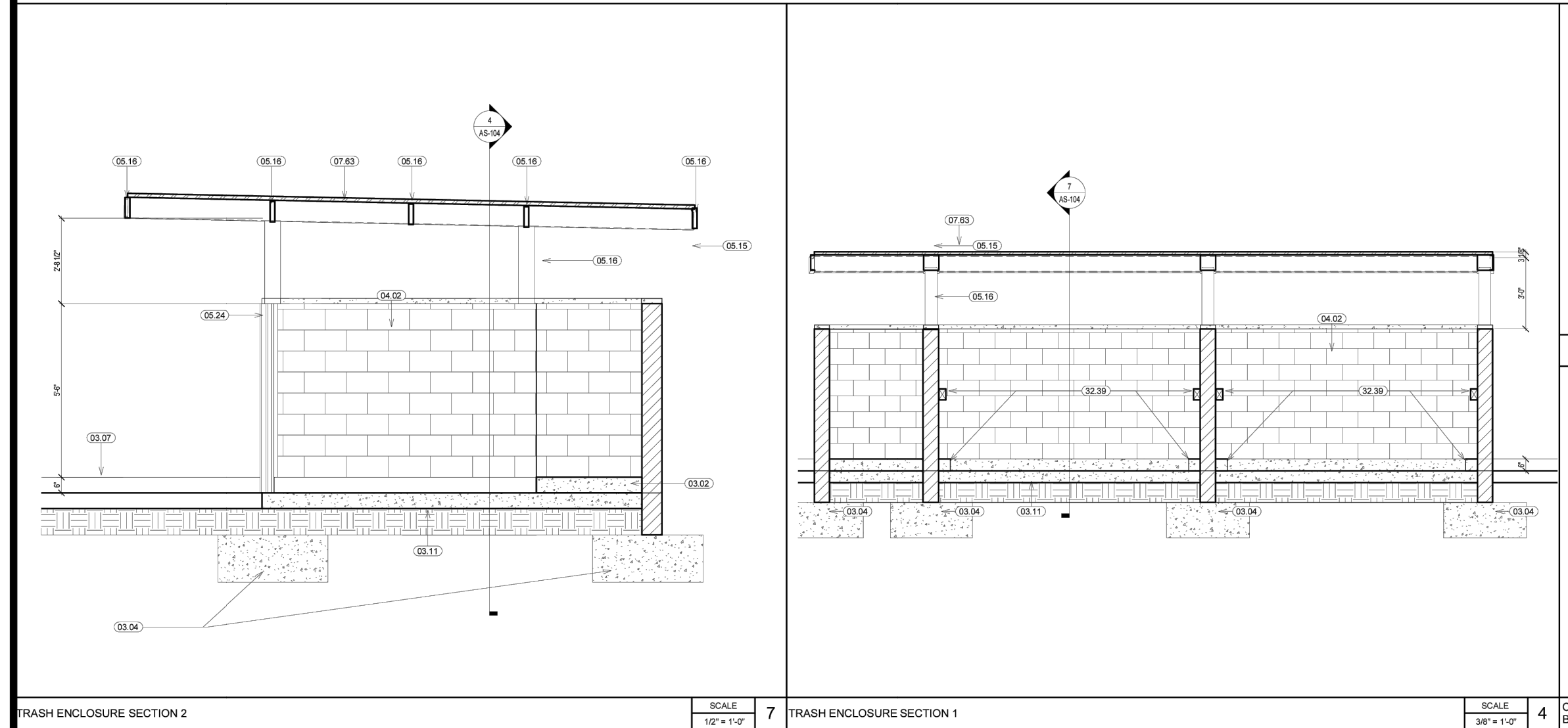
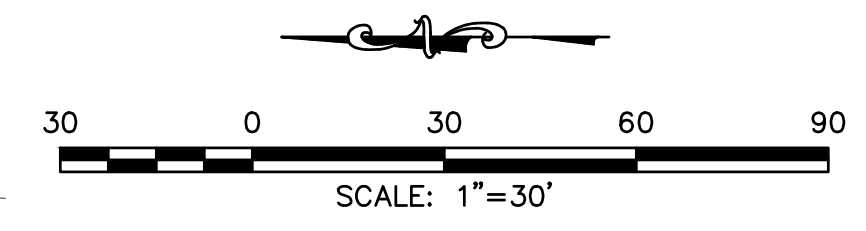


**DA - 1 COVER INFORMATION:**

COVER TYPE:	QTY:
ROOF:	69,617 SF.
CONCRETE/ ASPHALT:	120,552 SF.
LANDSCAPING:	54,829 SF.
GRAVEL:	991 SF.
<b>TOTAL:</b>	<b>245,989 S.F.</b>

**DA - 2 COVER INFORMATION:**

COVER TYPE:	QTY:
CONCRETE/ ASPHALT:	645 SF.
LANDSCAPING:	17,711 SF.
GRAVEL:	100 SF.
<b>TOTAL:</b>	<b>18,456 S.F.</b>



- W.Q.M.P. NOTES:**
- STRUCTURAL BMPs MAY BE SUBSTITUTED WITH EQUIVALENT PRODUCTS AT THE DISCRETION OF THE CONTRACTOR AND WITH APPROVAL FROM THE ENGINEER AND COUNTY OF SAN BERNARDINO BASED ON AVAILABILITY AT TIME OF CONSTRUCTION.
  - TREATMENT CONTROL BMPs MAY BE SUBSTITUTED WITH EQUIVALENT PRODUCTS BASED ON AVAILABILITY AND WITH APPROVAL FROM THE ENGINEER AND COUNTY OF SAN BERNARDINO AT TIME OF CONSTRUCTION AND AS LONG AS THE MINIMUM DCV IS RETAINED AND MITIGATED AS INTENDED IN THE PROJECT SPECIFIC WQMP.

- PROPOSED SITE DESIGN BMP'S:**
- ① SITE DESIGN & LANDSCAPE PLANNING (SD-10) [S13]
  - ① EFFICIENT IRRIGATION (SD-12) [S4]
  - ② STORM DRAIN SIGNAGE (SD-13) [S1]
- PROPOSED SOURCE CONTROL BMP'S:**
- ① SPILL PREVENTION, CONTROL & CLEANUP (SC-11) [N7]
  - ① WASTE HANDLING & DISPOSAL (SC-34)
  - ① GROUNDS MAINTENANCE (SC-41)
  - ② PARKING AREA MAINTENANCE (SC-43) [N15]
  - ③ DRAINAGE SYSTEM MAINTENANCE (SC-44)
  - ① HOUSEKEEPING PRACTICES (SC-60) [N11]
  - ⑤ LANDSCAPE MAINTENANCE (SC-73) [N3]
  - ⑥ TRASH STORAGE AREAS (SD-32)

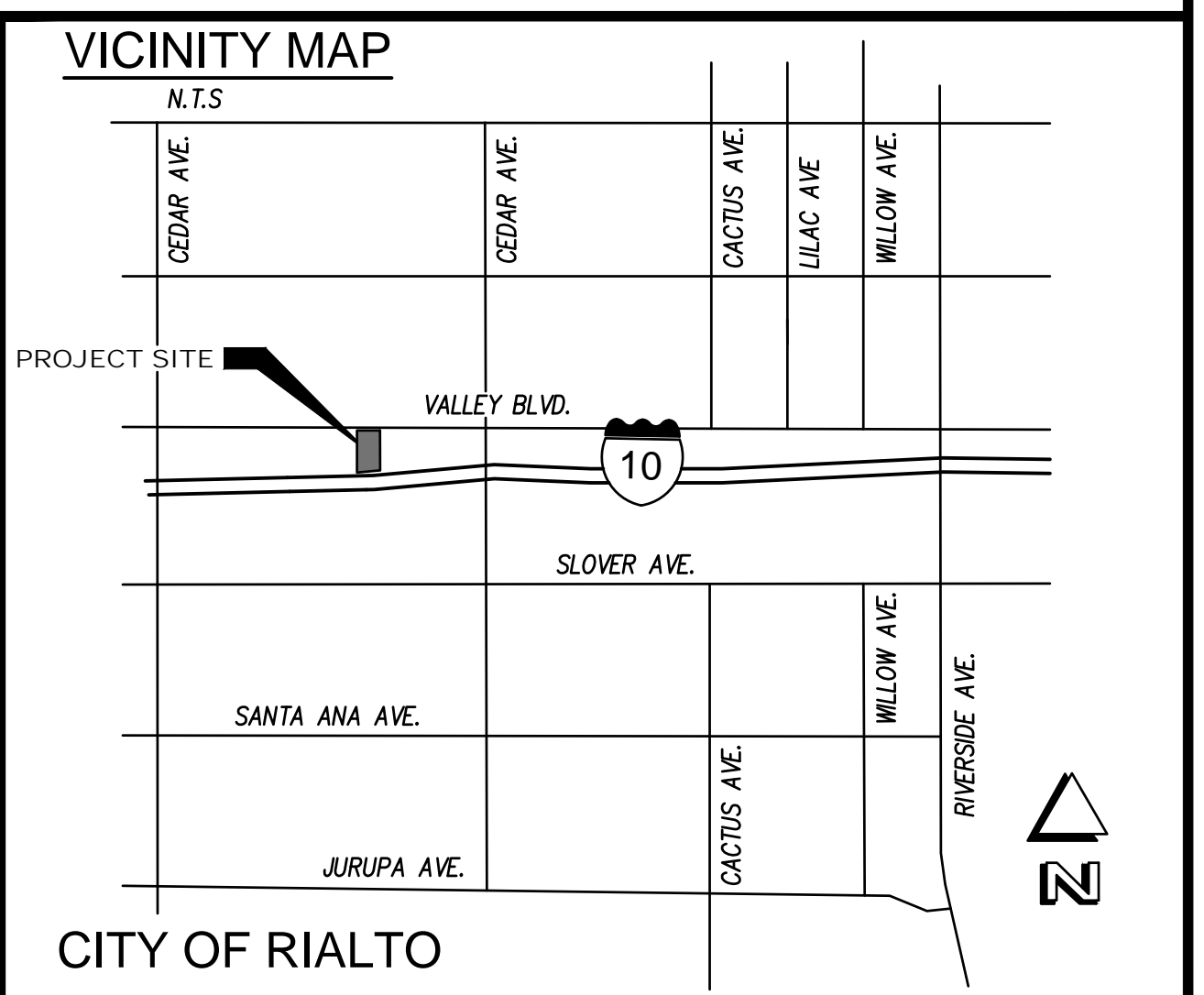
- PROPOSED TREATMENT CONTROL BMP'S:**
- ① UNDERGROUND INFILTRATION SYSTEM (TC-11) [V<sub>BMP</sub> = 15,803 CF]
  - ② DRAIN INSERTS (MP-52)
  - ③ INFILTRATION BASIN (TC-11) [V<sub>BMP</sub> = \*\*CF]

**OWNER/APPLICANT:**  
 SAN BERNARDINO COUNTY  
 385 N. ARROWHEAD AVE., 3<sup>RD</sup> FLOOR  
 SAN BERNARDINO, CA 92415  
 ATTN: KENNETH HYLIN, SR. PROJECT MANAGER  
 PHONE: (909) 387-5000  
 EMAIL: KENNETH.HYLIN@PFM.SBCOUNTY.GOV

**PARCEL COVER AGE:**

TYPE:	AREA:	%
BUILDING:	69,617 SF	26.33%
HARDSCAPE:	122,288 SF	46.24%
LANDSCAPE:	72,540 SF	27.43%
<b>TOTAL:</b>	<b>264,445 SF</b>	<b>100%</b>

- LEGEND:**
- W.Q.M.P. DRAINAGE AREA/PROPERTY LIMITS
  - PROPOSED LANDSCAPING AREAS
  - PROPOSED CONCRETE PAVING AREAS
  - PROPOSED A/C PAVING AREAS
  - PROPOSED STRUCTURES
  - PROPOSED GRAVEL



**W.Q.M.P.**  
 APN(S): 0252-161-09 & 10  
 CITY OF RIALTO, CA



PREPARED FOR: SAN BERNARDINO COUNTY

DRAWN BY: SG	SCALE: 1" = 30'	SHEET: 1 OF 2
CHECKED BY: JTS	JOB NO: 225038	
DISREGARD PRINTS BEARING EARLIER REVISION DATES		09-26-24



## **Appendix 6.2 – Electronic Data Submittal**

Note: This WQMP was submitted digitally, in PDF format, per reviewing agency requirements. There is no CD attachment included.



## **Appendix 6.3 – Post Construction**



## Description

As a consequence of its function, the stormwater conveyance system collects and transports urban runoff and stormwater that may contain certain pollutants. The protocols in this fact sheet are intended to reduce pollutants reaching receiving waters through proper conveyance system operation and maintenance.

## Approach

### *Pollution Prevention*

Maintain catch basins, stormwater inlets, and other stormwater conveyance structures on a regular basis to remove pollutants, reduce high pollutant concentrations during the first flush of storms, prevent clogging of the downstream conveyance system, restore catch basins' sediment trapping capacity, and ensure the system functions properly hydraulically to avoid flooding.

### *Suggested Protocols*

#### *Catch Basins/Inlet Structures*

- Staff should regularly inspect facilities to ensure compliance with the following:
  - Immediate repair of any deterioration threatening structural integrity.
  - Cleaning before the sump is 40% full. Catch basins should be cleaned as frequently as needed to meet this standard.
  - Stenciling of catch basins and inlets (see SC34 Waste Handling and Disposal).

## Objectives

- Cover
- Contain
- Educate
- Reduce/Minimize

## Targeted Constituents

Sediment	✓
Nutrients	
Trash	✓
Metals	
Bacteria	✓
Oil and Grease	
Organics	



# Site Design & Landscape Planning SD-10



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## Design Objectives

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- Maximize Infiltration
  - Provide Retention
  - Slow Runoff
  - Minimize Impervious Land Coverage
  - Prohibit Dumping of Improper Materials
  - Contain Pollutants
  - Collect and Convey
- 

## Description

Each project site possesses unique topographic, hydrologic, and vegetative features, some of which are more suitable for development than others. Integrating and incorporating appropriate landscape planning methodologies into the project design is the most effective action that can be done to minimize surface and groundwater contamination from stormwater.

## Approach

Landscape planning should couple consideration of land suitability for urban uses with consideration of community goals and projected growth. Project plan designs should conserve natural areas to the extent possible, maximize natural water storage and infiltration opportunities, and protect slopes and channels.

## Suitable Applications

Appropriate applications include residential, commercial and industrial areas planned for development or redevelopment.

## Design Considerations

Design requirements for site design and landscapes planning should conform to applicable standards and specifications of agencies with jurisdiction and be consistent with applicable General Plan and Local Area Plan policies.



# SD-10 Site Design & Landscape Planning

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## *Designing New Installations*

Begin the development of a plan for the landscape unit with attention to the following general principles:

- Formulate the plan on the basis of clearly articulated community goals. Carefully identify conflicts and choices between retaining and protecting desired resources and community growth.
- Map and assess land suitability for urban uses. Include the following landscape features in the assessment: wooded land, open unwooded land, steep slopes, erosion-prone soils, foundation suitability, soil suitability for waste disposal, aquifers, aquifer recharge areas, wetlands, floodplains, surface waters, agricultural lands, and various categories of urban land use. When appropriate, the assessment can highlight outstanding local or regional resources that the community determines should be protected (e.g., a scenic area, recreational area, threatened species habitat, farmland, fish run). Mapping and assessment should recognize not only these resources but also additional areas needed for their sustenance.

Project plan designs should conserve natural areas to the extent possible, maximize natural water storage and infiltration opportunities, and protect slopes and channels.

## *Conserve Natural Areas during Landscape Planning*

If applicable, the following items are required and must be implemented in the site layout during the subdivision design and approval process, consistent with applicable General Plan and Local Area Plan policies:

- Cluster development on least-sensitive portions of a site while leaving the remaining land in a natural undisturbed condition.
- Limit clearing and grading of native vegetation at a site to the minimum amount needed to build lots, allow access, and provide fire protection.
- Maximize trees and other vegetation at each site by planting additional vegetation, clustering tree areas, and promoting the use of native and/or drought tolerant plants.
- Promote natural vegetation by using parking lot islands and other landscaped areas.
- Preserve riparian areas and wetlands.

## *Maximize Natural Water Storage and Infiltration Opportunities Within the Landscape Unit*

- Promote the conservation of forest cover. Building on land that is already deforested affects basin hydrology to a lesser extent than converting forested land. Loss of forest cover reduces interception storage, detention in the organic forest floor layer, and water losses by evapotranspiration, resulting in large peak runoff increases and either their negative effects or the expense of countering them with structural solutions.
- Maintain natural storage reservoirs and drainage corridors, including depressions, areas of permeable soils, swales, and intermittent streams. Develop and implement policies and

# Site Design & Landscape Planning SD-10

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regulations to discourage the clearing, filling, and channelization of these features. Utilize them in drainage networks in preference to pipes, culverts, and engineered ditches.

- Evaluating infiltration opportunities by referring to the stormwater management manual for the jurisdiction and pay particular attention to the selection criteria for avoiding groundwater contamination, poor soils, and hydrogeological conditions that cause these facilities to fail. If necessary, locate developments with large amounts of impervious surfaces or a potential to produce relatively contaminated runoff away from groundwater recharge areas.

## *Protection of Slopes and Channels during Landscape Design*

- Convey runoff safely from the tops of slopes.
- Avoid disturbing steep or unstable slopes.
- Avoid disturbing natural channels.
- Stabilize disturbed slopes as quickly as possible.
- Vegetate slopes with native or drought tolerant vegetation.
- Control and treat flows in landscaping and/or other controls prior to reaching existing natural drainage systems.
- Stabilize temporary and permanent channel crossings as quickly as possible, and ensure that increases in run-off velocity and frequency caused by the project do not erode the channel.
- Install energy dissipaters, such as riprap, at the outlets of new storm drains, culverts, conduits, or channels that enter unlined channels in accordance with applicable specifications to minimize erosion. Energy dissipaters shall be installed in such a way as to minimize impacts to receiving waters.
- Line on-site conveyance channels where appropriate, to reduce erosion caused by increased flow velocity due to increases in tributary impervious area. The first choice for linings should be grass or some other vegetative surface, since these materials not only reduce runoff velocities, but also provide water quality benefits from filtration and infiltration. If velocities in the channel are high enough to erode grass or other vegetative linings, riprap, concrete, soil cement, or geo-grid stabilization are other alternatives.
- Consider other design principles that are comparable and equally effective.

## ***Redeveloping Existing Installations***

Various jurisdictional stormwater management and mitigation plans (SUSMP, WQMP, etc.) define “redevelopment” in terms of amounts of additional impervious area, increases in gross floor area and/or exterior construction, and land disturbing activities with structural or impervious surfaces. The definition of “redevelopment” must be consulted to determine whether or not the requirements for new development apply to areas intended for redevelopment. If the definition applies, the steps outlined under “designing new installations” above should be followed.

# **SD-10 Site Design & Landscape Planning**

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Redevelopment may present significant opportunity to add features which had not previously been implemented. Examples include incorporation of depressions, areas of permeable soils, and swales in newly redeveloped areas. While some site constraints may exist due to the status of already existing infrastructure, opportunities should not be missed to maximize infiltration, slow runoff, reduce impervious areas, disconnect directly connected impervious areas.

## **Other Resources**

A Manual for the Standard Urban Stormwater Mitigation Plan (SUSMP), Los Angeles County Department of Public Works, May 2002.

Stormwater Management Manual for Western Washington, Washington State Department of Ecology, August 2001.

Model Standard Urban Storm Water Mitigation Plan (SUSMP) for San Diego County, Port of San Diego, and Cities in San Diego County, February 14, 2002.

Model Water Quality Management Plan (WQMP) for County of Orange, Orange County Flood Control District, and the Incorporated Cities of Orange County, Draft February 2003.

Ventura Countywide Technical Guidance Manual for Stormwater Quality Control Measures, July 2002.



## Design Objectives

- Maximize Infiltration
- Provide Retention
- Slow Runoff
- Minimize Impervious Land Coverage
- Prohibit Dumping of Improper Materials
- Contain Pollutants
- Collect and Convey

## Description

Irrigation water provided to landscaped areas may result in excess irrigation water being conveyed into stormwater drainage systems.

## Approach

Project plan designs for development and redevelopment should include application methods of irrigation water that minimize runoff of excess irrigation water into the stormwater conveyance system.

## Suitable Applications

Appropriate applications include residential, commercial and industrial areas planned for development or redevelopment. (Detached residential single-family homes are typically excluded from this requirement.)

## Design Considerations

### ***Designing New Installations***

The following methods to reduce excessive irrigation runoff should be considered, and incorporated and implemented where determined applicable and feasible by the Permittee:

- Employ rain-triggered shutoff devices to prevent irrigation after precipitation.
- Design irrigation systems to each landscape area's specific water requirements.
- Include design featuring flow reducers or shutoff valves triggered by a pressure drop to control water loss in the event of broken sprinkler heads or lines.
- Implement landscape plans consistent with County or City water conservation resolutions, which may include provision of water sensors, programmable irrigation times (for short cycles), etc.



- Design timing and application methods of irrigation water to minimize the runoff of excess irrigation water into the storm water drainage system.
- Group plants with similar water requirements in order to reduce excess irrigation runoff and promote surface filtration. Choose plants with low irrigation requirements (for example, native or drought tolerant species). Consider design features such as:
  - Using mulches (such as wood chips or bar) in planter areas without ground cover to minimize sediment in runoff
  - Installing appropriate plant materials for the location, in accordance with amount of sunlight and climate, and use native plant materials where possible and/or as recommended by the landscape architect
  - Leaving a vegetative barrier along the property boundary and interior watercourses, to act as a pollutant filter, where appropriate and feasible
  - Choosing plants that minimize or eliminate the use of fertilizer or pesticides to sustain growth
- Employ other comparable, equally effective methods to reduce irrigation water runoff.

### ***Redeveloping Existing Installations***

Various jurisdictional stormwater management and mitigation plans (SUSMP, WQMP, etc.) define “redevelopment” in terms of amounts of additional impervious area, increases in gross floor area and/or exterior construction, and land disturbing activities with structural or impervious surfaces. The definition of “redevelopment” must be consulted to determine whether or not the requirements for new development apply to areas intended for redevelopment. If the definition applies, the steps outlined under “designing new installations” above should be followed.

### **Other Resources**

A Manual for the Standard Urban Stormwater Mitigation Plan (SUSMP), Los Angeles County Department of Public Works, May 2002.

Model Standard Urban Storm Water Mitigation Plan (SUSMP) for San Diego County, Port of San Diego, and Cities in San Diego County, February 14, 2002.

Model Water Quality Management Plan (WQMP) for County of Orange, Orange County Flood Control District, and the Incorporated Cities of Orange County, Draft February 2003.

Ventura Countywide Technical Guidance Manual for Stormwater Quality Control Measures, July 2002.





## Design Objectives

- Maximize Infiltration
- Provide Retention
- Slow Runoff
- Minimize Impervious Land Coverage
- Prohibit Dumping of Improper Materials
- Contain Pollutants
- Collect and Convey

## Description

Waste materials dumped into storm drain inlets can have severe impacts on receiving and ground waters. Posting notices regarding discharge prohibitions at storm drain inlets can prevent waste dumping. Storm drain signs and stencils are highly visible source controls that are typically placed directly adjacent to storm drain inlets.

## Approach

The stencil or affixed sign contains a brief statement that prohibits dumping of improper materials into the urban runoff conveyance system. Storm drain messages have become a popular method of alerting the public about the effects of and the prohibitions against waste disposal.

## Suitable Applications

Stencils and signs alert the public to the destination of pollutants discharged to the storm drain. Signs are appropriate in residential, commercial, and industrial areas, as well as any other area where contributions or dumping to storm drains is likely.

## Design Considerations

Storm drain message markers or placards are recommended at all storm drain inlets within the boundary of a development project. The marker should be placed in clear sight facing toward anyone approaching the inlet from either side. All storm drain inlet locations should be identified on the development site map.

## *Designing New Installations*

The following methods should be considered for inclusion in the project design and show on project plans:

- Provide stenciling or labeling of all storm drain inlets and catch basins, constructed or modified, within the project area with prohibitive language. Examples include “NO DUMPING



– DRAINS TO OCEAN” and/or other graphical icons to discourage illegal dumping.

- Post signs with prohibitive language and/or graphical icons, which prohibit illegal dumping at public access points along channels and creeks within the project area.

Note - Some local agencies have approved specific signage and/or storm drain message placards for use. Consult local agency stormwater staff to determine specific requirements for placard types and methods of application.

### ***Redeveloping Existing Installations***

Various jurisdictional stormwater management and mitigation plans (SUSMP, WQMP, etc.) define “redevelopment” in terms of amounts of additional impervious area, increases in gross floor area and/or exterior construction, and land disturbing activities with structural or impervious surfaces. If the project meets the definition of “redevelopment”, then the requirements stated under “designing new installations” above should be included in all project design plans.

### **Additional Information**

#### ***Maintenance Considerations***

- Legibility of markers and signs should be maintained. If required by the agency with jurisdiction over the project, the owner/operator or homeowner’s association should enter into a maintenance agreement with the agency or record a deed restriction upon the property title to maintain the legibility of placards or signs.

#### ***Placement***

- Signage on top of curbs tends to weather and fade.
- Signage on face of curbs tends to be worn by contact with vehicle tires and sweeper brooms.

### **Supplemental Information**

#### ***Examples***

- Most MS4 programs have storm drain signage programs. Some MS4 programs will provide stencils, or arrange for volunteers to stencil storm drains as part of their outreach program.

### **Other Resources**

A Manual for the Standard Urban Stormwater Mitigation Plan (SUSMP), Los Angeles County Department of Public Works, May 2002.

Model Standard Urban Storm Water Mitigation Plan (SUSMP) for San Diego County, Port of San Diego, and Cities in San Diego County, February 14, 2002.

Model Water Quality Management Plan (WQMP) for County of Orange, Orange County Flood Control District, and the Incorporated Cities of Orange County, Draft February 2003.

Ventura Countywide Technical Guidance Manual for Stormwater Quality Control Measures, July 2002.

## Description

Trash storage areas are areas where a trash receptacle (s) are located for use as a repository for solid wastes. Stormwater runoff from areas where trash is stored or disposed of can be polluted. In addition, loose trash and debris can be easily transported by water or wind into nearby storm drain inlets, channels, and/or creeks. Waste handling operations that may be sources of stormwater pollution include dumpsters, litter control, and waste piles.

## Approach

This fact sheet contains details on the specific measures required to prevent or reduce pollutants in stormwater runoff associated with trash storage and handling. Preventative measures including enclosures, containment structures, and impervious pavements to mitigate spills, should be used to reduce the likelihood of contamination.

## Suitable Applications

Appropriate applications include residential, commercial and industrial areas planned for development or redevelopment. (Detached residential single-family homes are typically excluded from this requirement.)

## Design Considerations

Design requirements for waste handling areas are governed by Building and Fire Codes, and by current local agency ordinances and zoning requirements. The design criteria described in this fact sheet are meant to enhance and be consistent with these code and ordinance requirements. Hazardous waste should be handled in accordance with legal requirements established in Title 22, California Code of Regulation.

Wastes from commercial and industrial sites are typically hauled by either public or commercial carriers that may have design or access requirements for waste storage areas. The design criteria in this fact sheet are recommendations and are not intended to be in conflict with requirements established by the waste hauler. The waste hauler should be contacted prior to the design of your site trash collection areas. Conflicts or issues should be discussed with the local agency.

## *Designing New Installations*

Trash storage areas should be designed to consider the following structural or treatment control BMPs:

- Design trash container areas so that drainage from adjoining roofs and pavement is diverted around the area(s) to avoid run-on. This might include berming or grading the waste handling area to prevent run-on of stormwater.
- Make sure trash container areas are screened or walled to prevent off-site transport of trash.

## Design Objectives

- Maximize Infiltration
- Provide Retention
- Slow Runoff
- Minimize Impervious Land Coverage
- Prohibit Dumping of Improper Materials
- Contain Pollutants
- Collect and Convey



- Use lined bins or dumpsters to reduce leaking of liquid waste.
- Provide roofs, awnings, or attached lids on all trash containers to minimize direct precipitation and prevent rainfall from entering containers.
- Pave trash storage areas with an impervious surface to mitigate spills.
- Do not locate storm drains in immediate vicinity of the trash storage area.
- Post signs on all dumpsters informing users that hazardous materials are not to be disposed of therein.

### ***Redeveloping Existing Installations***

Various jurisdictional stormwater management and mitigation plans (SUSMP, WQMP, etc.) define “redevelopment” in terms of amounts of additional impervious area, increases in gross floor area and/or exterior construction, and land disturbing activities with structural or impervious surfaces. The definition of “redevelopment” must be consulted to determine whether or not the requirements for new development apply to areas intended for redevelopment. If the definition applies, the steps outlined under “designing new installations” above should be followed.

### **Additional Information**

#### ***Maintenance Considerations***

The integrity of structural elements that are subject to damage (i.e., screens, covers, and signs) must be maintained by the owner/operator. Maintenance agreements between the local agency and the owner/operator may be required. Some agencies will require maintenance deed restrictions to be recorded of the property title. If required by the local agency, maintenance agreements or deed restrictions must be executed by the owner/operator before improvement plans are approved.

### **Other Resources**

A Manual for the Standard Urban Stormwater Mitigation Plan (SUSMP), Los Angeles County Department of Public Works, May 2002.

Model Standard Urban Storm Water Mitigation Plan (SUSMP) for San Diego County, Port of San Diego, and Cities in San Diego County, February 14, 2002.

Model Water Quality Management Plan (WQMP) for County of Orange, Orange County Flood Control District, and the Incorporated Cities of Orange County, Draft February 2003.

Ventura Countywide Technical Guidance Manual for Stormwater Quality Control Measures, July 2002.



## Description

Stormwater runoff from building and grounds maintenance activities can be contaminated with toxic hydrocarbons in solvents, fertilizers and pesticides, suspended solids, heavy metals, abnormal pH, and oils and greases. Utilizing the protocols in this fact sheet will prevent or reduce the discharge of pollutants to stormwater from building and grounds maintenance activities by washing and cleaning up with as little water as possible, following good landscape management practices, preventing and cleaning up spills immediately, keeping debris from entering the storm drains, and maintaining the stormwater collection system.

## Approach

Reduce potential for pollutant discharge through source control pollution prevention and BMP implementation. Successful implementation depends on effective training of employees on applicable BMPs and general pollution prevention strategies and objectives.

## Pollution Prevention

- Switch to non-toxic chemicals for maintenance when possible.
- Choose cleaning agents that can be recycled.
- Encourage proper lawn management and landscaping, including use of native vegetation.

## Objectives

- Cover
- Contain
- Educate
- Reduce/Minimize
- Product Substitution

## Targeted Constituents

Sediment	✓
Nutrients	✓
Trash	
Metals	✓
Bacteria	✓
Oil and Grease	
Organics	



# SC-41 Building & Grounds Maintenance

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- Encourage use of Integrated Pest Management techniques for pest control.
- Encourage proper onsite recycling of yard trimmings.
- Recycle residual paints, solvents, lumber, and other material as much as possible.

## ***Suggested Protocols***

### *Pressure Washing of Buildings, Rooftops, and Other Large Objects*

- In situations where soaps or detergents are used and the surrounding area is paved, pressure washers must use a water collection device that enables collection of wash water and associated solids. A sump pump, wet vacuum or similarly effective device must be used to collect the runoff and loose materials. The collected runoff and solids must be disposed of properly.
- If soaps or detergents are not used, and the surrounding area is paved, wash runoff does not have to be collected but must be screened. Pressure washers must use filter fabric or some other type of screen on the ground and/or in the catch basin to trap the particles in wash water runoff.
- If you are pressure washing on a grassed area (with or without soap), runoff must be dispersed as sheet flow as much as possible, rather than as a concentrated stream. The wash runoff must remain on the grass and not drain to pavement.

### *Landscaping Activities*

- Dispose of grass clippings, leaves, sticks, or other collected vegetation as garbage, or by composting. Do not dispose of collected vegetation into waterways or storm drainage systems.
- Use mulch or other erosion control measures on exposed soils.

### *Building Repair, Remodeling, and Construction*

- Do not dump any toxic substance or liquid waste on the pavement, the ground, or toward a storm drain.
- Use ground or drop cloths underneath outdoor painting, scraping, and sandblasting work, and properly dispose of collected material daily.
- Use a ground cloth or oversized tub for activities such as paint mixing and tool cleaning.
- Clean paintbrushes and tools covered with water-based paints in sinks connected to sanitary sewers or in portable containers that can be dumped into a sanitary sewer drain. Brushes and tools covered with non-water-based paints, finishes, or other materials must be cleaned in a manner that enables collection of used solvents (e.g., paint thinner, turpentine, etc.) for recycling or proper disposal.
- Use a storm drain cover, filter fabric, or similarly effective runoff control mechanism if dust, grit, wash water, or other pollutants may escape the work area and enter a catch basin. This is particularly necessary on rainy days. The containment device(s) must be in place at the beginning of the work day, and accumulated dirty runoff and solids must be collected and disposed of before removing the containment device(s) at the end of the work day.

- If you need to de-water an excavation site, you may need to filter the water before discharging to a catch basin or off-site. If directed off-site, you should direct the water through hay bales and filter fabric or use other sediment filters or traps.
- Store toxic material under cover during precipitation events and when not in use. A cover would include tarps or other temporary cover material.

### *Mowing, Trimming, and Planting*

- Dispose of leaves, sticks, or other collected vegetation as garbage, by composting or at a permitted landfill. Do not dispose of collected vegetation into waterways or storm drainage systems.
- Use mulch or other erosion control measures when soils are exposed.
- Place temporarily stockpiled material away from watercourses and drain inlets, and berm or cover stockpiles to prevent material releases to the storm drain system.
- Consider an alternative approach when bailing out muddy water: do not put it in the storm drain; pour over landscaped areas.
- Use hand weeding where practical.

### *Fertilizer and Pesticide Management*

- Follow all federal, state, and local laws and regulations governing the use, storage, and disposal of fertilizers and pesticides and training of applicators and pest control advisors.
- Use less toxic pesticides that will do the job when applicable. Avoid use of copper-based pesticides if possible.
- Do not use pesticides if rain is expected.
- Do not mix or prepare pesticides for application near storm drains.
- Use the minimum amount needed for the job.
- Calibrate fertilizer distributors to avoid excessive application.
- Employ techniques to minimize off-target application (e.g., spray drift) of pesticides, including consideration of alternative application techniques.
- Apply pesticides only when wind speeds are low.
- Fertilizers should be worked into the soil rather than dumped or broadcast onto the surface.
- Irrigate slowly to prevent runoff and then only as much as is needed.
- Clean pavement and sidewalk if fertilizer is spilled on these surfaces before applying irrigation water.
- Dispose of empty pesticide containers according to the instructions on the container label.

# SC-41 Building & Grounds Maintenance

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- Use up the pesticides. Rinse containers, and use rinse water as product. Dispose of unused pesticide as hazardous waste.
- Implement storage requirements for pesticide products with guidance from the local fire department and County Agricultural Commissioner. Provide secondary containment for pesticides.

## *Inspection*

- Inspect irrigation system periodically to ensure that the right amount of water is being applied and that excessive runoff is not occurring. Minimize excess watering and repair leaks in the irrigation system as soon as they are observed.

## **Training**

- Educate and train employees on pesticide use and in pesticide application techniques to prevent pollution.
- Train employees and contractors in proper techniques for spill containment and cleanup.
- Be sure the frequency of training takes into account the complexity of the operations and the nature of the staff.

## **Spill Response and Prevention**

- Keep your Spill Prevention Control and Countermeasure (SPCC) Plan up-to-date.
- Place a stockpile of spill cleanup materials, such as brooms, dustpans, and vacuum sweepers (if desired) near the storage area where it will be readily accessible.
- Have employees trained in spill containment and cleanup present during the loading/unloading of dangerous wastes, liquid chemicals, or other materials.
- Familiarize employees with the Spill Prevention Control and Countermeasure Plan.
- Clean up spills immediately.

## **Other Considerations**

Alternative pest/weed controls may not be available, suitable, or effective in many cases.

## **Requirements**

### **Costs**

- Cost will vary depending on the type and size of facility.
- Overall costs should be low in comparison to other BMPs.

### **Maintenance**

Sweep paved areas regularly to collect loose particles. Wipe up spills with rags and other absorbent material immediately, do not hose down the area to a storm drain.



## Supplemental Information

### *Further Detail of the BMP*

#### *Fire Sprinkler Line Flushing*

Building fire sprinkler line flushing may be a source of non-stormwater runoff pollution. The water entering the system is usually potable water, though in some areas it may be non-potable reclaimed wastewater. There are subsequent factors that may drastically reduce the quality of the water in such systems. Black iron pipe is usually used since it is cheaper than potable piping, but it is subject to rusting and results in lower quality water. Initially, the black iron pipe has an oil coating to protect it from rusting between manufacture and installation; this will contaminate the water from the first flush but not from subsequent flushes. Nitrates, poly-phosphates and other corrosion inhibitors, as well as fire suppressants and antifreeze may be added to the sprinkler water system. Water generally remains in the sprinkler system a long time (typically a year) and between flushes may accumulate iron, manganese, lead, copper, nickel, and zinc. The water generally becomes anoxic and contains living and dead bacteria and breakdown products from chlorination. This may result in a significant BOD problem and the water often smells. Consequently dispose fire sprinkler line flush water into the sanitary sewer. Do not allow discharge to storm drain or infiltration due to potential high levels of pollutants in fire sprinkler line water.

## References and Resources

California's Nonpoint Source Program Plan <http://www.swrcb.ca.gov/nps/index.html>

Clark County Storm Water Pollution Control Manual  
<http://www.co.clark.wa.us/pubworks/bmpman.pdf>

King County Storm Water Pollution Control Manual <http://dnr.metrokc.gov/wlr/dss/spcm.htm>

Mobile Cleaners Pilot Program: Final Report. 1997. Bay Area Stormwater Management Agencies Association (BASMAA). <http://www.basmaa.org/>

Pollution from Surface Cleaning Folder. 1996. Bay Area Stormwater Management Agencies Association (BASMAA). <http://www.basmaa.org/>

Santa Clara Valley Urban Runoff Pollution Prevention Program <http://www.scvurppp.org>

The Storm Water Managers Resource Center <http://www.stormwatercenter.net/>

# Parking/Storage Area Maintenance SC-43



## Description

Parking lots and storage areas can contribute a number of substances, such as trash, suspended solids, hydrocarbons, oil and grease, and heavy metals that can enter receiving waters through stormwater runoff or non-stormwater discharges. The protocols in this fact sheet are intended to prevent or reduce the discharge of pollutants from parking/storage areas and include using good housekeeping practices, following appropriate cleaning BMPs, and training employees.

## Approach

The goal of this program is to ensure stormwater pollution prevention practices are considered when conducting activities on or around parking areas and storage areas to reduce potential for pollutant discharge to receiving waters. Successful implementation depends on effective training of employees on applicable BMPs and general pollution prevention strategies and objectives.

## Pollution Prevention

- Encourage alternative designs and maintenance strategies for impervious parking lots. (See New Development and Redevelopment BMP Handbook)
- Keep accurate maintenance logs to evaluate BMP implementation.

## Objectives

- Cover
- Contain
- Educate
- Reduce/Minimize
- Product Substitution

## Targeted Constituents

Sediment	✓
Nutrients	
Trash	✓
Metals	✓
Bacteria	
Oil and Grease	✓
Organics	✓



# **SC-43 Parking/Storage Area Maintenance**

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## ***Suggested Protocols***

### *General*

- Keep the parking and storage areas clean and orderly. Remove debris in a timely fashion.
- Allow sheet runoff to flow into biofilters (vegetated strip and swale) and/or infiltration devices.
- Utilize sand filters or oleophilic collectors for oily waste in low quantities.
- Arrange rooftop drains to prevent drainage directly onto paved surfaces.
- Design lot to include semi-permeable hardscape.
- Discharge soapy water remaining in mop or wash buckets to the sanitary sewer through a sink, toilet, clean-out, or wash area with drain.

### *Controlling Litter*

- Post “No Littering” signs and enforce anti-litter laws.
- Provide an adequate number of litter receptacles.
- Clean out and cover litter receptacles frequently to prevent spillage.
- Provide trash receptacles in parking lots to discourage litter.
- Routinely sweep, shovel, and dispose of litter in the trash.

### *Surface Cleaning*

- Use dry cleaning methods (e.g., sweeping, vacuuming) to prevent the discharge of pollutants into the stormwater conveyance system if possible.
- Establish frequency of public parking lot sweeping based on usage and field observations of waste accumulation.
- Sweep all parking lots at least once before the onset of the wet season.
- Follow the procedures below if water is used to clean surfaces:
  - Block the storm drain or contain runoff.
  - Collect and pump wash water to the sanitary sewer or discharge to a pervious surface. Do not allow wash water to enter storm drains.
  - Dispose of parking lot sweeping debris and dirt at a landfill.
- Follow the procedures below when cleaning heavy oily deposits:
  - Clean oily spots with absorbent materials.
  - Use a screen or filter fabric over inlet, then wash surfaces.

# **Parking/Storage Area Maintenance SC-43**

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- Do not allow discharges to the storm drain.
- Vacuum/pump discharges to a tank or discharge to sanitary sewer.
- Appropriately dispose of spilled materials and absorbents.

## *Surface Repair*

- Preheat, transfer or load hot bituminous material away from storm drain inlets.
- Apply concrete, asphalt, and seal coat during dry weather to prevent contamination from contacting stormwater runoff.
- Cover and seal nearby storm drain inlets where applicable (with waterproof material or mesh) and manholes before applying seal coat, slurry seal, etc. Leave covers in place until job is complete and all water from emulsified oil sealants has drained or evaporated. Clean any debris from these covered manholes and drains for proper disposal.
- Use only as much water as necessary for dust control, to avoid runoff.
- Catch drips from paving equipment that is not in use with pans or absorbent material placed under the machines. Dispose of collected material and absorbents properly.

## *Inspection*

- Have designated personnel conduct inspections of parking facilities and stormwater conveyance systems associated with parking facilities on a regular basis.
- Inspect cleaning equipment/sweepers for leaks on a regular basis.

## **Training**

- Provide regular training to field employees and/or contractors regarding cleaning of paved areas and proper operation of equipment.
- Train employees and contractors in proper techniques for spill containment and cleanup.

## **Spill Response and Prevention**

- Keep your Spill Prevention Control and Countermeasure (SPCC) Plan up-to-date.
- Place a stockpile of spill cleanup materials where it will be readily accessible or at a central location.
- Clean up fluid spills immediately with absorbent rags or material.
- Dispose of spilled material and absorbents properly.

## **Other Considerations**

Limitations related to sweeping activities at large parking facilities may include high equipment costs, the need for sweeper operator training, and the inability of current sweeper technology to remove oil and grease.

# **SC-43 Parking/Storage Area Maintenance**

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## **Requirements**

### ***Costs***

Cleaning/sweeping costs can be quite large. Construction and maintenance of stormwater structural controls can be quite expensive as well.

### ***Maintenance***

- Sweep parking lot regularly to minimize cleaning with water.
- Clean out oil/water/sand separators regularly, especially after heavy storms.
- Clean parking facilities regularly to prevent accumulated wastes and pollutants from being discharged into conveyance systems during rainy conditions.

## **Supplemental Information**

### ***Further Detail of the BMP***

#### ***Surface Repair***

Apply concrete, asphalt, and seal coat during dry weather to prevent contamination from contacting stormwater runoff. Where applicable, cover and seal nearby storm drain inlets (with waterproof material or mesh) and manholes before applying seal coat, slurry seal, etc. Leave covers in place until job is complete and all water from emulsified oil sealants has drained or evaporated. Clean any debris from these covered manholes and drains for proper disposal. Only use only as much water as is necessary for dust control to avoid runoff.

## **References and Resources**

California's Nonpoint Source Program Plan <http://www.swrcb.ca.gov/nps/index.html>

Clark County Storm Water Pollution Control Manual  
<http://www.co.clark.wa.us/pubworks/bmpman.pdf>

King County Storm Water Pollution Control Manual <http://dnr.metrokc.gov/wlr/dss/spcm.htm>

Pollution from Surface Cleaning Folder. 1996. Bay Area Stormwater Management Agencies Association (BASMAA). <http://www.basmaa.org/>

Oregon Association of Clean Water Agencies. Oregon Municipal Stormwater Toolbox for Maintenance Practices. June 1998.

Santa Clara Valley Urban Runoff Pollution Prevention Program <http://www.scvurppp.org>

The Storm Water Managers Resource Center <http://www.stormwatercenter.net/>

# SC-44      Drainage System Maintenance

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- Clean catch basins, storm drain inlets, and other conveyance structures before the wet season to remove sediments and debris accumulated during the summer.
- Conduct inspections more frequently during the wet season for problem areas where sediment or trash accumulates more often. Clean and repair as needed.
- Keep accurate logs of the number of catch basins cleaned.
- Store wastes collected from cleaning activities of the drainage system in appropriate containers or temporary storage sites in a manner that prevents discharge to the storm drain.
- Dewater the wastes if necessary with outflow into the sanitary sewer if permitted. Water should be treated with an appropriate filtering device prior to discharge to the sanitary sewer. If discharge to the sanitary sewer is not allowed, water should be pumped or vacuumed to a tank and properly disposed. Do not dewater near a storm drain or stream.

## *Storm Drain Conveyance System*

- Locate reaches of storm drain with deposit problems and develop a flushing schedule that keeps the pipe clear of excessive buildup.
- Collect and pump flushed effluent to the sanitary sewer for treatment whenever possible.

## *Pump Stations*

- Clean all storm drain pump stations prior to the wet season to remove silt and trash.
- Do not allow discharge to reach the storm drain system when cleaning a storm drain pump station or other facility.
- Conduct routine maintenance at each pump station.
- Inspect, clean, and repair as necessary all outlet structures prior to the wet season.

## *Open Channel*

- Modify storm channel characteristics to improve channel hydraulics, increase pollutant removals, and enhance channel/creek aesthetic and habitat value.
- Conduct channel modification/improvement in accordance with existing laws. Any person, government agency, or public utility proposing an activity that will change the natural (emphasis added) state of any river, stream, or lake in California, must enter into a Stream or Lake Alteration Agreement with the Department of Fish and Game. The developer-applicant should also contact local governments (city, county, special districts), other state agencies (SWRCB, RWQCB, Department of Forestry, Department of Water Resources), and Federal Corps of Engineers and USFWS.

## *Illicit Connections and Discharges*

- Look for evidence of illegal discharges or illicit connections during routine maintenance of conveyance system and drainage structures:
  - Is there evidence of spills such as paints, discoloring, etc?

- Are there any odors associated with the drainage system?
- Record locations of apparent illegal discharges/illicit connections?
- Track flows back to potential dischargers and conduct aboveground inspections. This can be done through visual inspection of upgradient manholes or alternate techniques including zinc chloride smoke testing, fluorometric dye testing, physical inspection testing, or television camera inspection.
- Eliminate the discharge once the origin of flow is established.
- Stencil or demarcate storm drains, where applicable, to prevent illegal disposal of pollutants. Storm drain inlets should have messages such as “Dump No Waste Drains to Stream” stenciled next to them to warn against ignorant or intentional dumping of pollutants into the storm drainage system.
- Refer to fact sheet SC-10 Non-Stormwater Discharges.

## *Illegal Dumping*

- Inspect and clean up hot spots and other storm drainage areas regularly where illegal dumping and disposal occurs.
- Establish a system for tracking incidents. The system should be designed to identify the following:
  - Illegal dumping hot spots
  - Types and quantities (in some cases) of wastes
  - Patterns in time of occurrence (time of day/night, month, or year)
  - Mode of dumping (abandoned containers, “midnight dumping” from moving vehicles, direct dumping of materials, accidents/spills)
  - Responsible parties
- Post “No Dumping” signs in problem areas with a phone number for reporting dumping and disposal. Signs should also indicate fines and penalties for illegal dumping.
- Refer to fact sheet SC-10 Non-Stormwater Discharges.

## **Training**

- Train crews in proper maintenance activities, including record keeping and disposal.
- Allow only properly trained individuals to handle hazardous materials/wastes.
- Have staff involved in detection and removal of illicit connections trained in the following:
  - OSHA-required Health and Safety Training (29 CFR 1910.120) plus annual refresher training (as needed).

- OSHA Confined Space Entry training (Cal-OSHA Confined Space, Title 8 and Federal OSHA 29 CFR 1910.146).
- Procedural training (field screening, sampling, smoke/dye testing, TV inspection).

### ***Spill Response and Prevention***

- Investigate all reports of spills, leaks, and/or illegal dumping promptly.
- Clean up all spills and leaks using “dry” methods (with absorbent materials and/or rags) or dig up, remove, and properly dispose of contaminated soil.
- Refer to fact sheet SC-11 Spill Prevention, Control, and Cleanup.

### ***Other Considerations (Limitations and Regulations)***

- Clean-up activities may create a slight disturbance for local aquatic species. Access to items and material on private property may be limited. Trade-offs may exist between channel hydraulics and water quality/riparian habitat. If storm channels or basins are recognized as wetlands, many activities, including maintenance, may be subject to regulation and permitting.
- Storm drain flushing is most effective in small diameter pipes (36-inch diameter pipe or less, depending on water supply and sediment collection capacity). Other considerations associated with storm drain flushing may include the availability of a water source, finding a downstream area to collect sediments, liquid/sediment disposal, and prohibition against disposal of flushed effluent to sanitary sewer in some areas.
- Regulations may include adoption of substantial penalties for illegal dumping and disposal.
- Local municipal codes may include sections prohibiting discharge of soil, debris, refuse, hazardous wastes, and other pollutants into the storm drain system.

## **Requirements**

### ***Costs***

- An aggressive catch basin cleaning program could require a significant capital and O&M budget.
- The elimination of illegal dumping is dependent on the availability, convenience, and cost of alternative means of disposal. The primary cost is for staff time. Cost depends on how aggressively a program is implemented. Other cost considerations for an illegal dumping program include:
  - Purchase and installation of signs.
  - Rental of vehicle(s) to haul illegally-disposed items and material to landfills.
  - Rental of heavy equipment to remove larger items (e.g., car bodies) from channels.
  - Purchase of landfill space to dispose of illegally-dumped items and material.



- Methods used for illicit connection detection (smoke testing, dye testing, visual inspection, and flow monitoring) can be costly and time-consuming. Site-specific factors, such as the level of impervious area, the density and ages of buildings, and type of land use will determine the level of investigation necessary.

## ***Maintenance***

- Two-person teams may be required to clean catch basins with vacuor trucks.
- Teams of at least two people plus administrative personnel are required to identify illicit discharges, depending on the complexity of the storm sewer system.
- Arrangements must be made for proper disposal of collected wastes.
- Technical staff are required to detect and investigate illegal dumping violations.

## **Supplemental Information**

### ***Further Detail of the BMP***

#### ***Storm Drain Flushing***

Flushing is a common maintenance activity used to improve pipe hydraulics and to remove pollutants in storm drainage systems. Flushing may be designed to hydraulically convey accumulated material to strategic locations, such as an open channel, another point where flushing will be initiated, or the sanitary sewer and the treatment facilities, thus preventing resuspension and overflow of a portion of the solids during storm events. Flushing prevents “plug flow” discharges of concentrated pollutant loadings and sediments. Deposits can hinder the designed conveyance capacity of the storm drain system and potentially cause backwater conditions in severe cases of clogging.

Storm drain flushing usually takes place along segments of pipe with grades that are too flat to maintain adequate velocity to keep particles in suspension. An upstream manhole is selected to place an inflatable device that temporarily plugs the pipe. Further upstream, water is pumped into the line to create a flushing wave. When the upstream reach of pipe is sufficiently full to cause a flushing wave, the inflated device is rapidly deflated with the assistance of a vacuum pump, thereby releasing the backed up water and resulting in the cleaning of the storm drain segment.

To further reduce impacts of stormwater pollution, a second inflatable device placed well downstream may be used to recollect the water after the force of the flushing wave has dissipated. A pump may then be used to transfer the water and accumulated material to the sanitary sewer for treatment. In some cases, an interceptor structure may be more practical or required to recollect the flushed waters.

It has been found that cleansing efficiency of periodic flush waves is dependent upon flush volume, flush discharge rate, sewer slope, sewer length, sewer flow rate, sewer diameter, and population density. As a rule of thumb, the length of line to be flushed should not exceed 700 feet. At this maximum recommended length, the percent removal efficiency ranges between 65-75% for organics and 55-65% for dry weather grit/inorganic material. The percent removal efficiency drops rapidly beyond that. Water is commonly supplied by a water truck, but fire hydrants can also supply water. To make the best use of water, it is recommended that reclaimed water be used or that fire hydrant line flushing coincide with storm sewer flushing.

# SC-44      Drainage System Maintenance

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## References and Resources

California's Nonpoint Source Program Plan <http://www.swrcb.ca.gov/nps/index.html>

Clark County Storm Water Pollution Control Manual  
<http://www.co.clark.wa.us/pubworks/bmpman.pdf>

Ferguson, B.K. 1991. Urban Stream Reclamation, p. 324-322, Journal of Soil and Water Conservation.

King County Storm Water Pollution Control Manual <http://dnr.metrokc.gov/wlr/dss/spcm.htm>

Oregon Association of Clean Water Agencies. Oregon Municipal Stormwater Toolbox for Maintenance Practices. June 1998.

Santa Clara Valley Urban Runoff Pollution Prevention Program <http://www.scvurppp.org>

The Storm Water Managers Resource Center <http://www.stormwatercenter.net>

United States Environmental Protection Agency (USEPA). 2002. Pollution Prevention/Good Housekeeping for Municipal Operations Storm Drain System Cleaning. On line:  
[http://www.epa.gov/npdes/menuofbmps/poll\\_16.htm](http://www.epa.gov/npdes/menuofbmps/poll_16.htm)

## General Description

Drain inlet inserts, also known as catch basin, drop inlet or curb inlet inserts, are used to remove pollutants at the point of entry to the storm drain system. There are a multitude of inserts of various shapes and configurations including baffles, baskets, boxes, fabrics, sorbent media, screens, and skimmers. The effectiveness of drain inlet inserts depends on their design, application, loading, and frequency of maintenance to remove accumulated sediment, trash, and debris.

## Inspection/Maintenance Considerations

Routine inspection and maintenance is necessary to maintain functionality of drain inlet inserts and to prevent re-suspension and discharge of accumulated pollutants. Maintenance activities vary depending on the type of drain inlet insert being implemented; refer to the manufacturer's recommendations for more information.

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## Advanced BMPs Covered

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## Maintenance Concerns

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- *Sediment, Trash, and Debris Accumulations*
- *Pollutant Re-suspension and Discharge*

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## Targeted Constituents\*

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<i>Sediment</i>	✓
<i>Nutrients</i>	✓
<i>Trash</i>	✓
<i>Metals</i>	✓
<i>Bacteria</i>	
<i>Oil and Grease</i>	✓
<i>Organics</i>	✓

*\*Removal Effectiveness varies for different manufacturer designs. See New Development and Redevelopment Handbook-Section 5 for more information.*

---



Inspection Activities	Suggested Frequency
<input type="checkbox"/> Verify that stormwater enters the unit and does not leak around the perimeter.	After construction.
<input type="checkbox"/> Inspect for sediment, trash, and debris buildup and proper functioning.	At the beginning of the wet season and after significant storms
Maintenance Activities	Suggested Frequency
<input type="checkbox"/> Remove accumulated sediment, trash, and debris. <input type="checkbox"/> Replace sorbent media.	At the beginning of the wet season and as necessary

## References

California Department of Transportation. *Treatment BMP Technology Report (CTSW-RT-09-239.06)*, April, 2010. <http://www.dot.ca.gov/hq/env/stormwater/pdf/CTSW-RT-09-239-06.pdf>.

California Stormwater Quality Association. *Stormwater Best Management Practice Handbook, New Development and Redevelopment*, 2003. <https://www.casqa.org/resources/bmp-handbooks/new-development-redevelopment-bmp-handbook>.

Orange County Stormwater Program. Technical Guidance Document BMP Fact Sheets. [http://media.ocgov.com/gov/pw/watersheds/documents/wqmp/tgd/technical\\_guidance\\_document\\_bmp\\_fact\\_sheets.asp](http://media.ocgov.com/gov/pw/watersheds/documents/wqmp/tgd/technical_guidance_document_bmp_fact_sheets.asp).

San Francisco Public Utilities Commission, et al. San Francisco Stormwater Design Guidelines. Appendix A, Stormwater BMP Fact Sheets, June, 2010. <http://www.sfwater.org/modules/showdocument.aspx?documentid=2778>.

Tahoe Regional Planning Agency. Best Management Practices Handbook, 2012. <http://www.tahoebmp.org/Documents/2012%20BMP%20Handbook.pdf>.

U.S. Environmental Protection Agency, Post-Construction Stormwater Management in New Development and Redevelopment. BMP Fact Sheets. Available at: [http://cfpub.epa.gov/npdes/stormwater/menuofbmps/index.cfm?action=min\\_measure&min\\_measure\\_id=5](http://cfpub.epa.gov/npdes/stormwater/menuofbmps/index.cfm?action=min_measure&min_measure_id=5).

Ventura Countywide Stormwater Quality Management Program. *Technical Guidance Manual for Stormwater Quality Control Measures*, May, 2010. [http://www.vcstormwater.org/documents/workproducts/technicalguidancemanual/2010revisions/Ventura%20Technical%20Guidance%20Document\\_5-6-10.pdf](http://www.vcstormwater.org/documents/workproducts/technicalguidancemanual/2010revisions/Ventura%20Technical%20Guidance%20Document_5-6-10.pdf).

# Stormwater Pollution Prevention

*Best Management Practices for Homeowner's Associations,  
Property Managers and Property Owners*



*Your Guide To Maintaining Water  
Friendly Standards In Your Community*

[sbcountystormwater.org](http://sbcountystormwater.org)

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## » Table of Contents

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Commercial Trash Enclosures	1
Hazardous Waste	2
Working Outdoors & Handling Spills	4
Commercial Landscape	5
Sidewalk, Plaza, Entry Monument & Fountain Maintenance	6
Equipment Maintenance & Repair	10
Pool Maintenance	14
Paint	16
Vehicle Maintenance	17
Pet Waste Disposal	18
Get In Touch With Us Online	19

# COMMERCIAL TRASH ENCLOSURES

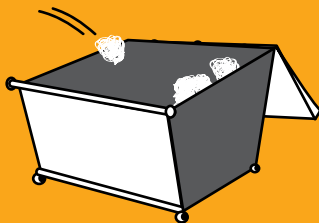
## FOLLOW THESE REQUIREMENTS TO KEEP OUR WATERWAYS CLEAN

Trash enclosures, such as those found in commercial and apartment complexes, typically contain materials that are intended to find their way to a landfill or a recycling facility.

**These materials are NOT meant to go into our local lakes and rivers.**

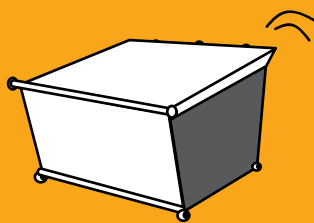
### PROTECT WATER QUALITY BY FOLLOWING THESE SIMPLE STEPS

#### PUT TRASH INSIDE



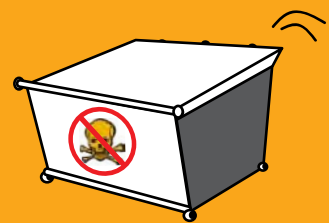
Place trash inside the bin (preferably in sealed bags)

#### CLOSE THE LID



Prevent rain from entering the bin in order to avoid leakage of polluted water runoff

#### KEEP TOXICS OUT



- Paint
- Grease, fats and used oils
- Batteries, electronics and fluorescent lights

### SOME ADDITIONAL GUIDELINES, INCLUDE

#### ✓ SWEEP FREQUENTLY

Sweep trash enclosure areas frequently, instead of hosing them down, to prevent polluted water from flowing into the streets and storm drains.

#### ✓ FIX LEAKS

Address trash bin leaks immediately by using dry clean up methods and report to your waste hauler to receive a replacement.

#### ✓ CONSTRUCT ROOF

Construct a solid cover roof over the existing trash enclosure structure to prevent rainwater from coming into contact with trash and garbage. Check with your local City/County for Building Codes.

In San Bernardino County, stormwater pollution is caused by food waste, landscape waste, chemicals and other debris that are washed into storm drains and end up in our waterways - untreated! You can be part of the solution by maintaining a water-friendly trash enclosure.

**THANK YOU FOR HELPING TO KEEP SAN BERNARDINO COUNTY CLEAN AND HEALTHY!**



In the event of a spill or discharge to a storm drain or waterway, contact San Bernardino County Stormwater immediately: (877) WASTE18 | [sbcountystormwater.org/report](http://sbcountystormwater.org/report)

[sbcountystormwater.org](http://sbcountystormwater.org)

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# COMMERCIAL LANDSCAPE

DISCHARGE TO THE STORM DRAIN, ACCIDENTAL OR NOT, COULD LEAD TO ENFORCEMENT ACTIONS, WHICH COULD INCLUDE FINES.

Follow the best practices below to **prevent water pollution from landscaping activities.**

## RECYCLE YARD WASTE



- ✓ Recycle leaves, grass clippings and other yard waste.
- ✓ Do not blow, sweep, rake or hose yard waste into the street or catch basin.
- ✓ **Try grasscycling:** the natural recycling of grass by leaving clippings on the lawn when mowing.

For more information, please visit:  
[www.calrecycle.ca.gov/organics/grasscycling](http://www.calrecycle.ca.gov/organics/grasscycling)

## USE FERTILIZERS, HERBICIDES AND PESTICIDES SAFELY



- ✓ Fertilizers, herbicides and pesticides are often carried into the storm drain system by sprinkler runoff. Use natural and non-toxic alternatives as often as possible.
- ✓ If you must use chemical fertilizers, herbicides or pesticides:
  - Spot apply, rather than blanketing entire areas.
  - Avoid applying near curbs and driveways, and **never** before a rain.
  - Apply fertilizers as needed: when plants could best use it and when the potential runoff would be low.
  - Follow the manufacturer's instructions carefully—this will not only give the best results, but will save money.

## USE WATER WISELY



- ✓ Control the amount of water and direction of sprinklers. Sprinklers should only be on long enough to allow water to soak into the ground, but not so long as to cause runoff.
- ✓ Periodically inspect, fix leaks and realign sprinkler heads.
- ✓ Plant native vegetation to reduce the need of water, fertilizers, herbicides and pesticides.

## ! HOMEOWNERS

KEEP THESE TIPS IN MIND WHEN HIRING PROFESSIONAL LANDSCAPERS AND REMIND AS NECESSARY.



Leftover pesticides, fertilizers, and herbicides contaminate landfills and should be disposed of through a Hazardous Waste Facility.

For more information on proper disposal call,

**(909) 382-5401 or 1-800-OILY CAT.**

\*FREE for San Bernardino County residents only. Businesses can call for cost inquiries and to schedule an appointment.



In the event of a spill or discharge to a storm drain or waterway, contact San Bernardino County Stormwater immediately: (877) WASTE18 | [sbcountystormwater.org/report](http://sbcountystormwater.org/report)

[sbcountystormwater.org](http://sbcountystormwater.org)

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# SIDEWALK, PLAZA, ENTRY MONUMENT & FOUNTAIN MAINTENANCE

**Pollutants on sidewalks and other pedestrian traffic areas and plazas are typically due to littering and vehicle use. Fountain water containing chlorine and copperbased algaecides is toxic to aquatic life. Proper inspection, cleaning, and repair of pedestrian areas and HOA owned surfaces and structures can reduce pollutant runoff from these areas. Maintaining these areas may involve one or more of the following activities:**

- 1. Surface Cleaning**
- 2. Graffiti Cleaning**
- 3. Sidewalk Repair**
- 4. Controlling Litter**
- 5. Fountain Maintenance**

## **POLLUTION PREVENTION:**

Pollution prevention measures have been considered and incorporated in the model procedures. Implementation of these measures may be more effective and reduce or eliminate the need to implement other more complicated or costly procedures. Possible pollution prevention measures for sidewalk, plaza, and fountain maintenance and cleaning include:

- Use dry cleaning methods whenever practical for surface cleaning activities.
- Use the least toxic materials available (e.g. water based paints, gels or sprays for graffiti removal).
- Once per year, educate HOA staff and tenants on pollution prevention measures.

## **MODEL PROCEDURES:**

### 1. Surface Cleaning

**Discharges of wash water to the storm water drainage system from cleaning or hosing of impervious surfaces is prohibited.**

#### **Sidewalks, Plazas**

- ✓ Use dry methods (e.g. sweeping, backpack blowers, vacuuming) whenever practical to clean sidewalks and plazas rather than hosing, pressure washing, or steam cleaning. **DO NOT** sweep or blow material into curb; use devices that contain the materials.
- ✓ If water must be used, block storm drain inlets and contain runoff. Discharge wash water to landscaping or contain and dispose of properly.



In the event of a spill or discharge to a storm drain or waterway, contact San Bernardino County Stormwater immediately: (877) WASTE18 | [sbcountystormwater.org/report](http://sbcountystormwater.org/report)

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# SIDEWALK, PLAZA, ENTRY MONUMENT & FOUNTAIN MAINTENANCE

## **Parking Areas, Driveways, Drive-thru**

- ✓ Parking facilities should be swept/vacuumed on a regular basis. Establish frequency of public parking lot sweeping based on usage and field observations of waste accumulation.
- ✓ If water must be used, block storm drain inlets and contain runoff. Discharge wash water to landscaping or contain and dispose of properly.
- ✓ Sweep all parking lots at least once before the onset of the wet season.
- ✓ Use absorbents to pick up oil; then dry sweep.
- ✓ Appropriately dispose of spilled materials and absorbents.

### OPTIONAL:

- Consider increasing sweeping frequency based on factors such as traffic volume, land use, field observations of sediment and trash accumulation, proximity to water courses, etc.

## **Building Surfaces, Decks, etc., without loose paint**

- ✓ Use high-pressure water, no soap.
- ✓ If water must be used, block storm drain inlets and contain runoff. Discharge wash water to landscaping or contain and dispose of properly.

## **Unpainted Building Surfaces, Wood Decks, etc.**

- ✓ If water must be used, block storm drain inlets and contain runoff. Discharge wash water to landscaping or contain and dispose of properly.
- ✓ Use biodegradable cleaning agents to remove deposits.
- ✓ Make sure pH is between 6.5 and 8.5 THEN discharge to landscaping (if cold water without a cleaning agent) otherwise dispose of properly.

## 2. Graffiti Cleaning

### **Graffiti Removal**

- ✓ Avoid graffiti abatement activities during rain events.
- ✓ When graffiti is removed by painting over, implement the procedures under Painting and Paint Removal in the Roads, Streets, and Highway Operation and Maintenance procedure sheet.
- ✓ Protect nearby storm drain inlets prior to removing graffiti from walls, signs, sidewalks, or other structures needing graffiti abatement. Clean up afterwards by sweeping or vacuuming thoroughly, and/or by using absorbent and properly disposing of the absorbent.



In the event of a spill or discharge to a storm drain or waterway, contact San Bernardino County Stormwater immediately: (877) WASTE18 | [sbcountystormwater.org/report](http://sbcountystormwater.org/report)

[sbcountystormwater.org](http://sbcountystormwater.org)

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# SIDEWALK, PLAZA, ENTRY MONUMENT & FOUNTAIN MAINTENANCE

- ✓ Note that care should be taken when disposing of waste since it may need to be disposed of as hazardous waste.

## OPTIONAL:

- Consider using a waterless and non-toxic chemical cleaning method for graffiti removal (e.g. gels or spray compounds).

## 3. Sidewalk Repair

### Surface Removal and Repair

- ✓ Schedule surface removal activities for dry weather if possible.
- ✓ Avoid creating excess dust when breaking asphalt or concrete.
- ✓ Take measures to protect nearby storm drain inlets prior to breaking up asphalt or concrete (e.g. place hay bales or sand bags around inlets). Clean afterwards by sweeping up material.
- ✓ Designate an area for clean up and proper disposal of excess materials.
- ✓ Remove and recycle as much of the broken pavement as possible.
- ✓ When making saw cuts in pavement, use as little water as possible. Cover each storm drain inlet with filter fabric during the sawing operation and contain the slurry by placing straw bales, sandbags, or gravel dams around the inlets. After the liquid drains shovel or vacuum the slurry, remove from site and dispose of properly.
- ✓ Always dry sweep first to clean up tracked dirt. Use a street sweeper or vacuum truck. Do not dump vacuumed liquid in storm drains. Once dry sweeping is complete, the area may be hosed down if needed. Discharge wash water to landscaping, pump to the sanitary sewer if permitted to do so or contain and dispose of properly.

### Concrete Installation and Repair

- ✓ Avoid mixing excess amounts of fresh concrete or cement mortar on-site. Only mix what is needed for the job.
- ✓ Wash concrete trucks off-site or in designated areas on-site, such that there is no discharge of concrete wash water into storm drain inlets, open ditches, streets, or other storm water conveyance structures. (See Concrete Waste Management BMP WM – 8)



In the event of a spill or discharge to a storm drain or waterway, contact San Bernardino County Stormwater immediately: (877) WASTE18 | [sbcountystormwater.org/report](http://sbcountystormwater.org/report)

[sbcountystormwater.org](http://sbcountystormwater.org)

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# SIDEWALK, PLAZA, ENTRY MONUMENT & FOUNTAIN MAINTENANCE

- ✓ Store dry and wet concrete materials under cover, protected from rainfall and runoff and away from drainage areas. After job is complete remove temporary stockpiles (asphalt materials, sand, etc.) and other materials as soon as possible.
- ✓ Return leftover materials to the transit mixer. Dispose of small amounts of excess concrete, grout, and mortar in the trash.
- ✓ When washing concrete to remove fine particles and expose the aggregate, contain the wash water for proper disposal.
- ✓ Do not wash sweepings from exposed aggregate concrete into the street or storm drain. Collect and return sweepings to aggregate base stock pile, or dispose in the trash.
- ✓ Protect applications of fresh concrete from rainfall and runoff until the material has hardened.

## 4. Litter Control

- ✓ Enforce anti-litter laws.
- ✓ Provide litter receptacles in busy, high pedestrian traffic areas of the community, at recreational facilities, and at community events.
- ✓ Cover litter receptacles and clean out frequently to prevent leaking/spillage or overflow.

### OPTIONAL:

- Post "No Littering" signs.

## 5. Fountain Maintenance

- ✓ Do not use copper-based algaecides. Control algae with chlorine or other alternatives, such as sodium bromide.
- ✓ Allow chlorine to dissipate for a few days and then recycle/reuse water by draining it gradually onto a landscaped area. Water must be tested prior to discharge to ensure that chlorine is not present (concentration must be less than 0.1 ppm).
- ✓ Contact local agency for approval to drain into sewer or storm drain.
- ✓ Avoid mixing excess amounts of fresh concrete or cement mortar on-site. Only mix what is needed for the job.



In the event of a spill or discharge to a storm drain or waterway, contact San Bernardino County Stormwater immediately: (877) WASTE18 | [sbcountystormwater.org/report](http://sbcountystormwater.org/report)

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## » For Residents

The following is a preview of the information we have available to residents. For more fact sheets, visit [sbcountystormwater.org](http://sbcountystormwater.org)

### Household Hazardous Waste Center Locations

# TOO TOXIC TO TRASH

Dispose of your **HOUSEHOLD HAZARDOUS WASTE (HHW)** at a **FREE** HHW Center near you. Examples of items collected: pesticides, fertilizers, paints, cleaners, antifreeze, batteries, motor oil, oil filters, and electronic waste.

SERVICE AREA	LOCATION	DAYS OPEN	HOURS
<b>Big Bear Lake</b> <small>(does not accept E-waste)</small>	42040 Garstin Dr. (cross: Big Bear Blvd.)	Saturdays	9 a.m. - 2 p.m.
<b>Chino</b>	5050 Schaefer Ave. (cross: 4th St.)	2 <sup>nd</sup> & 4 <sup>th</sup> Sat.	8 a.m. - 1 p.m.
<b>Fontana</b> <small>(Fontana residents only)</small>	16454 Orange Way (cross: Cypress Ave.) <small>Note: Provide a trash bill and a driver's license as proof of residency.</small>	Saturdays	8 a.m. - 12 p.m.
<b>Ontario</b>	1430 S. Cucamonga Ave. (cross: Belmont St.)	Fri. & Sat.	9 a.m. - 2 p.m.
<b>Rancho Cucamonga</b>	8794 Lion Street. (Off 9th St, between Vineyard and Hellman)	Saturdays	8 a.m. - 12 p.m.
<b>Redlands</b>	500 Kansas St. (cross: Park Ave.)	Saturdays	9:30 a.m. - 12:30 p.m.
<b>Rialto</b> <small>(does not accept E-waste)</small>	246 Willow Ave. (cross: Rialto Ave.)	2 <sup>nd</sup> & 4 <sup>th</sup> Fri. & Sat.	8 a.m. - 12 p.m.
<b>San Bernardino</b>	2824 East 'W' St., 302 (cross: Victoria Ave.)	Mon. - Fri.	9 a.m. - 4 p.m.
<b>Upland</b>	1370 N. Benson Ave. (cross: 14th St.)	Saturdays	9 a.m. - 2 p.m.



To report illegal dumping, call **(877) WASTE18**  
or visit [sbcountystormwater.org](http://sbcountystormwater.org)

Artwork Courtesy of the City of Los Angeles Stormwater Program. Printed on recycled paper.

## TAKE ONE

# VEHICLE MAINTENANCE

Oil, grease, anti-freeze and other toxic automotive fluids often make their way into the San Bernardino County storm drain system, and do not get treated before reaching the Santa Ana River. This pollutes our drinking water and contaminates waterways, making them unsafe for people and wildlife. Follow these best management practices to prevent pollution and protect public health.

## **Cleaning Auto Parts**

Scrape parts with a wire brush or use a bake oven rather than liquid cleaners. Arrange drip pans, drying racks and drain boards so that fluids are directed back into the parts washer or the fluid holding tank. Do not wash parts or equipment in a sink, parking lot, driveway or street.

## **Storing Hazardous Waste**

Keep your liquid waste segregated. Many fluids can be recycled via hazardous waste disposal companies if they are not mixed. Store all materials under cover with spill containment or inside to prevent contamination of rainwater runoff.

## **Preventing Leaks and Spills**

Conduct all vehicle maintenance inside of a garage. Place drip pans underneath vehicle to capture fluids. Use absorbent materials instead of water to clean work areas.

## **Cleaning Spills**

Use dry methods for spill cleanup (sweeping, absorbent materials). To report accidental spills into the street or storm drain call (877) WASTE18 or 911.

## **Proper Disposal of Hazardous Waste**

Dispose of household hazardous waste by taking it to your nearest household hazardous waste center. For more information, call 1-800-OILY CAT or check out [sbcountystormwater.org/Disposal.html](http://sbcountystormwater.org/Disposal.html)



In the event of a spill or discharge to a storm drain or waterway, contact San Bernardino County Stormwater immediately: (877) WASTE18 | [sbcountystormwater.org/report](http://sbcountystormwater.org/report)

[sbcountystormwater.org](http://sbcountystormwater.org)

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[sbcountystormwater.org](http://sbcountystormwater.org)

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» **eUpdates**  
[sbcountystormwater.org/newsletter](http://sbcountystormwater.org/newsletter)

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» **Facebook**  
[facebook.com/sbcountystormwater](https://facebook.com/sbcountystormwater)

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» **YouTube**  
[youtube.com/sbcountystormwater](https://youtube.com/sbcountystormwater)

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» **Report Pollution Violations**  
[sbcountystormwater.org/report](http://sbcountystormwater.org/report)

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» **Email**  
[info@sbcountystormwater.org](mailto:info@sbcountystormwater.org)

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## **Appendix 6.4 – Other Supporting Documentation**



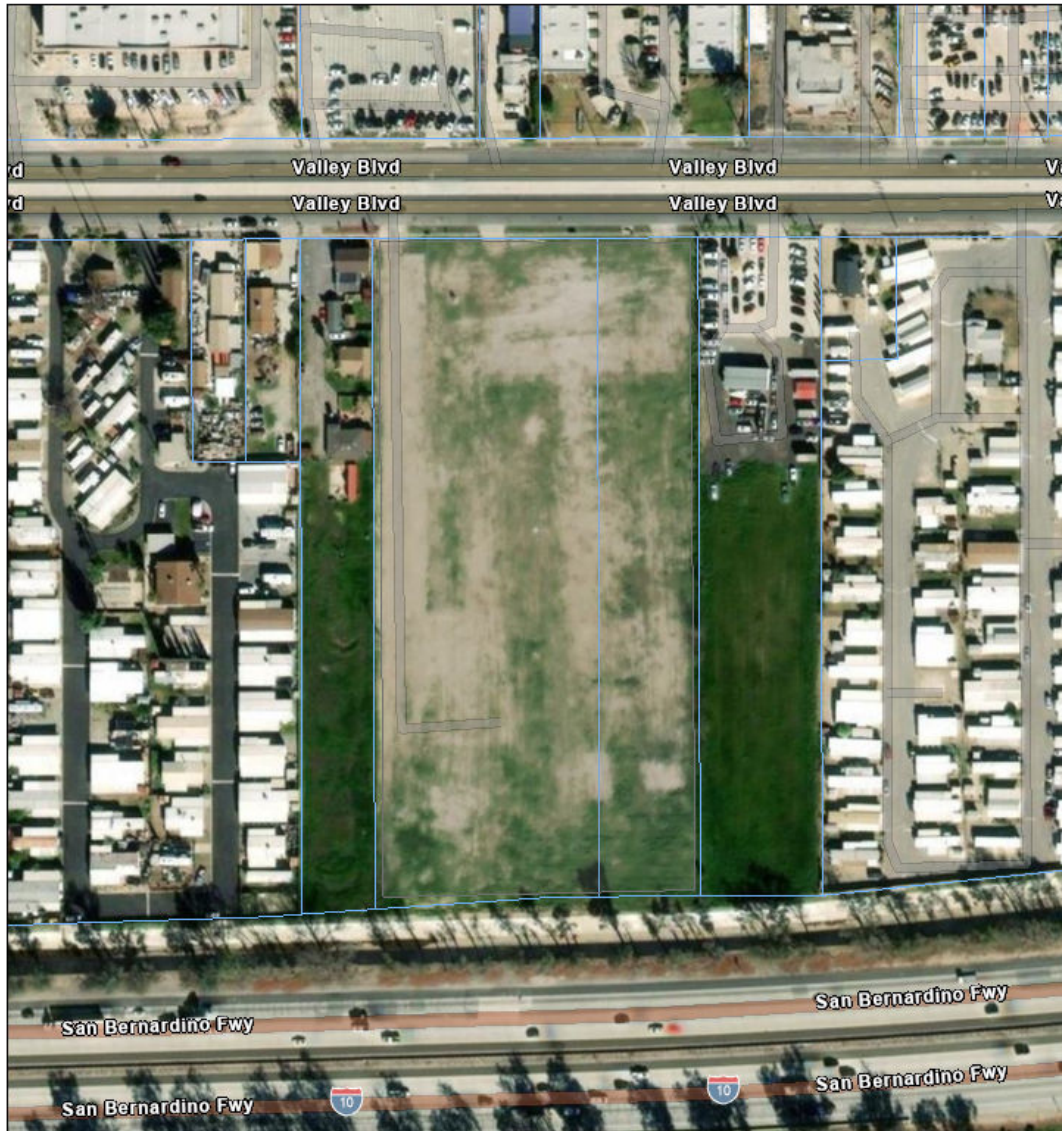


# WQMP Project Report - San Bernardino Co. Stormwater Program

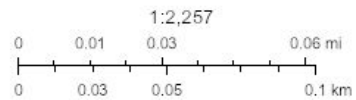
## Area of Interest (AOI) Information

Area : 246,179.88 ft<sup>2</sup>

Aug 7 2023 15:42:44 Pacific Daylight Time



 Parcels



Maxar, Microsoft, Esri Community Maps Contributors, City of Fontana, County of Riverside, County of San Bernardino, California State Parks, © OpenStreetMap, Microsoft, Esri, HERE, Garmin, SafeGraph, GeoTechnologies, Inc, METI/NASA, USGS, Bureau of Land Management.

## Project Site Parcel Numbers

#	ParcelNumber	Acreage	Area(ft <sup>2</sup> )
1	025216109	2.20	73,896.22
2	025216110	4.52	172,283.62

## HCOC Exempt Area

#	Type	Status	Area(ft <sup>2</sup> )
1	HCOC Exempt Areas	Yes	246,179.84

## Drainage Segment Details

#	System Number	Facility Name	Closest channel segment's susceptibility to Hydromodification	Highest downstream hydromodification susceptibility	Is this drainage segment subject to TMDLs?
1	0-000-0	Mulberry Channel	EHM	EHM	No
2	2-122-6A	Marigold Storm Drain	EHM	EHM	No
3	2-101-6B	East Fontana Storm Drain (Linden Ave. Storm Drain)	EHM	High	No

#	Are there downstream drainage segments subject to TMDLs?	Is this drainage segment a 303d listed stream?	Are there 303d listed streams downstream?	Area(ft <sup>2</sup> )
1	No	No	No	689.27
2	No	No	No	7,488.86
3	No	No	Yes	238,001.79

## Onsite Soil Groups

#	Onsite Soils Group	Soil Type	Soil Type Abbreviation	Area(ft <sup>2</sup> )
1	Soils - Hydro Group A	TuB TUJUNGA LOAMY SAND, 0 TO 5 PERCENT SLOPES A	TUJUNGA LOAMY SAND	246,179.84

## Ground Water Contour

#	GW_Contour	Length(ft)
1	-300	378.88

## Studies and Reports Related to Project Site

#	Report Link	Source	Date	Area(ft <sup>2</sup> )
1	<a href="#">SBVMWD High Groundwater / Pressure Zone Area</a>	USGS & San Bern Valley Municipal Water District	2005	246,179.84
2	<a href="#">Chino Basin Water Master 32nd Annual Report</a>	Chino Basin Watermaster	2008-2009	246,179.84
3	<a href="#">CSDP 3 CALC SHEET FOR HYDRO</a>	San Bernardino County Flood Control District	April 1973	246,179.84
4	<a href="#">CSDP 3-3 Rialto Channel Drainage Area Volume I</a>	James M. Montgomery	April 1988	246,179.84
5	<a href="#">CSDP 3-3 Rialto Channel Drainage Area Volume II</a>	James M. Montgomery	April 1988	246,179.84
6	<a href="#">CSDP 3-3 Rialto Channel Drainage Area Volume III</a>	James M. Montgomery	April 1988	246,179.84
7	<a href="#">CSDP 3-3 Rialto Channel Drainage Area Volume I</a>	James M. Montgomery	April 1988	246,179.84
8	<a href="#">CSDP 3-3 Rialto Channel Drainage Area Volume IV</a>	James M. Montgomery	April 1988	246,179.84
9	<a href="#">CSDP 3-3 Rialto Channel Drainage Area Volume V</a>	James M. Montgomery	April 1988	246,179.84
10	<a href="#">Chino Basin Recharge Master Plan</a>	WE, Inc	August 2001	246,179.84
11	<a href="#">Rialto MPD Vol II</a>	Hall & Foreman, Inc	February 2009	246,179.84
12	<a href="#">RS-Rialto Map Book-FINAL Layout2</a>	Hall & Foreman, Inc	February 2009	246,179.84
13	<a href="#">CSDP 3-3 Rialto Channel Drain Area Draft</a>	James M. Montgomery	January 1987	246,179.84
14	<a href="#">SBCounty CSDP Project No.2 Volume 1</a>	Moffatt & Nichol	March 1969	246,179.84
15	<a href="#">SBCounty CSDP Project No.2 Volume 2</a>	Moffatt & Nichol	March 1969	246,179.84
16	<a href="#">Volume 2 Map</a>	Moffatt & Nichol	March 1969	246,179.84
17	<a href="#">Project Report Mulberry Creek</a>	San Bernardino County Flood Control District	May 1967	246,179.84
18	<a href="#">SBCounty CSDP Project No.3 Volume I</a>	Verpet Engineering Company	May 1973	246,179.84
19	<a href="#">SBCounty CSDP Project No.3 Volume II</a>	Verpet Engineering Company	May 1973	246,179.84
20	<a href="#">Cactus Basin</a>	San Bernardino County Flood Control District	October 1985	246,179.84

Note: The information provided in this report and on the Stormwater Geodatabase for the County of San Bernardino Stormwater Program is intended to provide basic guidance in the preparation of the applicant's Water Quality Management Plan (WQMP) and should not be relied upon without independent verification.



**NOAA Atlas 14, Volume 6, Version 2**  
**Location name: Bloomington, California, USA\***  
**Latitude: 34.0694°, Longitude: -117.405°**  
**Elevation: 1109 ft\*\***



\* source: ESRI Maps  
 \*\* source: USGS

**POINT PRECIPITATION FREQUENCY ESTIMATES**

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

[PF\\_tabular](#) | [PF\\_graphical](#) | [Maps\\_&\\_aerials](#)

**PF tabular**

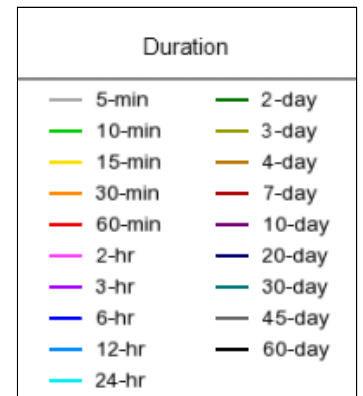
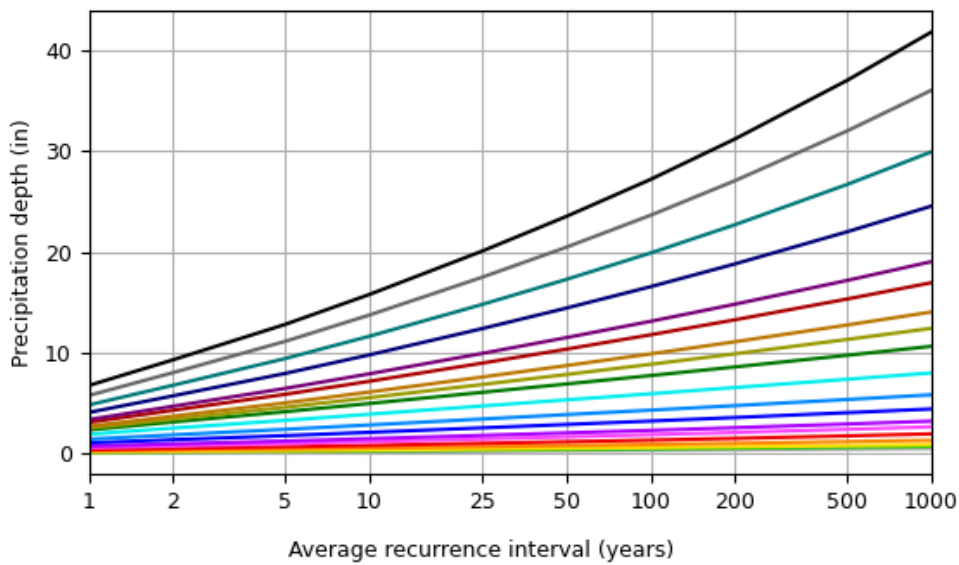
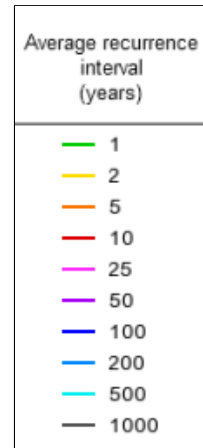
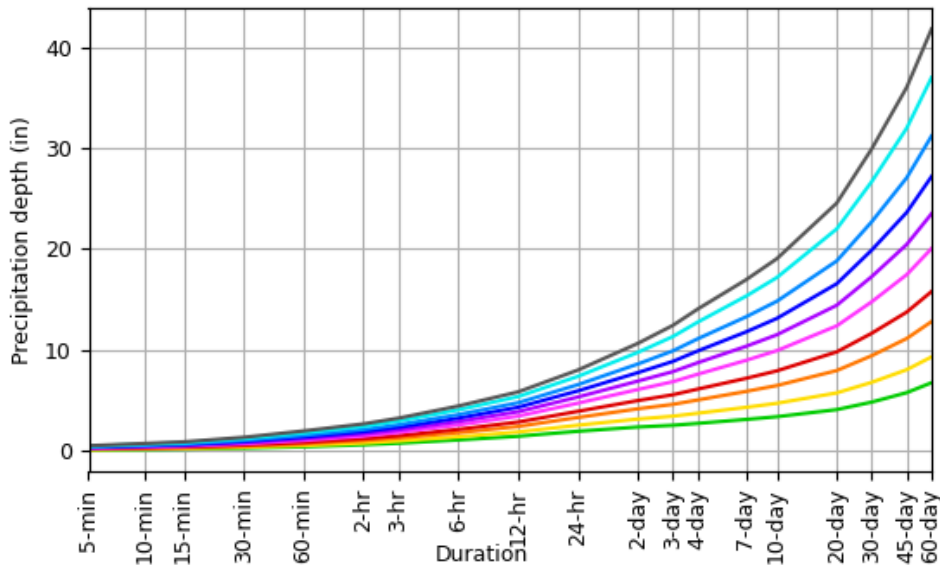
<b>PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches)<sup>1</sup></b>										
<b>Duration</b>	<b>Average recurrence interval (years)</b>									
	<b>1</b>	<b>2</b>	<b>5</b>	<b>10</b>	<b>25</b>	<b>50</b>	<b>100</b>	<b>200</b>	<b>500</b>	<b>1000</b>
<b>5-min</b>	<b>0.110</b> (0.092-0.134)	<b>0.143</b> (0.119-0.174)	<b>0.188</b> (0.156-0.228)	<b>0.225</b> (0.185-0.276)	<b>0.276</b> (0.220-0.351)	<b>0.318</b> (0.247-0.413)	<b>0.361</b> (0.274-0.481)	<b>0.407</b> (0.300-0.558)	<b>0.472</b> (0.333-0.675)	<b>0.525</b> (0.358-0.778)
<b>10-min</b>	<b>0.158</b> (0.132-0.192)	<b>0.206</b> (0.171-0.249)	<b>0.269</b> (0.223-0.327)	<b>0.322</b> (0.265-0.395)	<b>0.396</b> (0.315-0.504)	<b>0.455</b> (0.354-0.591)	<b>0.517</b> (0.392-0.689)	<b>0.583</b> (0.430-0.799)	<b>0.676</b> (0.477-0.968)	<b>0.752</b> (0.512-1.12)
<b>15-min</b>	<b>0.192</b> (0.160-0.232)	<b>0.249</b> (0.207-0.302)	<b>0.325</b> (0.270-0.396)	<b>0.389</b> (0.320-0.478)	<b>0.479</b> (0.381-0.609)	<b>0.551</b> (0.428-0.715)	<b>0.625</b> (0.474-0.833)	<b>0.705</b> (0.520-0.967)	<b>0.818</b> (0.577-1.17)	<b>0.910</b> (0.620-1.35)
<b>30-min</b>	<b>0.286</b> (0.238-0.346)	<b>0.371</b> (0.309-0.450)	<b>0.485</b> (0.402-0.590)	<b>0.581</b> (0.478-0.713)	<b>0.715</b> (0.568-0.908)	<b>0.822</b> (0.639-1.07)	<b>0.933</b> (0.708-1.24)	<b>1.05</b> (0.775-1.44)	<b>1.22</b> (0.861-1.75)	<b>1.36</b> (0.925-2.01)
<b>60-min</b>	<b>0.418</b> (0.349-0.507)	<b>0.543</b> (0.452-0.659)	<b>0.710</b> (0.589-0.864)	<b>0.850</b> (0.700-1.04)	<b>1.05</b> (0.832-1.33)	<b>1.20</b> (0.936-1.56)	<b>1.37</b> (1.04-1.82)	<b>1.54</b> (1.14-2.11)	<b>1.79</b> (1.26-2.56)	<b>1.99</b> (1.35-2.95)
<b>2-hr</b>	<b>0.616</b> (0.513-0.747)	<b>0.793</b> (0.660-0.962)	<b>1.03</b> (0.852-1.25)	<b>1.22</b> (1.00-1.50)	<b>1.48</b> (1.18-1.88)	<b>1.69</b> (1.31-2.20)	<b>1.90</b> (1.44-2.53)	<b>2.13</b> (1.57-2.92)	<b>2.44</b> (1.72-3.49)	<b>2.68</b> (1.83-3.98)
<b>3-hr</b>	<b>0.772</b> (0.643-0.936)	<b>0.991</b> (0.825-1.20)	<b>1.28</b> (1.06-1.56)	<b>1.51</b> (1.24-1.86)	<b>1.83</b> (1.46-2.33)	<b>2.08</b> (1.62-2.70)	<b>2.33</b> (1.77-3.11)	<b>2.60</b> (1.91-3.56)	<b>2.96</b> (2.09-4.24)	<b>3.25</b> (2.21-4.82)
<b>6-hr</b>	<b>1.10</b> (0.914-1.33)	<b>1.41</b> (1.17-1.71)	<b>1.81</b> (1.50-2.21)	<b>2.14</b> (1.76-2.63)	<b>2.58</b> (2.05-3.28)	<b>2.92</b> (2.27-3.78)	<b>3.26</b> (2.47-4.34)	<b>3.60</b> (2.66-4.94)	<b>4.08</b> (2.88-5.84)	<b>4.45</b> (3.03-6.60)
<b>12-hr</b>	<b>1.46</b> (1.22-1.77)	<b>1.89</b> (1.57-2.29)	<b>2.43</b> (2.02-2.96)	<b>2.87</b> (2.36-3.52)	<b>3.45</b> (2.74-4.39)	<b>3.89</b> (3.03-5.06)	<b>4.33</b> (3.29-5.77)	<b>4.78</b> (3.52-6.56)	<b>5.39</b> (3.80-7.71)	<b>5.85</b> (3.99-8.68)
<b>24-hr</b>	<b>1.96</b> (1.73-2.26)	<b>2.56</b> (2.26-2.95)	<b>3.32</b> (2.93-3.84)	<b>3.93</b> (3.44-4.59)	<b>4.74</b> (4.02-5.71)	<b>5.35</b> (4.44-6.58)	<b>5.96</b> (4.82-7.50)	<b>6.57</b> (5.18-8.51)	<b>7.39</b> (5.59-9.97)	<b>8.02</b> (5.86-11.2)
<b>2-day</b>	<b>2.38</b> (2.10-2.74)	<b>3.16</b> (2.80-3.65)	<b>4.18</b> (3.68-4.83)	<b>5.00</b> (4.37-5.82)	<b>6.09</b> (5.16-7.34)	<b>6.92</b> (5.74-8.51)	<b>7.76</b> (6.28-9.78)	<b>8.61</b> (6.79-11.2)	<b>9.76</b> (7.39-13.2)	<b>10.7</b> (7.79-14.9)
<b>3-day</b>	<b>2.55</b> (2.26-2.94)	<b>3.45</b> (3.05-3.98)	<b>4.62</b> (4.07-5.34)	<b>5.57</b> (4.87-6.50)	<b>6.86</b> (5.81-8.27)	<b>7.86</b> (6.52-9.67)	<b>8.87</b> (7.19-11.2)	<b>9.91</b> (7.81-12.8)	<b>11.3</b> (8.57-15.3)	<b>12.4</b> (9.10-17.3)
<b>4-day</b>	<b>2.74</b> (2.42-3.15)	<b>3.74</b> (3.31-4.32)	<b>5.06</b> (4.46-5.85)	<b>6.13</b> (5.36-7.15)	<b>7.60</b> (6.44-9.16)	<b>8.74</b> (7.25-10.8)	<b>9.90</b> (8.02-12.5)	<b>11.1</b> (8.76-14.4)	<b>12.8</b> (9.65-17.2)	<b>14.1</b> (10.3-19.6)
<b>7-day</b>	<b>3.13</b> (2.77-3.60)	<b>4.32</b> (3.82-4.99)	<b>5.90</b> (5.20-6.83)	<b>7.20</b> (6.30-8.39)	<b>8.98</b> (7.60-10.8)	<b>10.4</b> (8.60-12.7)	<b>11.8</b> (9.55-14.9)	<b>13.3</b> (10.5-17.2)	<b>15.3</b> (11.6-20.7)	<b>17.0</b> (12.4-23.7)
<b>10-day</b>	<b>3.40</b> (3.01-3.91)	<b>4.72</b> (4.18-5.45)	<b>6.48</b> (5.72-7.50)	<b>7.94</b> (6.94-9.26)	<b>9.94</b> (8.42-12.0)	<b>11.5</b> (9.55-14.2)	<b>13.1</b> (10.6-16.5)	<b>14.8</b> (11.7-19.2)	<b>17.2</b> (13.0-23.2)	<b>19.0</b> (13.9-26.6)
<b>20-day</b>	<b>4.10</b> (3.63-4.72)	<b>5.75</b> (5.09-6.64)	<b>7.96</b> (7.02-9.22)	<b>9.81</b> (8.58-11.4)	<b>12.4</b> (10.5-14.9)	<b>14.4</b> (12.0-17.7)	<b>16.6</b> (13.4-20.9)	<b>18.8</b> (14.8-24.4)	<b>22.0</b> (16.6-29.7)	<b>24.5</b> (18.0-34.2)
<b>30-day</b>	<b>4.84</b> (4.29-5.58)	<b>6.80</b> (6.01-7.85)	<b>9.44</b> (8.32-10.9)	<b>11.7</b> (10.2-13.6)	<b>14.8</b> (12.5-17.8)	<b>17.3</b> (14.3-21.2)	<b>19.9</b> (16.1-25.1)	<b>22.7</b> (17.9-29.4)	<b>26.7</b> (20.2-36.0)	<b>29.9</b> (21.9-41.7)
<b>45-day</b>	<b>5.78</b> (5.12-6.66)	<b>8.05</b> (7.12-9.29)	<b>11.1</b> (9.82-12.9)	<b>13.8</b> (12.0-16.0)	<b>17.5</b> (14.8-21.0)	<b>20.5</b> (17.0-25.2)	<b>23.6</b> (19.1-29.8)	<b>27.1</b> (21.3-35.0)	<b>32.0</b> (24.2-43.1)	<b>36.0</b> (26.3-50.2)
<b>60-day</b>	<b>6.76</b> (5.99-7.80)	<b>9.32</b> (8.24-10.8)	<b>12.8</b> (11.3-14.8)	<b>15.8</b> (13.8-18.4)	<b>20.0</b> (17.0-24.2)	<b>23.5</b> (19.5-28.9)	<b>27.2</b> (22.0-34.2)	<b>31.2</b> (24.6-40.4)	<b>37.0</b> (28.0-49.9)	<b>41.8</b> (30.6-58.3)

<sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

[Back to Top](#)

**PF graphical**

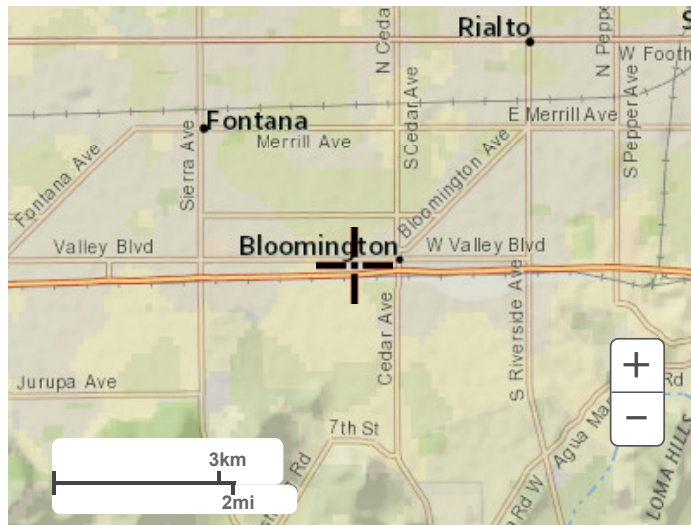
PDS-based depth-duration-frequency (DDF) curves  
 Latitude: 34.0694°, Longitude: -117.4050°



[Back to Top](#)

**Maps & aeriels**

**Small scale terrain**



Large scale terrain



Large scale map



Large scale aerial



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[National Water Center](#)  
1325 East West Highway  
Silver Spring, MD 20910  
Questions?: [HDSC.Questions@noaa.gov](mailto:HDSC.Questions@noaa.gov)

[Disclaimer](#)



**Converse Consultants**

Geotechnical Engineering  
Environmental & Groundwater Science  
Inspection & Testing Services

# GEOTECHNICAL INVESTIGATION AND WATER INFILTRATION TEST REPORT

**BLOOMINGTON ANIMAL SHELTER**  
18313 Valley Boulevard  
Bloomington Area of San Bernardino County, California

*CONVERSE PROJECT No. 22-81-206-01*



*Prepared For:*  
**MILLER ARCHITECTURAL CORPORATION**  
1177 Idaho Street, Suite 200  
Redlands, CA 92374

*Presented By:*  
**CONVERSE CONSULTANTS**  
2021 Rancho Drive, Suite 1  
Redlands, CA 92373  
909-796-0544

January 18, 2023





# Converse Consultants

Geotechnical Engineering, Environmental & Groundwater Science, Inspection & Testing Services

January 18, 2022

Mr. Gary Miller  
President/CEO  
Miller Architectural Corporation  
1177 Idaho Street, Suite 200  
Redlands, CA 92374

Subject: **GEOTECHNICAL INVESTIGATION AND WATER INFILTRATION TEST REPORT**  
**Bloomington Animal Shelter**  
18313 Valley Boulevard  
Bloomington Area of San Bernardino County, California  
Converse Project No. 22-81-206-01

Dear Mr. Miller:

Converse Consultants (Converse) is pleased to submit this geotechnical investigation and water infiltration test report to assist with the design and construction of the Bloomington Animal Shelter project located at 18313 Valley Blvd. in the Bloomington Area, San Bernardino County, California. This report was prepared in accordance with our proposal dated June 16, 2022, your Acceptance of Agreement and Authorization to Proceed dated November 3, 2022.

Based upon our field investigation, laboratory data, and analyses, the project site is considered feasible from a geotechnical standpoint, provided the recommendations presented in this report are incorporated into the design and development of the project.

We appreciate the opportunity to be of service to Miller Architectural Corporation and San Bernardino County Real Estate Services, Department of Project Management. Should you have any questions, please do not hesitate to contact us at 909-474-2847.

## CONVERSE CONSULTANTS

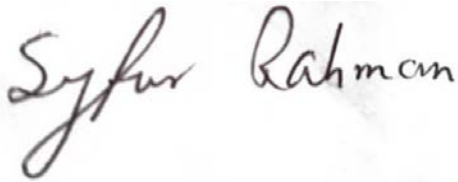
Hashmi S. E. Quazi, PhD, GE, PE  
Principal Engineer

Dist: 1-Electronic PDF/Addressee  
HSQ/SR/SM/kvg

## PROFESSIONAL CERTIFICATION

This report has been prepared by the individuals whose seals and signatures appear herein.

The findings, recommendations, specifications, or professional opinions contained in this report were prepared in accordance with generally accepted professional engineering, engineering geologic principles, and practice in this area of Southern California. There is no warranty, either expressed or implied.



---

SK Syfur Rahman, PhD, EIT  
Sr. Staff Engineer



---

Stephen McPherson  
Staff Geologist



---

Hashmi S. E. Quazi, PhD, PE, GE  
Principal Engineer



## TABLE OF CONTENTS

<b>1.0</b>	<b>INTRODUCTION .....</b>	<b>1</b>
<b>2.0</b>	<b>PROJECT DESCRIPTION .....</b>	<b>1</b>
<b>3.0</b>	<b>SITE DESCRIPTION .....</b>	<b>2</b>
<b>4.0</b>	<b>SCOPE OF WORK.....</b>	<b>3</b>
4.1	Project Set-up .....	3
4.2	Subsurface Exploration.....	4
4.3	Laboratory Testing .....	5
4.4	Analysis and Report Preparation .....	5
<b>5.0</b>	<b>SUBSURFACE CONDITIONS .....</b>	<b>5</b>
5.1	Subsurface Profile .....	5
5.2	Groundwater .....	6
5.3	Expansive Soils .....	7
5.4	Collapse Potential.....	7
5.5	Excavatability.....	8
5.6	Subsurface Variations.....	8
<b>6.0</b>	<b>ENGINEERING GEOLOGY .....</b>	<b>8</b>
6.1	Regional Geology .....	8
6.2	Local Geology .....	9
6.3	Flooding .....	9
<b>7.0</b>	<b>FAULTING AND SEISMICITY .....</b>	<b>10</b>
7.1	Faulting .....	10
7.2	CBC Seismic Design Parameters .....	11
7.3	Secondary Effects of Seismic Activity.....	12
<b>8.0</b>	<b>LABORATORY TEST RESULTS .....</b>	<b>14</b>
8.1	Physical Testing.....	14
8.2	Chemical Testing - Corrosivity Evaluation .....	15
<b>9.0</b>	<b>PERCOLATION TESTING.....</b>	<b>15</b>
<b>10.0</b>	<b>EARTHWORK RECOMMENDATIONS .....</b>	<b>15</b>
10.1	General .....	16
10.2	Remedial Grading.....	16
10.3	Engineered Fill.....	17
10.4	Compacted Fill Placement.....	17
10.5	Shrinkage and Subsidence .....	18
10.6	Site Drainage .....	18



<b>11.0</b>	<b>UTILITY TRENCH BACKFILL .....</b>	<b>19</b>
11.1	Pipe Sub-grade Preparation .....	19
11.2	Pipe Bedding .....	19
11.3	Trench Zone Backfill .....	20
<b>12.0</b>	<b>DESIGN RECOMMENDATIONS .....</b>	<b>21</b>
12.1	Shallow Foundation Design Parameters.....	21
12.2	Lateral Earth Pressures and Resistance to Lateral Loads.....	22
12.3	Slabs-on-Grade .....	23
12.4	Soil Parameters for Pipe Design.....	24
12.5	Settlement.....	24
12.6	Soil Corrosivity .....	25
12.7	Flexible Pavement Recommendations.....	26
12.8	Rigid Pavement Recommendations.....	27
12.9	Concrete Flatwork.....	28
<b>13.0</b>	<b>CONSTRUCTION RECOMMENDATIONS .....</b>	<b>28</b>
13.1	General.....	28
13.2	Temporary Sloped Excavations.....	29
<b>14.0</b>	<b>GEOTECHNICAL SERVICES DURING CONSTRUCTION.....</b>	<b>29</b>
<b>15.0</b>	<b>CLOSURE.....</b>	<b>29</b>
<b>16.0</b>	<b>REFERENCES.....</b>	<b>31</b>

### FIGURES

	<b>Following Page</b>
Figure No. 1, <i>Approximate Project Location Map</i> .....	1
Figure No. 2, <i>Approximate Boring and Percolation Test Locations Map</i> .....	4

### TABLES

	<b>Page No.</b>
Table No. 1, Summary of Borings.....	4
Table No. 2, Summary of USGS Groundwater Depth Data.....	6
Table No. 3, Collapse Potential Values .....	8
Table No. 4, Summary of Regional Faults.....	10
Table No. 5, CBC Seismic Design Parameters.....	12
Table No. 6, Summary of Corrosivity Test Results .....	15
Table No. 7, Estimated Infiltration Rates .....	15
Table No. 8, Overexcavation Depths.....	16
Table No. 9, Recommended Foundation Parameters .....	22
Table No. 10, Active and At-Rest Earth Pressures .....	22
Table No. 11, Soil Parameters for Pipe Design.....	24
Table No. 12, Correlation Between Resistivity and Corrosion .....	26



Table No. 13, Recommended Preliminary Flexible Pavement Sections ..... 26  
Table No. 14, Recommended Preliminary Rigid Pavement Sections ..... 27  
Table No. 15, Slope Ratios for Temporary Excavations ..... 29

**APPENDICES**

Appendix A.....*Field Exploration*  
Appendix B.....*Laboratory Testing Program*  
Appendix C.....*Liquefaction and Settlement Analysis*  
Appendix D.....*Percolation Testing*



## 1.0 INTRODUCTION

This report contains the findings of the geotechnical investigation performed by Converse to assist with the design and construction of the Bloomington Animal Shelter located at 18313 Valley Boulevard Bloomington Area of San Bernardino County, California. The approximate location of the project is shown in Figure No. 1, *Approximate Project Location Map*.

The purposes of this investigation were to evaluate the nature and engineering properties of the subsurface soils and groundwater conditions, and to provide geotechnical recommendations for the design and construction of the proposed project.

This report was prepared for the project described herein and is intended for use solely by Miller Architectural Corporation, San Bernardino County Real Estate Services-Project Management, and their authorized agents. This report may be made available to the prospective bidders for bidding purposes. However, the bidders are responsible for their own interpretation of the site conditions between and beyond the boring locations, based on factual data contained in this report. This report may not contain sufficient information for use by others and/or other purposes.

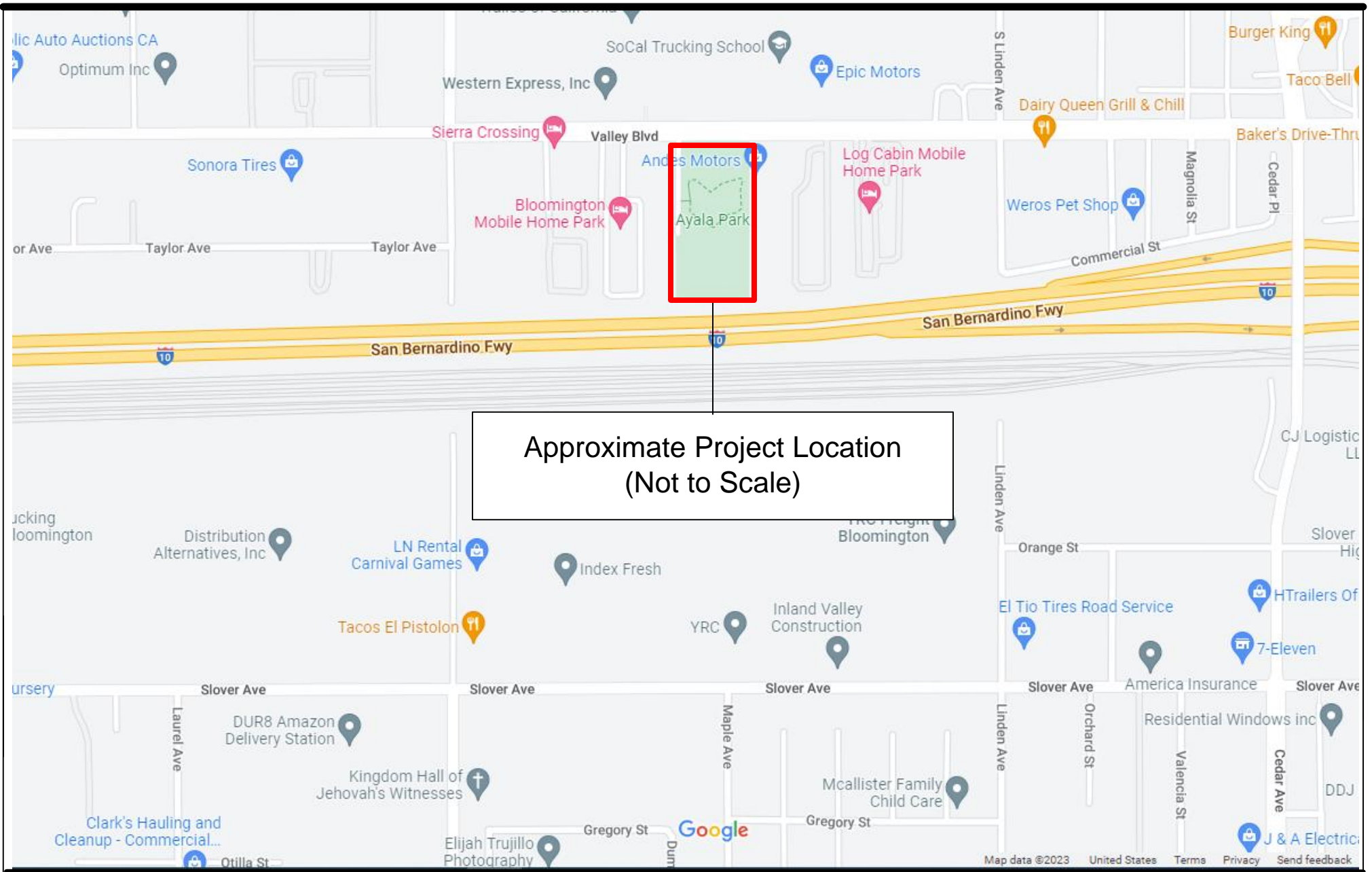
## 2.0 PROJECT DESCRIPTION

According to the information provided by Miller Architectural Corporation, the Bloomington Animal Shelter project will consist of the following.

- 16,000 square feet building which will include the following
  - Animal housing
  - Administration
- Veterinary care building
- Animal intake
- Quarantine and isolation building/private area
- Barn
- Storage building
- 3 stall garages
- Power generator building
- Euthanasia building
- 10-foot-high x 8" thick CMU wall along the Interstate freeway 10 (I-10).
- 8-foot-high x 8" thick CMU wall along the east and west property lines.
- Outdoor community events for school group, tours, and presentations
- Trash disposal
- Segregated and covered parking

We have assumed that there will also be one water infiltration device installed within the project area. Also, associated with the above-mentioned development, there will be





Project: Bloomington Animal Shelter  
 Location: 18313 Valley Boulevard  
 Bloomington area of San Bernardino County, California

For: Miller Architectural Corporation

## Approximate Project Location Map

Project No.  
 22-81-206-01

interior streets, concrete walkways, underground utilities, and landscaping. Based on the shallow relief on the site, it is anticipated that grading will consist of cuts and fills of up to about 5 feet or less.

### 3.0 SITE DESCRIPTION

The approximately 6-acre, 330' x 800' site is located in the unincorporated community of Bloomington in the San Bernardino Valley, surrounded by the cities of Rialto and Fontana in San Bernardino County, and Jurupa Valley in Riverside County. The site is bounded to the north by Valley Boulevard, to the west by residential properties, to the east by a used car lot and vacant lot and to the south by Interstate Freeway 10 (I-10).

A review of Google Maps indicates that Ayala Park was previously situated within the footprint of the proposed animal shelter location. Ayala Park had three to four enclosed structures, two gazebos, parking areas with associated access roads, a basketball court, children's play area, paved walkways, approximately fifty trees and grass covered parkland. At the time of the field investigation, all of the structures, paved areas, trees, and grassland had been removed with the exception of a utility box and the soil had been disced in preparation for the construction of the proposed Bloomington Animal shelter.

The subject site terrain is almost flat, gently slopes southward toward concrete storm drain channel along I-10. The site is presently fenced off and vacant. Photograph Nos. 1 and 2 depict the present site conditions.



*Photograph No. 1, Present site conditions facing northeast from the eastern edge of the infiltration basin.*







*Photograph No. 2, Present site conditions facing north from the proposed cats building*

## **4.0 SCOPE OF WORK**

The scope of Converse's investigation is described in the following sections.

### **4.1 Project Set-up**

We reviewed the following documents.

- Plans and documents for construction.
- Previous geologic/geotechnical publications of the site and surrounding area.
- Faulting and seismic hazard maps.
- Groundwater data.
- Aerial photographs.

As part of the project set-up, our staff performed the following.

- Prepared a geotechnical exploration plan and submitted it to Mr. Brent Adams with Miller Architectural Corporation for approval.
- Coordinated with Mr. Brent Adams for site access.
- Conducted a site reconnaissance and staked/marked the field exploration locations such that is available.
- Notified Underground Service Alert (USA) at least 48 hours prior to drilling to clear the boring locations of any conflict with existing underground utilities.
- Engaged a California-licensed driller to drill exploratory borings.



## 4.2 Subsurface Exploration

Eight borings (BH-01 through BH-08) were drilled on December 8, 2022, to investigate the subsurface conditions using a truck mounted drill rig equipped with an 8-inch diameter hollow stem auger for soil sampling. The borings were drilled to depths ranging between 5.0 and 50.0 feet below ground surface (bgs). Two test holes (PT-01 and PT-02) were drilled on December 8, 2023, to depths of 5.3 and 10.2 bgs, respectively to perform percolation testing. The boreholes were fit with perforated pipe for percolation testing that was performed on December 9, 2022.

The purpose of the borings was to:

- Estimate the extent and depths of remedial grading.
- Classify the soils within the borings.
- Collect soils samples for laboratory testing.
- Determine the excavatability of the soil.
- Perform percolation testing in two of the borings at depths of 5.3 and 10.2 feet bgs.

Details of these borings are presented in Table No. 1, *Summary of Borings*.

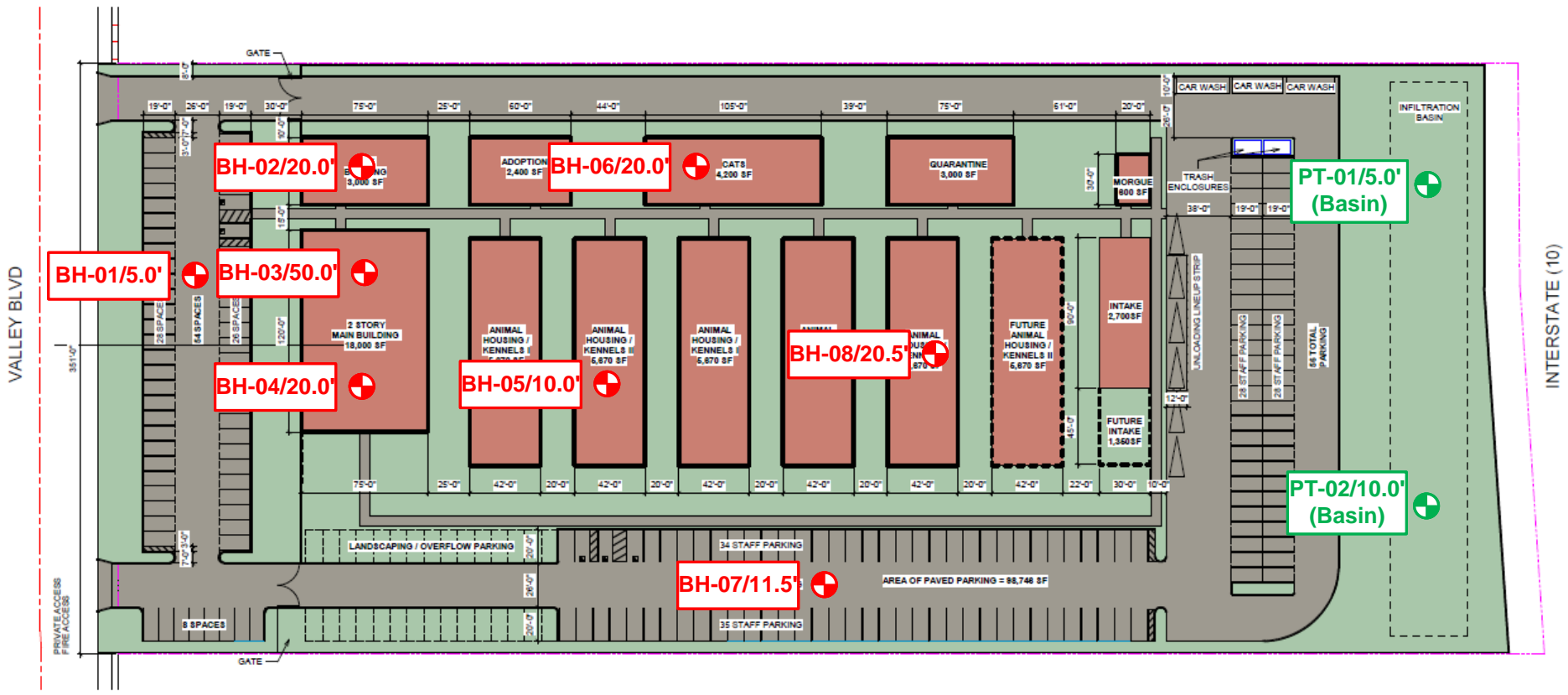
**Table No. 1, Summary of Borings**

Boring No.	Boring Depth (ft, bgs)		Groundwater Depth (ft, bgs)	Date Completed
	Proposed	Completed		
BH-01	5.0	5.0	N/E	12/8/2022
BH-02	20.0	20.0	N/E	12/8/2022
BH-03	50.0	50.0	N/E	12/8/2022
BH-04	20.0	20.0	N/E	12/8/2022
BH-05	10.0	10.0	N/E	12/8/2022
BH-06	20.0	20.0	N/E	12/8/2022
BH-07	10.0	11.5	N/E	12/8/2022
BH-08	20.0	20.5	N/E	12/8/2022
PT-01	5.0	5.3	N/E	12/8/2022
PT-02	10.0	10.2	N/E	12/8/2022

**Note:**  
 N/E = Not Encountered  
 For location of the borings, see Figure No. 2, *Approximate Boring Locations Map*.

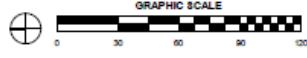
The approximate locations of the borings are shown on Figure No. 2, *Approximate Boring and Percolation Test Locations Map*. A detailed discussion of subsurface exploration is presented in Appendix A, *Field Exploration*.





1 CONCEPTUAL SITE DIAGRAM  
1" = 30'-0"

BUILDING SQUARE FOOTAGES	
NAME	SQUARE FOOTAGE
MAIN BUILDING	18,000 SF
ANIMAL HOUSING / KENNELS I	5,670 SF
ANIMAL HOUSING / KENNELS II	5,670 SF
ANIMAL HOUSING / KENNELS III	5,670 SF
ANIMAL HOUSING / KENNELS IV	5,670 SF
ANIMAL HOUSING / KENNELS V	5,670 SF
INTAKE	2,700 SF
VET BUILDING	3,000 SF
ADOPTION	2,400 SF
CATS	4,200 SF
QUARANTINE	3,000 SF
MORGUE	800 SF
TOTAL:	62,250 SF
FUTURE SQUARE FOOTAGE	
FUTURE ANIMAL HOUSING / KENNELS I	5,670 SF
FUTURE INTAKE	1,350 SF
TOTAL:	7,020 SF
GRAND TOTAL:	69,270 SF



**Legend**

**BH-04/20.5'** Number/Depth and Approximate Location of Exploratory Borings

**PT-06/10.0'** Number/Depth and Approximate Location of Percolation Tests

Project: Bloomington Animal Shelter  
 Location: 18313 Valley Boulevard  
 Bloomington area of San Bernardino County, California

# Approximate Boring and Percolation Test Locations Map

Project No.  
22-81-206-01

For: Miller Architectural Corporation

### **4.3 Laboratory Testing**

Representative samples of the site soils were tested in the laboratory to aid in soil classification, and to evaluate relevant engineering properties. These tests included the following.

- *In-situ* moisture contents and dry densities (ASTM D2216 and D2937)
- R-value (California Test 301)
- Soil corrosivity (California Test Methods 643, 422, and 417)
- Collapse potential (ASTM D4546)
- Grain size analysis (ASTM D6913)
- Maximum dry density and optimum-moisture content (ASTM D1557)
- Direct shear (ASTM D3080)
- Consolidation (ASTM D2435)

For *in-situ* moisture and dry density data, see the logs of borings in Appendix A, *Field Exploration*. For a description of the laboratory test methods and test results, see Appendix B, *Laboratory Testing Program*.

### **4.4 Analysis and Report Preparation**

Data obtained from the field exploration and laboratory testing program was assembled and evaluated. Geotechnical analyses of the compiled data were performed, followed by the preparation of this report to present our findings, conclusions, and recommendations for the proposed project.

## **5.0 SUBSURFACE CONDITIONS**

A general description of the subsurface conditions, various materials and groundwater conditions encountered at the site during our field exploration is discussed below.

### **5.1 Subsurface Profile**

Based on the exploratory borings and laboratory test results, the subsurface materials at the site primarily consist of a mixture of sand, silt, gravel and cobbles. Few to some gravels up to 3 inches in maximum dimension and cobbles up to 6 inches in maximum dimension were observed in the borings.

Discernible fill soils were not identified in our subsurface exploration; however, the site may have been previously graded for the former Ayala Park and fill soil is likely present. If present, the fill soils were likely derived from on-site sources and are similar to the native alluvial soils in composition and density.



For a detailed description of the subsurface materials encountered in the exploratory borings, see Drawings No. A-2 through A-11, *Logs of Borings*, in Appendix A, *Field Exploration*.

## 5.2 Groundwater

Groundwater was not encountered during the field investigation up to a depth of 50.0 feet bgs.

The GeoTracker database (SWRCB, 2022) was reviewed for groundwater data from sites within an approximately 1.0-mile radius of the proposed development. Results of that search are as follows.

- Merit Oil (Site No. # T0607100201), located approximately 5,200 feet northeast of the project site reported groundwater at a depth of 350 feet bgs in 2001.
- SBCFD Central Valley #76 (Site No. # T0607100439), located approximately 2,300 feet east of the project site reported groundwater depths ranging from 200 to 300 feet bgs in 1997.

The National Water Information System (USGS, 2022) was reviewed for current and historical groundwater data from sites within an approximately 1.0-mile radius of the proposed development and the results of that search are included below.

**Table No. 2, Summary of USGS Groundwater Depth Data**

Site Number	Location	Groundwater Depth Range (ft. bgs)	Date Range
340402117234601	Cedar Place south of railroad tracks; approximately 2,700 feet east of project site	240.0-288.0	1956-2001
340402117234501	Cedar Place south of railroad tracks; approximately 2,800 feet east of project site	250.0-260.81	2001-2008

The California Department of Water Resources database (DWR, 2022) was reviewed for historical groundwater data from sites within a 1.0-mile radius of the project site. One site was identified within a 1.0-mile radius of the project site that contained groundwater elevation data. Details of that record are listed below.

- Well Name Chino 1006993 (Station 340672N1173970W001), located approximately 2,800 feet east of the project site, reported groundwater at a depth ranging from 101.00 to 335.00 feet bgs in 1993.
- Well Number 01S05W22M003S (Station 340672N1173967W001), located approximately 2,800 feet east of the project site, reported groundwater at a depth ranging from 127.21 to 260.81 feet bgs between 2005 and 2008.



Based on available data, the historical high groundwater level reported at wells within approximately one mile of the site was approximately 101.00 feet bgs. Current groundwater is expected to be deeper than 101.00 feet bgs. Groundwater is not expected to be encountered during excavation or construction. It should be noted that the groundwater level could vary depending upon the seasonal precipitation and possible groundwater pumping activity in the site vicinity. Shallow perched groundwater may be present locally, particularly following precipitation.

### **5.3 Expansive Soils**

Expansive soils are characterized by their ability to undergo significant volume changes (shrink or swell) due to variations in moisture content. Changes in soil moisture content can result from precipitation, landscape irrigation, utility leakage, roof drainage, perched groundwater, drought, or other factors and may result in unacceptable settlement or heave of structures or concrete slabs supported on grade. Depending on the extent and location below finish subgrade, expansive soils can have a detrimental effect on structures.

Based on the laboratory test results, the expansion indices of the upper 5 feet soils were 0, corresponding to very low expansion potentials.

### **5.4 Collapse Potential**

Soil deposits subjected to collapse/hydro-consolidation generally exist in regions of moisture deficiency. Collapsible soils are generally defined as soils that have potential to suddenly decrease in volume upon an increase in moisture content even without an increase in external loads. Moreover, some soils may have a different degree of collapse/hydro-consolidation based on the amount of proposed fill or structure loads. Soils susceptible to collapse/hydro-consolidation include wind-blown silt, weakly cemented sand, and silt where the cementing agent is soluble (e.g., soluble gypsum, halite), alluvial or colluvial deposits within semi-arid to arid climate, and certain weathered bedrock above the groundwater table.

Granular soils may have a potential to collapse upon wetting in arid climate regions. Collapse/hydro-consolidation may occur when the soluble cements (carbonates) in the soil matrix dissolve, causing the soil to densify from its loose/low density configuration from deposition.

The degree of collapse of a soil can be defined by the collapse potential value, which is expressed as a percent of collapse of the total sample using the Collapse Potential Test (ASTM D4546). According to the ASTM guideline, the severity of collapse potential is commonly evaluated by the following Table No. 3, *Collapse Potential Values*.



**Table No. 3, Collapse Potential Values**

Collapse Potential Value (%)	Severity of Problem
0	None
0.1 to 2	Slight
2.1 to 6.0	Moderate
6.0 to 10.0	Moderately Severe
>10	Severe

Based on the laboratory test results (collapse potential of 0.6 and 1.5 percent), slight collapse potential is anticipated at the site. Collapse potential distress is typically considered a concern when collapse potential is over 2% (LA County, 2013).

### **5.5 Excavatability**

The subsurface materials at the project are expected to be excavatable by conventional heavy-duty earth moving equipment. However, Excavation will be difficult if high concentration of gravel or cobbles are encountered within the excavation depth.

The phrase “conventional heavy-duty excavation equipment” is intended to include commonly used equipment such as excavators, scrapers, and trenching machines. It does not include hydraulic hammers (“breakers”), jackhammers, blasting, or other specialized equipment and techniques used to excavate hard earth materials. Selection of an appropriate excavation equipment models should be done by an experienced earthwork contractor.

### **5.6 Subsurface Variations**

Based on results of the subsurface exploration and our experience, some variations in the continuity and nature of subsurface soil conditions within the project site should be anticipated. Because of the uncertainties involved in the nature and depositional characteristics of the earth material, care should be exercised in interpolating or extrapolating subsurface conditions between or beyond the boring locations.

## **6.0 ENGINEERING GEOLOGY**

The regional and local geology within the proposed project area is discussed below.

### **6.1 Regional Geology**

The project site lies within the northernmost portion of the Peninsular Ranges Geomorphic Province of California, near the boundary with the Transverse Ranges Province. The Peninsular Ranges Province is characterized by northwest trending



valleys and mountain ranges, which have formed in response to the regional tectonic forces along the boundary between the Pacific and North American tectonic plates. The geologic structure is dominated by northwest trending right-lateral faults, most notably, the San Andreas Fault System. The Peninsular Ranges Geomorphic Province consists of a series of northwest-trending mountain ranges and valleys bounded on the north by the San Bernardino and San Gabriel Mountains, on the west by the Los Angeles Basin, and on the southwest by the Pacific Ocean and extends southward from the Transverse Ranges into the Baja California Peninsula.

The province is a seismically active region characterized by a series of northwest-trending strike-slip faults. The most prominent of the nearby fault zones include the San Jacinto and Elsinore faults, as well as the San Gorgonio and San Andreas fault zones (CGS, 2007), all of which have been known to be active during Quaternary time.

Topography within the province is generally characterized by broad alluvial valleys separated by linear mountain ranges. This northwest-trending linear fabric is created by the regional faulting within the granitic basement rock of the Southern California Batholith. Broad, linear, alluvial valleys have been formed by erosion of these principally granitic mountain ranges.

The project site is located at the extreme northeast margin of a structural block within the Peninsular Ranges known as the Perris Block. The Perris Block is a relatively stable structural block bounded by the San Jacinto fault and Elsinore fault. The northern boundary is formed by the east-west compressional faults associated with the Transverse Ranges Physiographic Province. The southern boundary is less clearly defined.

The project site is located in an active seismic area. The active Cucamonga, San Jacinto, and San Andreas faults are located nearby. A detailed discussion on site-specific faulting and seismicity is presented in Section 7.0, Faulting and Seismicity.

## **6.2 Local Geology**

The project site is underlain by late Holocene aged young alluvial-fan deposits (Qyf<sub>5</sub>), consisting of unconsolidated to slightly consolidated coarse-grained sand having slightly dissected to undissected surfaces to alluvial deposited boulders (Morton and Miller, 2006).

## **6.3 Flooding**

Review of National Flood Insurance Rate Maps indicates that the project site is within a Flood Hazard Zone "X". The Zone "X" is designated as an "Area of Minimal Flood Hazard" (FEMA, 2008).





## 7.0 FAULTING AND SEISMICITY

The approximate distance and seismic characteristics of nearby faults as well as seismic design coefficients are presented in the following subsections.

### 7.1 Faulting

The proposed site is situated in a seismically active region. As is the case for most areas of Southern California, ground-shaking resulting from earthquakes associated with nearby and more distant faults may occur at the project site. During the life of the project, seismic activity associated with active faults can be expected to generate moderate to strong ground shaking at the site. Review of recent seismological and geophysical publications indicates that the seismic hazard for the project is high.

The project site is not located within a currently mapped State of California Earthquake Fault Zone for surface fault rupture (CGS, 2007; Riverside County, 2022). Table No. 4, *Summary of Regional Faults*, summarizes selected data of known faults capable of seismic activity within 100 kilometers of the site based on the generalized coordinates (34.0694N, 117.4053W). The data presented below was calculated using the National Seismic Hazard Maps Database (USGS, 2008) and other published geologic data.

**Table No. 4, Summary of Regional Faults**

Fault Name and Section	Closest Distance (km)	Slip Sense	Length (km)	Slip Rate (mm/year)	Maximum Magnitude
San Jacinto	8.13	strike slip	241	n/a	7.88
Cucamonga	12.42	thrust	28	5.0	6.70
S. San Andreas	16.15	strike slip	548	n/a	8.18
Cleghorn	24.06	strike slip	25	3.0	6.80
San Jose	26.81	strike slip	20	0.5	6.70
Chino, alt 1	28.8	strike slip	24	1.0	6.70
Chino, alt 2	28.87	strike slip	29	1.0	6.80
North Frontal (West)	30.18	reverse	50	1.0	7.20
Elsinore	31.39	strike slip	241	n/a	7.85
Sierra Madre	31.53	reverse	57	2.0	7.20
Sierra Madre Connected	31.53	reverse	76	2.0	7.30
Clamshell-Sawpit	44.88	reverse	16	0.5	6.70
Puente Hills (Coyote Hills)	46.81	thrust	17	0.7	6.90
Raymond	55.01	strike slip	22	1.5	6.80
San Joaquin Hills	55.99	thrust	27	0.5	7.10
Puente Hills (Santa Fe Springs)	58.7	thrust	11	0.7	6.70



Fault Name and Section	Closest Distance (km)	Slip Sense	Length (km)	Slip Rate (mm/year)	Maximum Magnitude
Helendale-So Lockhart	59.08	strike slip	114	0.6	7.40
North Frontal (East)	63.14	thrust	27	0.5	7.00
Pinto Mtn	63.18	strike slip	74	2.5	7.30
Elysian Park (Upper)	64.18	reverse	20	1.3	6.70
Puente Hills (LA)	67.57	thrust	22	0.7	7.00
Verdugo	69.46	reverse	29	0.5	6.90
Newport Inglewood Connected alt 2	69.76	strike slip	208	1.3	7.50
Newport-Inglewood, alt 1	69.88	strike slip	65	1.0	7.20
Newport Inglewood Connected alt 1	69.88	strike slip	208	1.3	7.50
Newport-Inglewood (Offshore)	71.01	strike slip	66	1.5	7.00
Hollywood	76.39	strike slip	17	1.0	6.70
Lenwood-Lockhart-Old Woman Springs	76.77	strike slip	145	0.9	7.50
Santa Monica Connected alt 2	81.29	strike slip	93	2.4	7.40
Johnson Valley (No)	83.52	strike slip	35	0.6	6.90
San Gabriel	85.28	strike slip	71	1.0	7.30
Sierra Madre (San Fernando)	85.32	thrust	18	2.0	6.70
Palos Verdes Connected	86.31	strike slip	285	3.0	7.70
Palos Verdes	86.31	strike slip	99	3.0	7.30
Landers	90.36	strike slip	95	0.6	7.40
Burnt Mtn	91.56	strike slip	21	0.6	6.80
Santa Monica, alt 1	92.97	strike slip	14	1.0	6.60
Santa Monica Connected alt 1	92.97	strike slip	79	2.6	7.30
Eureka Peak	93.39	strike slip	19	0.6	6.70
Northridge	93.61	thrust	33	1.5	6.90
So Emerson-Copper Mtn	94.56	strike slip	54	0.6	7.10
Gravel Hills-Harper Lk	99.57	strike slip	65	0.7	7.10
Coronado Bank	99.63	strike slip	186	3.0	7.40

(Source: [https://earthquake.usgs.gov/cfusion/hazfaults\\_2008\\_search/](https://earthquake.usgs.gov/cfusion/hazfaults_2008_search/))

## 7.2 CBC Seismic Design Parameters

Seismic parameters based on the 2022 California Building Code (CBC, 2022) and ASCE 7-16 are provided in the following table. These parameters were determined



using the generalized coordinates (34.0694N, 117.4053W) and the Seismic Design Maps ATC online tool.

**Table No. 5, CBC Seismic Design Parameters**

Seismic Parameters	
Site Coordinates	34.0694N, 117.4053W
Site Class	D
Risk Category	II
Mapped Short period (0.2-sec) Spectral Response Acceleration, $S_s$	1.560g
Mapped 1-second Spectral Response Acceleration, $S_1$	0.604g
Site Coefficient (from Table 1613.5.3(1)), $F_a$	1.0
Site Coefficient (from Table 1613.5.3(2)), $F_v$	1.7
MCE 0.2-sec period Spectral Response Acceleration, $S_{MS}$	1.560g
MCE 1-second period Spectral Response Acceleration, $SM_1$	1.027g
Design Spectral Response Acceleration for short period $S_{DS}$	1.040g
Design Spectral Response Acceleration for 1-second period, $S_{D1}$	0.685g
Maximum Peak Ground Acceleration, $PGA_M$	0.727g

### 7.3 Secondary Effects of Seismic Activity

In addition to ground shaking, effects of seismic activity on a project site may include surface fault rupture, soil liquefaction, landslides, lateral spreading, seismic settlement, tsunamis, seiches and earthquake-induced flooding. Results of a site-specific evaluation of each of the above secondary effects are explained below.

**Surface Fault Rupture:** The project site is not located within a currently designated State of California or San Bernardino County Hazard Map fault zone (CGS, 2007; San Bernardino County, 2019b). Based on review of existing geologic information, no major surface fault crosses through or extends toward the site. The potential for surface rupture resulting from the movement of active faults near the site is not known with certainty but is considered very low.

**Liquefaction:** Liquefaction is defined as the phenomenon in a soil mass, because of the development of excess pore pressures, soil mass suffers a substantial reduction in its shear strength. During earthquakes, excess pore pressures in saturated soil deposits may develop as a result of induced cyclic shear stresses, resulting in liquefaction. Soil liquefaction occurs in submerged granular soils during or after strong ground shaking. There are several requirements for liquefaction to occur. They are as follows.



- Soils must be submerged.
- Soils must be primarily granular.
- Soils must be contractive, that is, loose to medium-dense.
- Ground motion must be intense.
- Duration of shaking must be sufficient for the soils to lose shear resistance.

The project site is not located within a currently designated area susceptible to liquefaction (San Bernardino County, 2019b). The potential for liquefaction of the site is expected to be very low. Based on a site-specific settlement analysis presented in Appendix C, *Liquefaction and Settlement Analysis*, liquefaction settlement is negligible for the site.

**Seismic Settlement:** Dynamic dry settlement may occur in loose, granular, unsaturated soils during a large seismic event. Based on a site-specific settlement analysis presented in Appendix C, *Liquefaction and Settlement Analysis*, we estimate that the site will have the potential for up to approximately 1.4 inches of total dry seismic settlement.

**Lateral Spreading:** Seismically induced lateral spreading involves primarily lateral movement of earth materials over underlying materials which are liquefied due to ground shaking. It differs from slope failure in that complete ground failure involving large movement does not occur due to the relatively smaller gradient of the initial ground surface. Lateral spreading is demonstrated by near-vertical cracks with predominantly horizontal movement of the soil mass involved. The topography at the project site and in the immediate vicinity is very flat. Under these circumstances, the potential for lateral spreading at the subject site is considered low to moderate.

**Tsunamis:** Tsunamis are tidal waves generated in large bodies of water by fault displacement or major ground movement. Based on the inland location of the site, tsunamis do not pose a hazard to this site.

**Seiches:** Seiches are large waves generated in enclosed bodies of water in response to ground shaking. Review of the area adjacent to the site indicates that there are no significant up-gradient lakes or reservoirs with the potential of flooding the site.

**Earthquake-Induced Flooding:** This is flooding caused by failure of dams or other water-retaining structures as a result of earthquakes. Review of the California Department Of Water Resources Dam Inundation Map and the San Bernardino County Hazard Map (DWR, San Bernardino County, 2019a) indicates the site is not located in any potential inundation path of any reservoir. The potential for flooding of the site due to dam failure is considered very low.



## 8.0 LABORATORY TEST RESULTS

Laboratory testing was performed to determine the physical and chemical characteristics and engineering properties of the subsurface soils. Tests results are included in Appendix A, *Field Exploration* and Appendix B, *Laboratory Testing Program*. Discussions of the various test results are presented below.

### 8.1 Physical Testing

- In-situ Moisture and Dry Density – *In-situ* dry density and moisture content of the subsurface alluvium soils were determined in accordance with ASTM Standard D2216 and D2937. The Dry densities of the alluvial soils at the site ranged from 83.0 to 118.0 pcf with moisture contents ranging from 1 to 17 percent. Results are presented in the log of borings in Appendix A, *Field Exploration*.
- Expansion Index – Four representative bulk soil samples from the upper 5 feet of the site materials were tested in accordance with ASTM Standard D4829 to evaluate the expansion potential. The test results indicated an expansion index of 0, corresponding to very low expansion potential.
- R-Value – Two representative bulk samples were tested in accordance with Caltrans Test Method 301. The results of the R-value tests were 74 and 81.
- Collapse Potential – The collapse potential of three relatively undisturbed samples were tested in accordance with ASTM Standard D4546 under a vertical stress of up to 2.0 kips per square foot (ksf). The test results showed collapse potential of 0.6 to 1.5 percent, indicating none to slight collapse potential.
- Grain Size Analysis – Four representative samples were tested in accordance with ASTM Standard D6913 to determine the relative grain size distribution. The test results are graphically presented in Drawing No. B-1, *Grain Size Distribution Results*.
- Maximum Dry Density and Optimum Moisture Content – Typical moisture-density relationships of two representative soil samples were performed in accordance with ASTM Standard D1557. The test results are presented in Drawing No. B-2, *Moisture-Density Relationship Results*, in Appendix B, *Laboratory Testing Program*. The laboratory maximum dry density was 118.2 and 121.0 pounds per cubic feet (pcf), with optimum moisture contents of 10.5 and 8.3 percent, respectively.
- Direct Shear – Two direct shear tests were performed in accordance with ASTM Standard D3080 on relatively undisturbed ring samples. The direct shear test results are presented in Drawings No. B-3 and B-4, *Direct Shear Test Results* in Appendix B, *Laboratory Testing Program*.
- Consolidation Test – Two consolidation tests were conducted in accordance with ASTM Standard D2435 method. For test results, including sample density and moisture content, see Drawing Nos. B-5 and B-6, *Consolidation Test Results* in Appendix B, *Laboratory Testing Program*.



## 8.2 Chemical Testing - Corrosivity Evaluation

Two representative soil samples were tested to determine minimum electrical resistivity, pH, and chemical content, including soluble sulfate and chloride concentrations. The purpose of these tests was to determine the corrosion potential of site soils when placed in contact with common pipe materials. These tests were performed by AP Engineering and Testing, Inc. (Pomona, CA) in accordance with California Test Methods 643, 422, and 417. The test results are summarized on the table below and are presented in Appendix B, *Laboratory Testing Program*.

**Table No. 6, Summary of Corrosivity Test Results**

Boring No.	Depth (feet)	pH	Soluble Sulfates (CA 417) (ppm)	Soluble Chlorides (CA 422) (ppm)	Min. Resistivity (CA 643) (Ohm-cm)
BH-03	3.0-8.0	8.0	187	18	3,989
BH-07	0.0-2.0	8.1	16	17	33,110

## 9.0 PERCOLATION TESTING

Two percolation tests (PT-01 and PT-02) were performed on December 9, 2022, to evaluate water infiltration rate. The measured percolation test data and calculations are represented in Appendix D, *Percolation Testing*. The estimated and design infiltration rates at each test hole are presented in the following table.

**Table No. 7, Estimated Infiltration Rates**

Percolation Test	Approx. Depth of Boring (feet)	Predominant Soil Types (USCS)	Average Percolation Rate (inches/hour)
PT-01	5.3	Silty Sand (SM)	1.82
PT-02	10.2	Silty Sand (SM)	6.30

Based on the calculated infiltration rate during the final respective intervals in each test, a design infiltration rate of 1.82 and 6.30 (inches/hour) can be used for depth of 5 feet and 10 feet respectfully for selected percolation testing locations. Please note that infiltration rates may change if the soil type and location of the proposed system changes. If that is the case, then additional percolation testing should be performed in the required location.

## 10.0 EARTHWORK RECOMMENDATIONS

Earthwork recommendations for the project are presented in the following sections.



**10.1 General**

This section contains our general recommendations regarding earthwork and grading for the project. These recommendations are based on the results of our field exploration, laboratory tests, our experience with similar projects, and data evaluation as presented in the preceding sections. These recommendations may require modification by the geotechnical consultant based on observation of the actual field conditions during grading. Prior to the start of construction, all existing underground utilities and appurtenances should be located at the project site. Such utilities should either be protected in-place or removed and replaced during construction as required by the project specifications. All excavations should be conducted in such a manner as not to cause loss of bearing and/or lateral support of existing utilities and structure (if any).

All debris, deleterious material, artificial fill and demolished materials should be removed from the site.

The final bottom surfaces of all excavations should be observed and approved by the project geotechnical consultant prior to placing any fill. Based on these observations, localized areas may require remedial grading deeper than indicated herein. Therefore, some variations in the depth and lateral extent of excavation recommended in this report should be anticipated.

**10.2 Remedial Grading**

Structures and building footings should be uniformly supported by compacted fill. In order to provide uniform support, structural areas should be overexcavated, scarified, and recompacted as follows.

**Table No. 8, Overexcavation Depths**

Structure	Minimum Overexcavation Depth
Building Footings	18 inches below footings bottom or 3 feet below ground surface, whichever is deeper
Slab-on-Grade	15 inches below slab bottom
Pavement	12 inches below finish grade

The overexcavation should extend to at least 2 feet beyond the footprint of the footings, slabs or building foundations and at least 1 foot beyond the edge of pavement. The overexcavation bottom should be scarified and compacted as described in Section 10.4, *Compacted Fill Placement*.



If isolated pockets of very soft, loose, eroded, or pumping soil are encountered, the unstable soil should be excavated as needed to expose undisturbed, firm, and unyielding soils.

The contractor should determine the best manner to conduct the excavations, such that there are no losses of bearing and/or lateral support to the existing structures or utilities (if any).

### **10.3 Engineered Fill**

No fill should be placed until excavations and/or natural ground preparation have been observed by the geotechnical consultant. The native soils encountered within the project sites are generally considered suitable for re-use as compacted fill. Excavated soils should be processed, including removal of roots and debris, removal of oversized particles, mixing, and moisture conditioning, before placing as compacted fill. On-site soils used as fill should meet the following criteria.

- No particles larger than 3 inches in largest dimension.
- Rocks larger than one inch should not be placed within the upper 12 inches of subgrade soil.
- Free of all organic matter, debris, or other deleterious material.
- Expansion index of 30 or less.
- Sand Equivalent greater than 15 (greater than 30 for pipe bedding).
- Contain less than 30 percent by weight retained in 3/4-inch sieve.
- Contain less than 40 percent fines (passing #200 sieve).

Based on field investigation and laboratory testing results, on-site soils may be suitable as fill materials provided proper screenings will be performed to remove large sized particles to meet above mentioned criteria.

Imported materials, if required, should meet the above criteria prior to being used as compacted fill. Any imported fills should be tested and approved by the geotechnical representative prior to delivery to the sites.

### **10.4 Compacted Fill Placement**

All surfaces to receive structural fills should be scarified to a depth of 6 inches. The soil should be moisture conditioned to within  $\pm 3$  percent of optimum moisture content for coarse soils and 0 to 2 percent above optimum moisture content for fine soils. The scarified soils should be recompacted to at least 90 percent of the laboratory maximum dry density.

Fill soils should be mixed thoroughly, and moisture conditioned to within  $\pm 3$  percent of optimum moisture content for coarse soils and 0 to 2 percent above optimum moisture





content for fine soils. Fill soils should be evenly spread in horizontal lifts not exceeding 8 inches in uncompacted thickness.

All fill placed at the site should be compacted to at least 90 percent of the laboratory maximum dry densities as determined by ASTM Standard D1557 test method, unless a higher compaction is specified herein.

Fill materials should not be placed, spread or compacted during unfavorable weather conditions. When sites grading is interrupted by heavy rain, filling operations should not resume until the geotechnical consultant approves the moisture and density conditions of the previously placed fill.

### **10.5 Shrinkage and Subsidence**

The volume of excavated and recompacted soils will decrease as a result of grading. The shrinkage would depend on, among other factors, the depth of cut and/or fill, and the grading method and equipment utilized. Based on our previous experience in the other projects in close vicinity of this site, for the preliminary estimation, shrinkage factors for various units of earth material at the site may be taken as presented below.

- The shrinkage factor (defined as a percentage of soil volume reduction when moisture conditioned and compacted to the average of 92 percent relative compaction) for the alluvial soils is estimated. An average value of 10 percent may be used for preliminary earthwork planning.
- Subsidence (defined as the settlement of native materials from the equipment load applied during grading) would depend on the construction methods including type of equipment utilized. Ground subsidence is estimated to be approximately 0.1 foot to 0.15 foot.

Although these values are only approximate, they represent our best estimates of the factors to be used to calculate lost volume that may occur during grading. If more accurate shrinkage and subsidence factors are needed, it is recommended that field-testing using the actual equipment and grading techniques be conducted.

### **10.6 Site Drainage**

Adequate positive drainage should be provided away from the structures and excavation areas to prevent ponding and to reduce percolation of water into the foundation soils. A desirable drainage gradient is 1 percent for paved areas and 2 percent in landscaped areas. Surface drainage should be directed to suitable non-erosive devices.



## **11.0 UTILITY TRENCH BACKFILL**

The following sections present earthwork recommendations for utility trench backfill, including subgrade preparation and trench zone backfill.

Open cuts adjacent to existing roadways or structures are not recommended within a 1:1 (horizontal: vertical) plane extending down and away from the roadway or structure perimeter (if any).

Soils from the trench excavation should not be stockpiled more than 6 feet in height or within a horizontal distance from the trench edge equal to the depth of the trench. Soils should not be stockpiled behind the shoring, if any, within a horizontal distance equal to the depth of the trench, unless the shoring has been designed for such loads.

### **11.1 *Pipe Sub-grade Preparation***

The final subgrade surface should be level, firm, uniform, and free of loose materials and properly graded to provide uniform bearing and support to the entire section of the pipe placed on bedding material. Protruding oversize particles larger than 2 inches in dimension, if any, should be removed from the trench bottom and replaced with compacted on-sites materials.

Any loose, soft and/or unsuitable materials encountered at the pipe subgrade should be removed and replaced with an adequate bedding material. During the digging of depressions for proper sealing of the pipe joints, the pipe should rest on a prepared bottom for as near its full length as is practicable.

### **11.2 *Pipe Bedding***

Bedding is defined as the material supporting and surrounding the pipe to 1 foot above the pipe. Recommendations for pipe bedding are provided below.

To provide uniform and firm support for the pipe, compacted granular materials such as clean sand, gravel or ¾-inch crushed aggregate, or crushed rock may be used as pipe bedding material. Typically, soils with sand equivalent value of 30 or more are used as pipe bedding material. The pipe designer should determine if the soils are suitable as pipe bedding material.

The type and thickness of the granular bedding placed underneath and around the pipe, if any, should be selected by the pipe designer. The load on the rigid pipes and deflection of flexible pipes and, hence, the pipe design, depends on the type and the amount of bedding placed underneath and around the pipe.



Bedding materials should be vibrated in-place to achieve compaction. Care should be taken to densify the bedding material below the springline of the pipe. Prior to placing the pipe bedding material, the pipe subgrade should be uniform and properly graded to provide uniform bearing and support to the entire section of the pipe placed on bedding material.

Migration of fines from the surrounding native and/or fill soils must be considered in selecting the gradation of any imported bedding material. We recommend that the pipe bedding material should satisfy the following criteria to protect migration of fine materials.

- i.  $\frac{D_{15}(F)}{D_{85}(B)} \leq 5$
- ii.  $\frac{D_{50}(F)}{D_{50}(B)} < 25$
- iii. Bedding Materials must have less than 5 percent passing No. 200 sieve (0.0074 mm) to avoid internal movement of fines.

Where,

F	=	Bedding Material
B	=	Surrounding Native and/or Fill Soils
$D_{15}(F)$	=	Particle size through which 15% of bedding material will pass
$D_{85}(B)$	=	Particle size through which 85% of surrounding soil will pass
$D_{50}(F)$	=	Particle size through which 50% of bedding material will pass
$D_{50}(B)$	=	Particle size through which 50% of surrounding soil will pass

If the above criteria do not satisfy, commercially available geofabric used for filtration purposes (such as Mirafi 140N or equivalent) may be wrapped around the bedding material encasing the pipe to separate the bedding material from the surrounding native or fill soils.

### **11.3 Trench Zone Backfill**

The trench zone is defined as the portion of the trench above the pipe bedding extending up to the final grade level of the trench surface. Excavated sites soil free of oversized particles and deleterious matter may be used to backfill the trench zone. Detailed trench backfill recommendations are provided below.

- Trench excavations to receive backfill should be free of trash, debris or other unsatisfactory materials at the time of backfill placement.
- Trench zone backfill should be compacted to at least 90 percent of the laboratory maximum dry density as per ASTM D1557 test method. At least the upper 1 foot



of trench backfill underlying pavement should be compacted to at least 95 percent of the laboratory maximum dry density as per ASTM D1557 test method.

- Particles larger than 1 inch should not be placed within 12 inches of the pavement subgrade. No more than 30 percent of the backfill volume should be larger than  $\frac{3}{4}$ -inch in the largest dimension. Gravel should be well mixed with finer soil. Rocks larger than 3 inches in the largest dimension should not be placed as trench backfill.
- Trench backfill should be compacted by mechanical methods, such as sheepsfoot, vibrating or pneumatic rollers or mechanical tampers to achieve the density specified herein. The backfill materials should be brought to within  $\pm 3$  percent of optimum moisture content for coarse-grained soil, and between optimum and 2 percent above optimum for fine-grained soil, then placed in horizontal layers. The thickness of uncompacted layers should not exceed 8 inches. Each layer should be evenly spread, moistened or dried as necessary, and then tamped or rolled until the specified density has been achieved.
- The contractor should select the equipment and processes to be used to achieve the specified density without damage to adjacent ground, structures, utilities and completed work.
- The field density of the compacted soil should be measured by the ASTM D1556 (Sand Cone) or ASTM D6938 (Nuclear Gauge) or equivalent.
- Observations and field tests should be performed by the project soils consultant to confirm that the required degree of compaction has been obtained. Where compaction is less than that specified, additional compactive effort should be made with adjustment of the moisture content as necessary, until the specified compaction is obtained.
- It should be the responsibility of the contractor to maintain safe working conditions during all phases of construction.
- Trench backfill should not be placed, spread or rolled during unfavorable weather conditions. When the work is interrupted by heavy rain, fill operations should not resume until field tests by the project's geotechnical consultant indicate that the moisture content and density of the fill are in compliance with project specifications.

## 12.0 DESIGN RECOMMENDATIONS

The various design recommendations provided in this section are based on the assumption that the above earthwork and grading recommendations will be implemented in the project design and construction.

### 12.1 *Shallow Foundation Design Parameters*

The proposed pole barn and buildings may be supported on continuous or isolated spread footings. The design of the shallow foundations should be based on the recommended parameters presented in the table below.



**Table No. 9, Recommended Foundation Parameters**

Parameter	Value
Minimum continuous footing width	18 inches
Minimum isolated footing width	18 inches
Minimum continuous or isolated footing depth of embedment below lowest adjacent grade	18 inches
Allowable net bearing capacity	2,500 psf

The footing dimensions and reinforcement should be based on structural design. The allowable bearing capacity can be increased by 500 pounds per square foot (psf) with each foot of additional embedment and 100 psf with each foot of additional width up to a maximum of 3,500 psf.

The net allowable bearing values indicated above are for the dead loads and frequently applied live loads and are obtained by applying a factor of safety of 3.0 to the net ultimate bearing capacity. If normal code requirements are applied for design, the above vertical bearing value may be increased by 33 percent for short duration loadings, which will include loadings induced by wind or seismic forces.

## **12.2 Lateral Earth Pressures and Resistance to Lateral Loads**

In the following subsections, the lateral earth pressures and resistance to lateral loads are estimated by using on-site native soils strength parameters obtained from laboratory testing.

### **12.2.1 Active Earth Pressures**

The active earth pressure behind any buried wall or foundation depends primarily on the allowable wall movement, type of backfill materials, backfill slopes, wall or foundation inclination, surcharges, and any hydrostatic pressures. The lateral earth pressures for the project site are presented in the following tables.

**Table No. 10, Active and At-Rest Earth Pressures**

Loading Conditions	Lateral Earth Pressure <sup>1</sup> (psf)
Active earth conditions (wall is free to deflect at least 0.001 radian)	45
At-rest (wall is restrained)	65

These pressures assume a level ground surface around the structure for a distance greater than the structure height, no surcharge, and no hydrostatic pressure.



If water pressure is allowed to build up behind the structure, the active pressures should be reduced by 50 percent and added to a full hydrostatic pressure to compute the design pressures against the structure.

### **12.2.2 Passive Earth Pressure**

Resistance to lateral loads can be assumed to be provided by a combination of friction acting at the base of foundations and by passive earth pressure. A coefficient of friction of 0.35 between formed concrete and soil may be used with the dead load forces. An allowable passive earth pressure of 220 psf per foot of depth may be used for the sides of footings poured against recompacted soils. A factor of safety of 1.5 was applied in calculating passive earth pressure. The maximum value of the passive earth pressure should be limited to 2,500 psf for compacted fill.

Vertical and lateral bearing values indicated above are for the total dead loads and frequently applied live loads. If normal code requirements are applied for design, the above vertical bearing and lateral resistance values may be increased by 33 percent for short duration loading, which will include the effect of wind or seismic forces.

Due to the low overburden stress of the soil at shallow depth, the upper 1 foot of passive resistance should be neglected unless the soil is confined by pavement or slab.

### **12.2.3 Seismic Earth Pressure**

The seismic force applied to structural wall is based on a horizontal seismic acceleration coefficient equal to one-third of the peak ground. An equivalent fluid seismic pressure of  $24H$  pcf may be assumed under active loading conditions (regular triangular pressure distribution) where  $H$  is the height of the backfill behind the wall.

## **12.3 Slabs-on-Grade**

Slabs-on-grade should be supported on properly compacted fill. Compacted fill used to support slabs-on-grade should be placed and compacted in accordance with Section 10.4 *Compacted Fill Placement*.

Structural design elements of slabs-on-grade, including but not limited to thickness, reinforcement, joint spacing of more heavily loaded slabs will be dependent upon the anticipated loading conditions and the modulus of subgrade reaction (200 kcf) of the supporting materials and should be designed by a structural engineer.

Slabs should be designed and constructed as promulgated by the American Concrete Institute (ACI) and the Portland Cement Association (PCA). Care should be taken during concrete placement to avoid slab curling. Prior to the slab pour, all utility trenches should be properly backfilled and compacted.



Subgrade for slabs-on-grade should be firm and uniform. All loose or disturbed soils including under-slab utility trench backfill should be recompacted.

In hot weather, the contractor should take appropriate curing precautions after placement of concrete to minimize cracking or curling of the slabs. The potential for slab cracking may be lessened by the addition of fiber mesh to the concrete and/or control of the water/cement ratio.

Concrete should be cured by protecting it against loss of moisture and rapid temperature change for at least 7 days after placement. Moist curing, waterproof paper, white polyethylene sheeting, white liquid membrane compound, or a combination thereof may be used after finishing operations have been completed. The edges of concrete slabs exposed after removal of forms should be immediately protected to provide continuous curing.

#### **12.4 Soil Parameters for Pipe Design**

Structural design requires proper evaluation of all possible loads acting on pipe. The stresses and strains induced on buried pipe depend on many factors, including the type of soil, density, bearing pressure, angle of internal friction, coefficient of passive earth pressure, and coefficient of friction at the interface between the backfill and native soils. The recommended values of the various soil parameters for design are provided in the following table.

**Table No. 11, Soil Parameters for Pipe Design**

Soil Parameters	Value
Average compacted fill total unit weight (assuming 92% relative compaction), $\gamma$ (pcf)	124
Angle of internal friction of soils, $\phi$	28
Soil cohesion, $c$ (psf)	35
Coefficient of friction between concrete and native soils, $f_s$	0.35
Coefficient of friction between PVC pipe and native soils, $f_s$	0.25
Bearing pressure against native soils (psf)	2,500
Coefficient of passive earth pressure, $K_p$	2.77
Coefficient of active earth pressure, $K_a$	0.36
Modulus of Soil Reaction $E'$ (psi)	1,500

#### **12.5 Settlement**

The total settlement of shallow footings designed as recommended above, from static structural loads and short-term settlement of properly compacted fill is anticipated to be



0.5 inch or less. The static differential settlement can be taken as equal to one-half of the static total settlement over a lateral distance of 40 feet.

Our analysis of the potential dynamic settlement is presented in Appendix C, *Liquefaction and Settlement Analysis*. We estimate that the site has negligible potential for liquefaction induced settlement with up to 1.44 inches of dry seismic settlement. The soil profile across the site is relatively similar. So, we anticipate that the total settlement will be uniform. We recommend that the planned structure be designed in anticipation of dynamic differential settlement of 0.72 inches in 40 horizontal feet.

Generally, static, and dynamic settlement does not occur at the same time. For design purposes, the structural engineer should decide whether static and dynamic settlement will be combined or not.

## **12.6 Soil Corrosivity**

The results of chemical testing of a representative sample of site soils were evaluated for corrosivity evaluation with respect to common construction materials such as concrete and steel. The test results are presented in Appendix B, *Laboratory Testing Program*, Summary of Corrosivity Test Results, and are discussed below.

The sulfate contents of the soils tested correspond to American Concrete Institute (ACI) exposure category S0 for these sulfate concentration (ACI 318-14, Table 19.3.1.1) ACI recommends a minimum compressive strength of 2,500 psi for exposure category S0 in ACI 318-14, Table 19.3.2.1.

We anticipate that concrete structures such as footings, slabs, and flatwork will be exposed to moisture from precipitation and irrigation. Based on the project location and the results of chloride testing of the site soils, we do not anticipate that concrete structures will be exposed to external sources of chlorides, such as deicing chemicals, salt, brackish water, or seawater. ACI specifies exposure category C1 where concrete is exposed to moisture, but not to external sources of chlorides (ACI 318-14, Table 19.3.1.1). ACI provides concrete design recommendations in ACI 318-14, Table 19.3.2.1, including a minimum compressive strength of 2,500 psi, and a maximum chloride content of 0.3 percent.

According to Romanoff, 1957, the following table provides general guideline of soil corrosion based on electrical resistivity.





**Table No. 12, Correlation Between Resistivity and Corrosion**

Soil Resistivity (ohm-cm) per Caltrans CT 643	Corrosivity Category
Over 10,000	Mildly corrosive
2,000 – 10,000	Moderately corrosive
1,000 – 2,000	corrosive
Less than 1,000	Severe corrosive

The measured values of the minimum electrical resistivities when saturated were 3,989 and 33,110 Ohm-cm. This indicates that the soils tested are mild to moderately corrosive for ferrous metals in contact with the soils. Converse does not practice in the area of corrosion consulting. If needed, a qualified corrosion consultant should provide appropriate corrosion mitigation measures for ferrous metals in contact with the site soils.

**12.7 Flexible Pavement Recommendations**

R-values of the subgrade soils were 74 and 81. For pavement design, we have utilized an R-value of 50 and design Traffic Indices (TIs) ranging from 5 to 8.

Based on the above information, asphalt concrete and aggregate base thickness results are presented using the Caltrans Highway Design Manual (Caltrans, 2020), Chapter 630 with a safety factor of 0.2 for asphalt concrete/aggregate base section and 0.1 for full depth asphalt concrete section. Preliminary asphalt concrete pavement sections are presented in the following table below.

**Table No. 13, Recommended Preliminary Flexible Pavement Sections**

R-value	Traffic Index (TI)	Pavement Section		
		Option 1		Option 2
		Asphalt Concrete (inches)	Aggregate Base (inches)	Full AC Section (inches)
50	5	3.0	3.0	4.5
	6	3.5	3.5	5.5
	7	4.0	4.5	7.0
	8	5.0	5.0	8.5

At or near the completion of grading, subsurface samples should be tested to evaluate the actual subgrade R-value for final pavement design.

Prior to placement of aggregate base, at least 12 inches below finish grade should be overexcavated, processed and replaced as compacted fill (recompact to at least 95



percent of the laboratory maximum dry density as defined by ASTM Standard D1557 test method).

Base materials should conform with Section 200-2.2, "*Crushed Aggregate Base*," of the current Standard Specifications for Public Works Construction (SSPWC; Public Works Standards, 2021) and should be placed in accordance with Section 301.2 of the SSPWC.

Asphaltic concrete materials should conform to Section 203 of the SSPWC and should be placed in accordance with Section 302.5 of the SSPWC.

### **12.8 Rigid Pavement Recommendations**

Rigid pavement design recommendations were provided in accordance with the Portland Cement Association's (PCA) Southwest Region Publication P-14, Portland Cement Concrete Pavement (PCCP) for Light, Medium and Heavy Traffic Rigid Pavement. For pavement design, we have utilized a design subgrade R-value of 50 and design Traffic Indices (TIs) ranging from 5 to 8. We recommend that the project structural engineer consider the loading conditions at various locations and select the appropriate pavement sections from the following table:

**Table No. 14, Recommended Preliminary Rigid Pavement Sections**

Design R-Value	Design Traffic Index (TI)	PCCP Pavement Section (inches)
50	5.0	6.0
	6.0	6.5
	7.0	6.5
	8.0	7.0

The above pavement section is based on a minimum 28-day Modulus of Rupture (M-R) of 550 psi and a compressive strength of 3,750 psi. The third point method of testing beams should be used to evaluate modulus of rupture. The concrete mix design should contain a minimum cement content of 5.5 sacks per cubic yard. Recommended maximum and minimum values of slump for pavement concrete are 3.0 inches to 1.0 inch, respectively.

Transverse contraction joints should not be spaced more than 10 feet and should be cut to a depth of 1/4 the thickness of the slab. Longitudinal joints should not be spaced more than 12 feet apart. A longitudinal joint is not necessary in the pavement adjacent to the curb and gutter section.

Prior to placement of concrete, at least the upper 12.0 inches of subgrade soils below rigid pavement sections should be compacted to at least 95% relative compaction as defined by the ASTM D 1557 standard test method.



Positive drainage should be provided away from all pavement areas to prevent seepage of surface and/or subsurface water into pavement base and/or subgrade.

### **12.9 Concrete Flatwork**

Except as modified herein, concrete walks, driveways, access ramps, curb and gutters should be constructed in accordance with Section 303-5, *Concrete Curbs, Walks, Gutters, Cross-Gutters, Alley Intersections, Access Ramps, and Driveways*, of the Standard Specifications for Public Works Construction (Public Works Standards, 2021).

The subgrade soils under the above structures should consist of compacted fill placed as described in this report. Prior to placement of concrete, the upper 2 feet of subgrade soils should be moisture conditioned within 3 percent of optimum moisture content for coarse-grained soils and 0 to 2 percent above optimum for fine-grained soils.

The cement concrete thickness of driveways for passenger vehicles should be at least 4 inches, or as required by the civil or structural engineer. Transverse control joints for driveways should be spaced not more than 10 feet apart. Driveways wider than 12 feet should be provided with a longitudinal control joint.

## **13.0 CONSTRUCTION RECOMMENDATIONS**

Temporary sloped excavation recommendations are presented in the following sections.

### **13.1 General**

Prior to the start of construction, all existing underground utilities (if any) should be located at the project site. Such utilities should either be protected in-place or removed and replaced during construction as required by the project specifications.

Sloped excavations may not be feasible in locations adjacent to existing utilities, pavement, or structure (if any). Recommendations pertaining to temporary excavations are presented in this section.

Excavations near existing structures may require vertical side wall excavation. Where the side of the excavation is a vertical cut, it should be adequately supported by temporary shoring to protect workers and any adjacent structures.

All applicable requirements of the California Construction and General Industry Safety Orders, the Occupational Safety and Health Act, and the Construction Safety Act should be met. The soil exposed in cuts should be observed during excavation by the geotechnical consultant and the competent person designated by the contractor. If potentially unstable soil conditions are encountered, modifications of slope ratios for temporary cuts may be required.



### 13.2 Temporary Sloped Excavations

Temporary open-cut trenches may be constructed with side slopes as recommended in the following table. Temporary cuts encountering soft and wet fine-grained soils; dry loose, cohesionless soils or loose fill from trench backfill may have to be constructed at a flatter gradient than presented below.

**Table No. 15, Slope Ratios for Temporary Excavations**

Soil Type	OSHA Soil Type	Depth of Cut (feet)	Recommended Maximum Slope (Horizontal:Vertical) <sup>1</sup>
Silty Sand (SM), Sand with Silt and Gravel (SP-SM), Sand (SP)	C	0-10	1.5:1

<sup>1</sup> Slope ratio assumed to be uniform from top to toe of slope.

For shallow excavations up to 4 feet bgs can be vertical. For steeper temporary construction slopes or deeper excavations, or unstable soil encountered during the excavation, shoring or trench shields should be provided by the contractor to protect the workers in the excavation.

Surfaces exposed in slope excavations should be kept moist but not saturated to retard raveling and sloughing during construction. Adequate provisions should be made to protect the slopes from erosion during periods of rainfall. Surcharge loads, including construction materials, should not be placed within 5 feet of the unsupported slope edge. Stockpiled soils with a height higher than 6 feet will require greater distance from trench edges.

## 14.0 GEOTECHNICAL SERVICES DURING CONSTRUCTION

The project geotechnical consultant should review plans and specifications as the project design progresses. Such a review is necessary to identify design elements, assumptions, or new conditions which require revisions or additions to our geotechnical recommendations.

The project geotechnical consultant should be present to observe conditions during construction. Geotechnical observation and testing should be performed as needed to verify compliance with project specifications. Additional geotechnical recommendations may be required based on subsurface conditions encountered during construction.

## 15.0 CLOSURE

This report is prepared for the project described herein and is intended for use solely by Miller Architectural Corporation, San Bernardino County Real Estate Services-Project Management, and their authorized agents, to assist in the development of the proposed project. Our findings and recommendations were obtained in accordance with generally



accepted professional principles practiced in geotechnical engineering. We make no other warranty, either expressed or implied.

Converse Consultants is not responsible or liable for any claims or damages associated with interpretation of available information provided to others. Site exploration identifies actual soil conditions only at those points where samples are taken, when they are taken. Data derived through sampling and laboratory testing is extrapolated by Converse employees who render an opinion about the overall soil conditions. Actual conditions in areas not sampled may differ. In the event that changes to the project occur, or additional, relevant information about the project is brought to our attention, the recommendations contained in this report may not be valid unless these changes and additional relevant information are reviewed, and the recommendations of this report are modified or verified in writing. In addition, the recommendations can only be finalized by observing actual subsurface conditions revealed during construction. Converse cannot be held responsible for misinterpretation or changes to our recommendations made by others during construction.

As the project evolves, a continued consultation and construction monitoring by a qualified geotechnical consultant should be considered an extension of geotechnical investigation services performed to date. The geotechnical consultant should review plans and specifications to verify that the recommendations presented herein have been appropriately interpreted, and that the design assumptions used in this report are valid. Where significant design changes occur, Converse may be required to augment or modify the recommendations presented herein. Subsurface conditions may differ in some locations from those encountered in the explorations, and may require additional analyses and, possibly, modified recommendations.

Design recommendations given in this report are based on the assumption that the recommendations contained in this report are implemented. Additional consultation may be prudent to interpret Converse's findings for contractors, or to possibly refine these recommendations based upon the review of the actual site conditions encountered during construction. If the scope of the project changes, if project completion is to be delayed, or if the report is to be used for another purpose, this office should be consulted.



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# Appendix A

Field Exploration





## APPENDIX A

### FIELD EXPLORATION

Our field investigation included a site reconnaissance and a subsurface exploration program consisting of drilling soil borings and conducting percolation testing. During the site reconnaissance, the surface conditions were noted, and the borings were marked at locations approved by Mr. Brent Adams with the Miller Architectural Corporation. The approximate boring locations were established in the field using approximate distances from local streets as a guide and should be considered accurate only to the degree implied by the method used to locate them.

Eight soil borings (BH-01 through BH-08) were drilled on December 8, 2022, to investigate the subsurface conditions. The borings were drilled to depths ranging between 5.0 and 50.0 feet below ground surface (bgs).

Two test holes (PT-01 and PT-02) were drilled on December 8, 2022, within the project site to perform water percolation testing. The borings were drilled to depths of 5.3 feet and 10.2 feet below ground surface (bgs) respectively. Details about the percolation tests are presented in Appendix D, *Percolation Testing*. Details of the exploratory borings are presented in the table (No. A-1) below.

**Table No. A-1, Summary of Borings**

Boring No.	Boring Depth (ft, bgs)		Groundwater Depth (ft, bgs)	Date Completed
	Proposed	Completed		
BH-01	5.0	5.0	N/E	12/8/2022
BH-02	20.0	20.0	N/E	12/8/2022
BH-03	50.0	50.0	N/E	12/8/2022
BH-04	20.0	20.0	N/E	12/8/2022
BH-05	10.0	10.0	N/E	12/8/2022
BH-06	20.0	20.0	N/E	12/8/2022
BH-07	10.0	11.5	N/E	12/8/2022
BH-08	20.0	20.5	N/E	12/8/2022
PT-01	5.0	5.3	N/E	12/8/2022
PT-02	10.0	10.2	N/E	12/8/2022

**Note:**  
 N/E = Not Encountered  
 For location of the borings, see Figure No. 2, *Approximate Boring and Percolation Test Locations Map*.

The borings were advanced using a truck-mounted drill rig equipped with 8-inch diameter hollow-stem augers for soils sampling. Encountered materials were



continuously logged by a Converse Geologist and classified in the field by visual classification in accordance with the Unified Soil Classification System. Where appropriate, the field descriptions and classifications have been modified to reflect laboratory test results.

Relatively undisturbed samples were obtained using California Modified Samplers (2.4 inches inside diameter and 3.0 inches outside diameter) lined with thin sample rings. The steel ring sampler was driven into the bottom of the borehole with successive drops of a 140-pound driving weight falling 30 inches. Blow counts at each sample interval are presented on the boring logs. Samples were retained in brass rings (2.4 inches inside diameter and 1.0 inch in height) and carefully sealed in waterproof plastic containers for shipment to the Converse laboratory. Bulk samples of typical soil types were also obtained in plastic bags.

Standard Penetration Testing (SPT) was also performed in accordance with the ASTM Standard D1586 test using 1.4 inches inside diameter and 2.0 inches outside diameter split-barrel sampler. The mechanically driven hammer for the SPT sampler was 140 pounds, falling 30 inches for each blow. The recorded blow counts for every 6 inches for a total of 1.5 feet of sampler penetration are shown on the Logs of Borings.

The exact depths at which material changes occur cannot always be established accurately. Unless a more precise depth can be established by other means, changes in material conditions that occur between drive samples are indicated on the logs at the top of the next drive sample.

Following the completion of logging and sampling, the borings (BH-01 through BH-08) were backfilled with soil cuttings and compacted by pushing down with an auger using the drill rig weight. After completion of the percolation testing, pipes were removed from PT-01 and PT-02 and the borings were backfilled with soil cuttings and compacted. If construction is delayed, the surface of the borings may settle over time. We recommend the owner monitor the boring locations and backfill any depressions that might occur or provide protection around the boring locations to prevent trip and fall injuries from occurring near the area of any potential settlement.

For a key to soil symbols and terminology used in the boring logs, refer to Drawing No. A-1a and A-1b, *Unified Soil Classification and Key to Boring Log Symbols*. For logs of borings, see Drawings No. A-2 through A-11, *Logs of Borings*.



# SOIL CLASSIFICATION CHART

MAJOR DIVISIONS			SYMBOLS		TYPICAL DESCRIPTIONS
			GRAPH	LETTER	
COARSE GRAINED SOILS  MORE THAN 50% OF MATERIAL IS LARGER THAN NO. 200 SIEVE SIZE	GRAVEL AND GRAVELLY SOILS  MORE THAN 50% OF COARSE FRACTION RETAINED ON NO. 4 SIEVE	CLEAN GRAVELS (LITTLE OR NO FINES)		<b>GW</b>	WELL-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
		GRAVELS WITH FINES (APPRECIABLE AMOUNT OF FINES)		<b>GP</b>	POORLY-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
		CLEAN SANDS (LITTLE OR NO FINES)		<b>SW</b>	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
		SANDS WITH FINES (APPRECIABLE AMOUNT OF FINES)		<b>SP</b>	POORLY-GRADED SANDS, GRAVELLY SAND, LITTLE OR NO FINES
	SAND AND SANDY SOILS  MORE THAN 50% OF COARSE FRACTION PASSING ON NO. 4 SIEVE	CLEAN SANDS (LITTLE OR NO FINES)		<b>SM</b>	SILTY SANDS, SAND - SILT MIXTURES
		SILTS AND CLAYS  LIQUID LIMIT LESS THAN 50		<b>ML</b>	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY
SILTS AND CLAYS  LIQUID LIMIT GREATER THAN 50		<b>MH</b>	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SAND OR SILTY SOILS		
				SILTS AND CLAYS  LIQUID LIMIT GREATER THAN 50	
SILTS AND CLAYS  LIQUID LIMIT GREATER THAN 50		<b>OH</b>	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS		
				HIGHLY ORGANIC SOILS	

FIELD AND LABORATORY TESTS	
<b>C</b>	Consolidation (ASTM D 2435)
<b>CL</b>	Collapse Potential (ASTM D 4546)
<b>CP</b>	Compaction Curve (ASTM D 1557)
<b>CR</b>	Corrosion, Sulfates, Chlorides (CTM 643-99; 417; 422)
<b>CU</b>	Consolidated Undrained Triaxial (ASTM D 4767)
<b>DS</b>	Direct Shear (ASTM D 3080)
<b>EI</b>	Expansion Index (ASTM D 4829)
<b>M</b>	Moisture Content (ASTM D 2216)
<b>OC</b>	Organic Content (ASTM D 2974)
<b>P</b>	Permeability (ASTM D 2434)
<b>PA</b>	Particle Size Analysis (ASTM D 6913 [2002])
<b>PI</b>	Liquid Limit, Plastic Limit, Plasticity Index (ASTM D 4318)
<b>PL</b>	Point Load Index (ASTM D 5731)
<b>PM</b>	Pressure Meter
<b>PP</b>	Pocket Penetrometer
<b>R</b>	R-Value (CTM 301)
<b>SE</b>	Sand Equivalent (ASTM D 2419)
<b>SG</b>	Specific Gravity (ASTM D 854)
<b>SW</b>	Swell Potential (ASTM D 4546)
<b>TV</b>	Pocket Torvane
<b>UC</b>	Unconfined Compression - Soil (ASTM D 2166)
	Unconfined Compression - Rock (ASTM D 7012)
<b>UU</b>	Unconsolidated Undrained Triaxial (ASTM D 2850)
<b>UW</b>	Unit Weight (ASTM D 2937)
<b>WA</b>	Passing No. 200 Sieve

NOTE: DUAL SYMBOLS ARE USED TO INDICATE BORDERLINE SOIL CLASSIFICATIONS

## BORING LOG SYMBOLS

DRILLING METHOD SYMBOLS			
	Auger Drilling		Mud Rotary Drilling
	Dynamic Cone or Hand Driven		Diamond Core

## SAMPLE TYPE

- STANDARD PENETRATION TEST  
Split barrel sampler in accordance with ASTM D-1586-84 Standard Test Method
- DRIVE SAMPLE 2.42" I.D. sampler (CMS).
- DRIVE SAMPLE No recovery
- BULK SAMPLE
- GROUNDWATER WHILE DRILLING
- GROUNDWATER AFTER DRILLING

## UNIFIED SOIL CLASSIFICATION AND KEY TO BORING LOG SYMBOLS



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Bloomington Animal Shelter  
18313 Valley Boulevard  
Bloomington Area of San Bernardino County, California  
For: Miller Architectural Corporation

Project No. Drawing No.  
**22-81-206-01 A-1a**

### CONSISTENCY OF COHESIVE SOILS

Descriptor	Unconfined Compressive Strength (tsf)	SPT Blow Counts	Pocket Penetrometer (tsf)	CA Sampler	Torvane (tsf)	Field Approximation
Very Soft	<0.25	< 2	<0.25	<3	<0.12	Easily penetrated several inches by fist
Soft	0.25 - 0.50	2 - 4	0.25 - 0.50	3 - 6	0.12 - 0.25	Easily penetrated several inches by thumb
Medium Stiff	0.50 - 1.0	5 - 8	0.50 - 1.0	7 - 12	0.25 - 0.50	Can be penetrated several inches by thumb with moderate effort
Stiff	1.0 - 2.0	9 - 15	1.0 - 2.0	13 - 25	0.50 - 1.0	Readily indented by thumb but penetrated only with great effort
Very Stiff	2.0 - 4.0	16 - 30	2.0 - 4.0	26 - 50	1.0 - 2.0	Readily indented by thumbnail
Hard	>4.0	>30	>4.0	>50	>2.0	Indented by thumbnail with difficulty

### APPARENT DENSITY OF COHESIONLESS SOILS

Descriptor	SPT N <sub>60</sub> Value (blows / foot)	CA Sampler
Very Loose	<4	<5
Loose	4- 10	5 - 12
Medium Dense	11 - 30	13 - 35
Dense	31 - 50	36 - 60
Very Dense	>50	>60

### MOISTURE

Descriptor	Criteria
Dry	Absence of moisture, dusty, dry to the touch
Moist	Damp but no visible water
Wet	Visible free water, usually soil is below water table

### PERCENT OF PROPORTION OF SOILS

Descriptor	Criteria
Trace (fine)/ Scattered (coarse)	Particles are present but estimated to be less than 5%
Few	5 to 10%
Little	15 to 25%
Some	30 to 45%
Mostly	50 to 100%

### SOIL PARTICLE SIZE

Descriptor	Size	
Boulder	> 12 inches	
Cobble	3 to 12 inches	
Gravel	Coarse	3/4 inch to 3 inches
	Fine	No. 4 Sieve to 3/4 inch
Sand	Coarse	No. 10 Sieve to No. 4 Sieve
	Medium	No. 40 Sieve to No. 10 Sieve
	Fine	No. 200 Sieve to No. 40 Sieve
Silt and Clay	Passing No. 200 Sieve	

### PLASTICITY OF FINE-GRAINED SOILS

Descriptor	Criteria
Nonplastic	A 1/8-inch thread cannot be rolled at any water content.
Low	The thread can barely be rolled, and the lump cannot be formed when drier than the plastic limit.
Medium	The thread is easy to roll, and not much time is required to reach the plastic limit; it cannot be rerolled after reaching the plastic limit. The lump crumbles when drier than the plastic limit.
High	It takes considerable time rolling and kneading to reach the plastic limit. The thread can be rerolled several times after reaching the plastic limit. The lump can be formed without crumbling when drier than the plastic limit.

### CEMENTATION/ Induration

Descriptor	Criteria
Weak	Crumbles or breaks with handling or little finger pressure.
Moderate	Crumbles or breaks with considerable finger pressure.
Strong	Will not crumble or break with finger pressure.

**NOTE:** This legend sheet provides descriptions and associated criteria for required soil description components only. Refer to Caltrans Soil and Rock Logging, Classification, and Presentation Manual (2010), Section 2, for tables of additional soil description components and discussion of soil description and identification.

## UNIFIED SOIL CLASSIFICATION AND KEY TO BORING LOG SYMBOLS



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 For: Miller Architectural Corporation

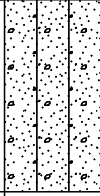


Project No. Drawing No.  
**22-81-206-01 A-1b**

# Log of Boring No. BH-01

Date Drilled: 12/8/2022      Logged by: Stephen McPherson      Checked By: Hashmi Quazi

Equipment: 8" DIAMETER HOLLOW STEM AUGER      Driving Weight and Drop: 140 lbs / 30 in

Ground Surface Elevation (ft): 1115      Depth to Water (ft, bgs): NOT ENCOUNTERED

Depth (ft)	Graphic Log	<p style="text-align: center;"><b>SUMMARY OF SUBSURFACE CONDITIONS</b></p> <p>This log is part of the report prepared by Converse for this project and should be read together with the report. This summary applies only at the location of the Boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered.</p>	SAMPLES		BLOWS	MOISTURE (%)	DRY UNIT WT. (pcf)	OTHER
			DRIVE	BULK				
5		<p><b>ALLUVIUM:</b>  <b>SILTY SAND (SM):</b> fine to coarse-grained, scattered gravel up to 1.0 inches maximum dimension, trace clay, medium dense, moist, brown.</p> <p>-@3.5': scattered gravel up to 3 inches maximum dimension.</p>			<p>10/12/15</p> <p>10/15/18</p>	<p>6</p> <p>4</p>	<p>117</p> <p>95</p>	<p>EI,R</p> <p>C</p>
		<p>End of boring at 5.0 feet bgs.                      Groundwater not encountered.                      Borehole backfilled with soil cuttings and compacted by pushing down with an auger using the drill rig weight on 12/8/2022.</p>						



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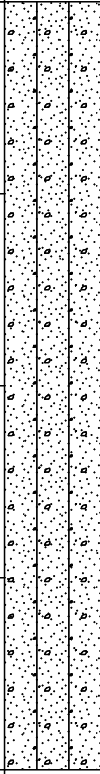
Project No. **22-81-206-01**      Drawing No. **A-2**

# Log of Boring No. BH-02

Date Drilled: 12/8/2022      Logged by: Stephen McPherson      Checked By: Hashmi Quazi

Equipment: 8" DIAMETER HOLLOW STEM AUGER      Driving Weight and Drop: 140 lbs / 30 in

Ground Surface Elevation (ft): 1110      Depth to Water (ft, bgs): NOT ENCOUNTERED

Depth (ft)	Graphic Log	SUMMARY OF SUBSURFACE CONDITIONS  This log is part of the report prepared by Converse for this project and should be read together with the report. This summary applies only at the location of the Boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered.	SAMPLES		BLOWS	MOISTURE (%)	DRY UNIT WT. (pcf)	OTHER
			DRIVE	BULK				
5		<b>ALLUVIUM:</b> <b>SILTY SAND (SM):</b> fine to coarse-grained, scattered gravel up to 0.75 inches maximum dimension, trace clay, medium dense, moist, brown.  -@4.0': scattered to few gravel up to 3 inches maximum dimension, scattered cobble up to 6 inches maximum dimension  -@7.0': very dense	12/13/20					*No Recovery
10		-@12.0': dense	29/35/36	3	84	CL		
15		-@17.0': medium dense.	19/19/22	3	112			
20		End of boring at 20.0 feet bgs. Groundwater not encountered. Borehole backfilled with soil cuttings and compacted by pushing down with an auger using the drill rig weight on 12/8/2022.	9/11/17	11	100			



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 For: Miller Architectural Corporation

Project No. **22-81-206-01**      Drawing No. **A-3**

# Log of Boring No. BH-03

Date Drilled: 12/8/2022      Logged by: Stephen McPherson      Checked By: Hashmi Quazi

Equipment: 8" DIAMETER HOLLOW STEM AUGER      Driving Weight and Drop: 140 lbs / 30 in

Ground Surface Elevation (ft): 1113      Depth to Water (ft, bgs): NOT ENCOUNTERED

Depth (ft)	Graphic Log	SUMMARY OF SUBSURFACE CONDITIONS <small>This log is part of the report prepared by Converse for this project and should be read together with the report. This summary applies only at the location of the Boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered.</small>	SAMPLES		BLOWS	MOISTURE (%)	DRY UNIT WT. (pcf)	OTHER
			DRIVE	BULK				
		<b>ALLUVIUM:</b>						EI, R, CP
		<b>SILTY SAND (SM):</b> fine to coarse-grained, scattered gravel up to 0.5 inches maximum dimension, trace clay, roots and rootlets, moist, brown.						
5		<b>SAND WITH SILT AND GRAVEL (SP-SM):</b> fine to coarse-grained, mostly gravel up to 3 inches maximum dimension, medium dense, moist, brown.			13/12/12	2	116	CR, PA
10		<b>GRAVEL WITH SILT AND SAND (GP-GM):</b> fine to coarse-grained, gravel up to 3" maximum dimension, scattered cobble up to 5" maximum dimension, dense., brown.			17/26/25			*No Recovery
15					13/40/38	5	104	
20					6/9/16	11	105	
25		<b>SILTY SAND-SANDY SILT (SM-ML):</b> fine to medium-grained, medium dense, moist, brown.			3/5/7	14		
30					7/11/17	17	113	
					4/7/9	14		



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For: Miller Architectural Corporation

Project No.  
**22-81-206-01**

Drawing No.  
**A-4a**

# Log of Boring No. BH-03

Date Drilled: 12/8/2022      Logged by: Stephen McPherson      Checked By: Hashmi Quazi

Equipment: 8" DIAMETER HOLLOW STEM AUGER      Driving Weight and Drop: 140 lbs / 30 in

Ground Surface Elevation (ft): 1113      Depth to Water (ft, bgs): NOT ENCOUNTERED

Depth (ft)	Graphic Log	SUMMARY OF SUBSURFACE CONDITIONS  This log is part of the report prepared by Converse for this project and should be read together with the report. This summary applies only at the location of the Boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered.	SAMPLES		BLOWS	MOISTURE (%)	DRY UNIT WT. (pcf)	OTHER
			DRIVE	BULK				
40	[Dotted pattern]	<p><b>ALLUVIUM:</b>  <b>SILTY SAND-SANDY SLIT (SM-ML):</b> fine to medium-grained, medium dense, moist, brown.</p> <p>-@38.0': dense.</p>	[Solid black]		9/17/27	7	117	
45	[Dotted pattern]	<p>-@48.0': very dense.</p>	[X symbol]		9/14/20	6		
50	[Dotted pattern]	<p>-@48.0': very dense.</p>	[Solid black]		12/35/48	5	116	
		<p>End of boring at 50.0 feet bgs.                      Groundwater not encountered.                      Borehole backfilled with soil cuttings and compacted by pushing down with an auger using the drill rig weight on 12/8/2022.</p>						



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 For: Miller Architectural Corporation

Project No. **22-81-206-01**      Drawing No. **A-4b**



# Log of Boring No. BH-04

Date Drilled: 12/8/2022      Logged by: Stephen McPherson      Checked By: Hashmi Quazi

Equipment: 8" DIAMETER HOLLOW STEM AUGER      Driving Weight and Drop: 140 lbs / 30 in

Ground Surface Elevation (ft): 1112      Depth to Water (ft, bgs): NOT ENCOUNTERED

Depth (ft)	Graphic Log	SUMMARY OF SUBSURFACE CONDITIONS  This log is part of the report prepared by Converse for this project and should be read together with the report. This summary applies only at the location of the Boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered.	SAMPLES		BLOWS	MOISTURE (%)	DRY UNIT WT. (pcf)	OTHER
			DRIVE	BULK				
5		<b>ALLUVIUM:</b> <b>SILTY SAND (SM):</b> fine to coarse-grained, scattered gravel up to 1 inches maximum dimension, trace clay, medium dense, moist, brown.  -@4.0': few to little gravel up to 3 inches maximum dimension, scattered cobble up to 5 inches maximum dimension  -@9.0': dense.  -@14.0': medium dense.	■		11/13/14	4	94	C
10		-@9.0': dense.	■		22/21/18	2	118	
15		-@14.0': medium dense.	■		7/10/15	7	106	
20		<b>SAND (SP):</b> fine to medium-grained, trace clay, medium dense, moist, brown.  -@19.0': very dense.	■		42/50-6"	4		*disturbed
		End of boring at 20.0 feet bgs. Groundwater not encountered. Borehole backfilled with soil cuttings and compacted by pushing down with an auger using the drill rig weight on 12/8/2022.						



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Bloomington Animal Shelter  
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 For: Miller Architectural Corporation

Project No.  
**22-81-206-01**

Drawing No.  
**A-5**

# Log of Boring No. BH-05

Date Drilled: 12/8/2022      Logged by: Stephen McPherson      Checked By: Hashmi Quazi

Equipment: 8" DIAMETER HOLLOW STEM AUGER      Driving Weight and Drop: 140 lbs / 30 in

Ground Surface Elevation (ft): 1115      Depth to Water (ft, bgs): NOT ENCOUNTERED

Depth (ft)	Graphic Log	<b>SUMMARY OF SUBSURFACE CONDITIONS</b> This log is part of the report prepared by Converse for this project and should be read together with the report. This summary applies only at the location of the Boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered.	SAMPLES		BLOWS	MOISTURE (%)	DRY UNIT WT. (pcf)	OTHER
			DRIVE	BULK				
5		<p><b>ALLUVIUM:</b>  <b>SILTY SAND (SM):</b> fine to coarse-grained, trace clay, roots and rootlets, medium dense, moist, brown.</p> <p>-@3.0': scattered to few gravel up to 3 inches maximum dimension, dense.</p> <p>-@6.0': mostly gravel up 2 inches maximum dimension.</p> <p>-@8.0': scattered gravel up to 0.75 inches maximum dimension, medium dense.</p>		<p>4/8/13</p> <p>21/31/28</p> <p>8/8/9</p>	<p>2</p> <p>2</p> <p>5</p>	<p>83</p> <p>98</p> <p>103</p>	<p>DS</p>	
10		<p>End of boring at 10.0 feet bgs.                      Groundwater not encountered.                      Borehole backfilled with soil cuttings and compacted by pushing down with an auger using the drill rig weight on 12/8/2022.</p>						



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Project No. **22-81-206-01**      Drawing No. **A-6**

# Log of Boring No. BH-06

Date Drilled: 12/8/2022      Logged by: Stephen McPherson      Checked By: Hashmi Quazi

Equipment: 8" DIAMETER HOLLOW STEM AUGER      Driving Weight and Drop: 140 lbs / 30 in

Ground Surface Elevation (ft): 1111      Depth to Water (ft, bgs): NOT ENCOUNTERED

Depth (ft)	Graphic Log	SUMMARY OF SUBSURFACE CONDITIONS  This log is part of the report prepared by Converse for this project and should be read together with the report. This summary applies only at the location of the Boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered.	SAMPLES		BLOWS	MOISTURE (%)	DRY UNIT WT. (pcf)	OTHER	
			DRIVE	BULK					
5		<b>ALLUVIUM:</b> <b>SILTY SAND (SM):</b> fine to coarse-grained, trace clay, roots and rootlets, dense, moist, brown. -@2.0': scattered gravel up to 3 inches maximum dimension  -@7.0': some gravel up to 3 inches maximum dimension, very dense.  -@12.0': dense.  -@17.0': medium dense.			9/24/33    21/36/28   21/27/31   11/8/16	1    2    1    6	107    115    117    112	CL    EI, PA	
10									
15									
20									
			End of boring at 20.0 feet bgs. Groundwater not encountered. Borehole backfilled with soil cuttings and compacted by pushing down with an auger using the drill rig weight on 12/8/2022.						



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Project No.  
**22-81-206-01**

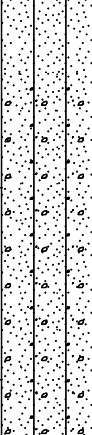
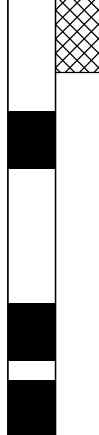

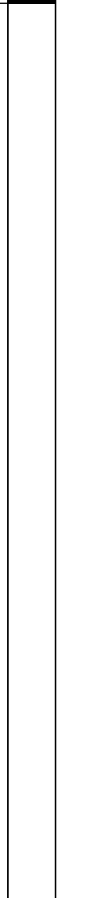
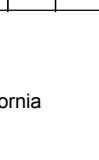
Drawing No.  
**A-7**

# Log of Boring No. BH-07

Date Drilled: 12/8/2022      Logged by: Stephen McPherson      Checked By: Hashmi Quazi

Equipment: 8" DIAMETER HOLLOW STEM AUGER      Driving Weight and Drop: 140 lbs / 30 in

Ground Surface Elevation (ft): 1112      Depth to Water (ft, bgs): NOT ENCOUNTERED

Depth (ft)	Graphic Log	SUMMARY OF SUBSURFACE CONDITIONS  This log is part of the report prepared by Converse for this project and should be read together with the report. This summary applies only at the location of the Boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered.	SAMPLES		BLOWS	MOISTURE (%)	DRY UNIT WT. (pcf)	OTHER
			DRIVE	BULK				
5		<p><b>ALLUVIUM:</b>  <b>SILTY SAND (SM):</b> fine to coarse-grained, trace clay, dense, moist, dark brown.                      little gravel up to 2.5 inches maximum dimension, roots and rootlets,.</p> <p>-@8.0': medium dense.</p> <p>-@10.0': dense.</p>		8/26/28	2	98	EI, CR, CP	
10		<p>End of boring at 11.5 feet bgs.                      Groundwater not encountered.                      Borehole backfilled with soil cuttings and compacted by pushing down with an auger using the drill rig weight on 12/8/2022.</p>		7/12/13	4	115		
				11/23/34	4	135		



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**22-81-206-01**

Drawing No.  
**A-8**

# Log of Boring No. BH-08

Date Drilled: 12/8/2022      Logged by: Stephen McPherson      Checked By: Hashmi Quazi

Equipment: 8" DIAMETER HOLLOW STEM AUGER      Driving Weight and Drop: 140 lbs / 30 in

Ground Surface Elevation (ft): 1108      Depth to Water (ft, bgs): NOT ENCOUNTERED

Depth (ft)	Graphic Log	SUMMARY OF SUBSURFACE CONDITIONS <small>This log is part of the report prepared by Converse for this project and should be read together with the report. This summary applies only at the location of the Boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered.</small>	SAMPLES		BLOWS	MOISTURE (%)	DRY UNIT WT. (pcf)	OTHER
			DRIVE	BULK				
5		<p><b>ALLUVIUM:</b>  <b>SILTY SAND (SM):</b> fine to coarse-grained, dense, moist, brown.</p> <p>-@4.0': trace clay,, roots and rootlets</p>			14/18/20	4	117	CL, DS  PA
10		<p>-@9.0': medium dense.</p>			4/6/9	6	91	
15		<p>-@14.0': caliche.</p>			5/8/12	9	83	
20		<p>-@14.0': caliche.</p>			8/9/13	6	107	
		<p>End of boring at 20.5 feet bgs.                      Groundwater not encountered.                      Borehole backfilled with soil cuttings and compacted by pushing down with an auger using the drill rig weight on 12/8/2022.</p>						



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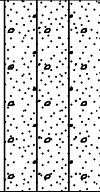

Project No. **22-81-206-01**      Drawing No. **A-9**

# Log of Boring No. PT-01

Date Drilled: 12/8/2022      Logged by: Stephen McPherson      Checked By: Hashmi Quazi

Equipment: 8" DIAMETER HOLLOW STEM AUGER      Driving Weight and Drop: N/A

Ground Surface Elevation (ft): 1101      Depth to Water (ft, bgs): NOT ENCOUNTERED

Depth (ft)	Graphic Log	<p style="text-align: center;"><b>SUMMARY OF SUBSURFACE CONDITIONS</b></p> <p>This log is part of the report prepared by Converse for this project and should be read together with the report. This summary applies only at the location of the Boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered.</p>	SAMPLES		BLOWS	MOISTURE (%)	DRY UNIT WT. (pcf)	OTHER
			DRIVE	BULK				
5		<p><b>ALLUVIUM:</b>  <b>SILTY SAND (SM):</b> fine to coarse-grained, scattered gravel up to 3 inches maximum dimension, trace clay, moist, dark brown.</p>						PA
		<p>End of boring at 5.0 feet bgs.                      Groundwater not encountered.                      Borehole fitted with perforated pipe, filter and gravel for percolation testing on 12/8/2022.                      Upon completion of percolation testing, pipe was removed and borehole was backfilled with soil cuttings and compacted on 12/9/2022.</p>						



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Project No.  
**22-81-206-01**


Drawing No.  
**A-10**

# Log of Boring No. PT-02

Date Drilled: 12/8/2022      Logged by: Stephen McPherson      Checked By: Hashmi Quazi

Equipment: 8" DIAMETER HOLLOW STEM AUGER      Driving Weight and Drop: N/A

Ground Surface Elevation (ft): 1103      Depth to Water (ft, bgs): NOT ENCOUNTERED

Depth (ft)	Graphic Log	<p style="text-align: center;"><b>SUMMARY OF SUBSURFACE CONDITIONS</b></p> <p>This log is part of the report prepared by Converse for this project and should be read together with the report. This summary applies only at the location of the Boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered.</p>	SAMPLES		BLOWS	MOISTURE (%)	DRY UNIT WT. (pcf)	OTHER
			DRIVE	BULK				
5		<p><b>ALLUVIUM:</b></p> <p><b>SILTY SAND (SM):</b> fine to coarse-grained, few gravel up to 3" maximum dimension, trace clay, moist, dark brown.</p> <p>-@9.0': scattered to few gravel up to 0.75" maximum dimension.</p>						
10		<p>End of boring at 10.0 feet bgs. Groundwater not encountered. Borehole fitted with perforated pipe, filter and gravel for percolation testing on 12/8/2022. Upon completion of percolation testing, pipe was removed and borehole was backfilled with soil cuttings and compacted on 12/9/2022.</p>						



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Project No. **22-81-206-01**      Drawing No. **A-11**

# Appendix B

Laboratory Testing Program





## APPENDIX B

### LABORATORY TESTING PROGRAM

Tests were conducted in our laboratory on representative soil samples for the purpose of classification and evaluation of their physical properties and engineering characteristics. The amount and selection of tests were based on the geotechnical parameters required for this project. Test results are presented herein and on the Logs of Borings, in Appendix A, *Field Exploration*. The following is a summary of the various laboratory tests conducted for this project.

#### **In-Situ Moisture Content and Dry Density**

In-situ dry density and moisture content tests were performed in accordance with ASTM Standard D2216 and D2937 on relatively undisturbed ring samples to aid soils classification and to provide qualitative information on strength and compressibility characteristics of the site soils. For test results, see the Logs of Borings in Appendix A, *Field Exploration*.

#### **Expansion Index**

Four representative bulk samples were tested in accordance with ASTM Standard D4829 to evaluate the expansion potential of materials encountered at the site. The test results are presented in the following table.

**Table No. B-1, Expansion Index Test Results**

Boring No.	Depth (feet)	Soil Description	Expansion Index	Expansion Potential
BH-01	0.0-5.0	Silty Sand (SM)	0	Very Low
BH-03	0.0-3.0	Silty Sand (SM)	0	Very Low
BH-06	2.0-7.0	Silty Sand (SM)	0	Very Low
BH-07	0.0-2.0	Silty Sand (SM)	0	Very Low

#### **R-value**

Two representative bulk soil samples were tested in accordance with California Test Method CT301 for resistance value (R-value). The test provides a relative measure of soil strength for use in pavement design. The test results are presented in the following table.

**Table No. B-2, R-Value Test Result**

Boring No.	Depth (feet)	Soil Classification	Measured R-value
BH-01*	0.0-5.0	Silty Sand (SM)	81
BH-03*	0.0-3.0	Silty Sand (SM)	74

\* Since the R-Values were slightly higher than usual range of R-Value for similar soil type, a design R-Value of 50 was used.



**Soil Corrosivity**

Two representative soil samples were tested in accordance with Caltrans Test Methods 643, 422 and 417 to determine minimum electrical resistivity, pH, and chemical content, including soluble sulfate and chloride concentrations. The purpose of these tests was to determine the corrosion potential of site soils when placed in contact with common construction materials. The tests were performed by AP Engineering and Testing, Inc. (Pomona, CA). Test results are presented in the following table.

**Table No. B-3, Summary of Soil Corrosivity Test Results**

Boring No.	Depth (feet)	pH	Soluble Sulfates (CA 417) (ppm)	Soluble Chlorides (CA 422) (ppm)	Min. Resistivity (CA 643) (Ohm-cm)
BH-03	3.0-8.0	8.0	187	18	3,989
BH-07	0.0-2.0	8.1	16	17	33,110

**Collapse**

To evaluate the moisture sensitivity (collapse/swell potential) of the encountered soils, three collapse tests were performed in accordance with the ASTM Standard D4546 laboratory procedure. The samples were loaded to approximately 2 kips per square foot (ksf), allowed to stabilize under load, and then submerged. The test results are presented in the following table.

**Table No. B-4, Collapse Test Results**

Boring No.	Depth (feet)	Soil Classification	Percent Swell (+) Percent Collapse (-)	Collapse Potential
BH-02	7.0-8.5	Silty Sand (SM)	-0.6	Slight
BH-06	2.0-3.5	Silty Sand (SM)	-0.6	Slight
BH-08	4.0-5.5	Silty Sand (SM)	-1.5	Slight

**Grain-Size Analyses**

To assist in soil classification, mechanical grain-size analyses were performed on four select samples in accordance with the ASTM Standard D6913. Grain-size curves are shown in Drawing No. B-1, *Grain Size Distribution Results*.



**Table No. B-5, Grain Size Distribution Test Results**

Boring No./Report	Depth (ft)	Soil Classification	% Gravel	% Sand	%Silt	%Clay
BH-03	3.0-8.0	Sand with Silt and Gravel (SP-SM)	39.0	49.7	11.3	
BH-06	2.0-7.0	Silty Sand (SM)	13.0	54.1	32.9	
BH-08	4.0-9.0	Silty Sand (SM)	6.0	57.6	36.4	
PT-01	0.0-5.0	Silty Sand (SM)	8.0	67.9	24.1	

**Maximum Dry Density and Optimum Moisture Content**

Laboratory maximum dry density-optimum moisture content relationship tests were performed on two representative bulk samples in accordance with the ASTM Standard D1557. The test results are presented in Drawing No. B-2, *Summary of Moisture-Density Relationship Results*, and are summarized in the following table.

**Table No B-6, Summary of Moisture-Density Relationship Results**

Boring No.	Depth (feet)	Soil Description	Optimum Moisture (%)	Maximum Density (lb/cft)
BH-03	0.0-3.0	Silty Sand (SM), Brown	10.5	118.2
BH-07	0.0-2.0	Silty Sand (SM), Brown	8.3	121.0

**Direct Shear**

One direct shear test was performed in accordance with ASTM Standard D3080 on relatively undisturbed samples in soaked moisture condition. One direct shear test was performed in accordance with ASTM Standard D3080 on remolded samples in soaked moisture condition. For each test, three samples contained in brass sampler rings were placed, one at a time, directly into the test apparatus and subjected to a range of normal loads appropriate for the anticipated conditions. The samples were then sheared at a constant strain rate of 0.02 inch/minute. Shear deformation was recorded until a maximum of about 0.25-inch shear displacement was achieved. Ultimate strength was selected from the shear-stress deformation data and plotted to determine the shear strength parameters. For test data, including sample density and moisture content, see Drawings No. B-3 and B-4, *Summary of Direct Shear Test Results*, and the following table.



**Table No. B-7, Summary of Direct Shear Test Results**

Boring No.	Depth (feet)	Soil Description	Peak Strength Parameters	
			Friction Angle (degrees)	Cohesion (psf)
BH-05	8.0-9.5	Silty Sand (SM)	28	70
*BH-08	4.0-5.5	Silty Sand (SM)	30	160

(\*Remolded to 90% of laboratory maximum dry density.)

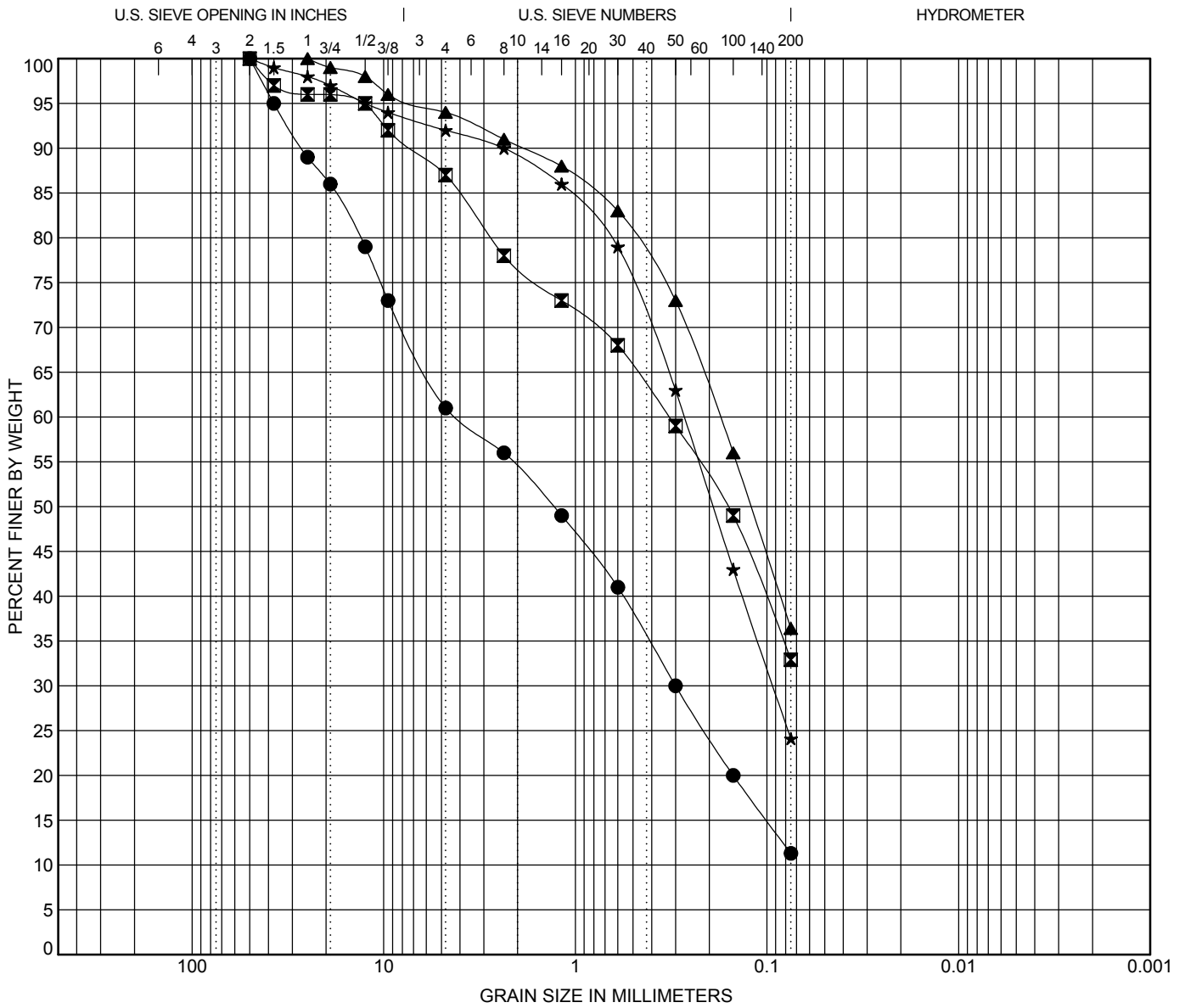
**Consolidation**

Two consolidation tests were conducted in accordance with ASTM Standard D2435 method. Data obtained from the test performed on one relatively undisturbed ring sample was used to evaluate the settlement characteristics of the on-site soils under load. Preparation for the test involved trimming the sample, placing it in a 1-inch-high brass ring, and loading it into the test apparatus, which contained porous stones to accommodate drainage during testing. Normal axial loads were applied to one end of the sample through the porous stones, and the resulting deflections were recorded at various time periods. The load was increased after the sample reached a reasonable state of equilibrium. Normal loads were applied at a constant load-increment ratio, successive loads being generally twice the preceding load. For test results, including sample density and moisture content, see Drawing Nos. B-5 and B-6, Consolidation Test Results.

**Sample Storage**

Soil samples presently stored in our laboratory will be discarded 30 days after the date of this report, unless this office receives a specific request to retain the samples for a longer period.





COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Boring No.	Depth (ft)	Description	LL	PL	PI	Cc	Cu
● BH-03	3.0-8.0	SAND WITH SILT AND GRAVEL (SP-SM)				0.32	61.07
☒ BH-06	2.0-7.0	SILTY SAND (SM)					
▲ BH-08	4.0-9.0	SILTY SAND (SM)					
★ PT-01	0.0-5.0	SILTY SAND (SM)					

Boring No.	Depth (ft)	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
● BH-03	3.0-8.0	50	4.13	0.3		39.0	49.7	11.3	
☒ BH-06	2.0-7.0	50	0.324			13.0	54.1	32.9	
▲ BH-08	4.0-9.0	25	0.177			6.0	57.6	36.4	
★ PT-01	0.0-5.0	50	0.27	0.093		8.0	67.9	24.1	

## GRAIN SIZE DISTRIBUTION RESULTS

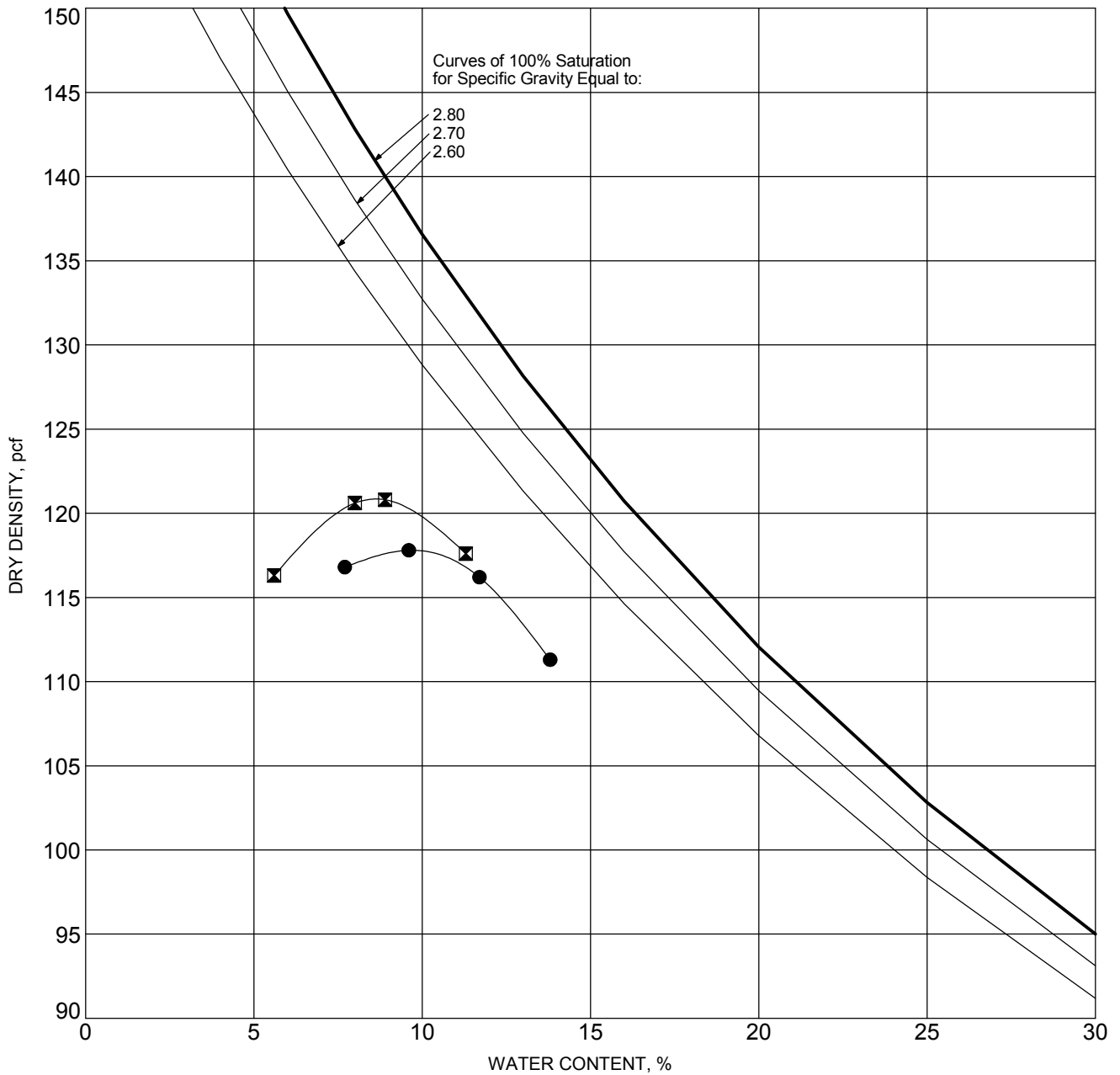


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Project No.  
 22-81-206-01

Drawing No.  
 B-1



SYMBOL	BORING NO.	DEPTH (ft)	DESCRIPTION	ASTM TEST METHOD	OPTIMUM WATER, %	MAXIMUM DRY DENSITY, pcf
●	BH-03	0.0-3.0	SILTY SAND (SM), BROWN	D1557 Method D	10.5	118.2
☒	BH-07	0.0-2.0	SILTY SAND (SM), DARK BROWN	D1557 Method D	8.3	121

## MOISTURE-DENSITY RELATIONSHIP RESULTS

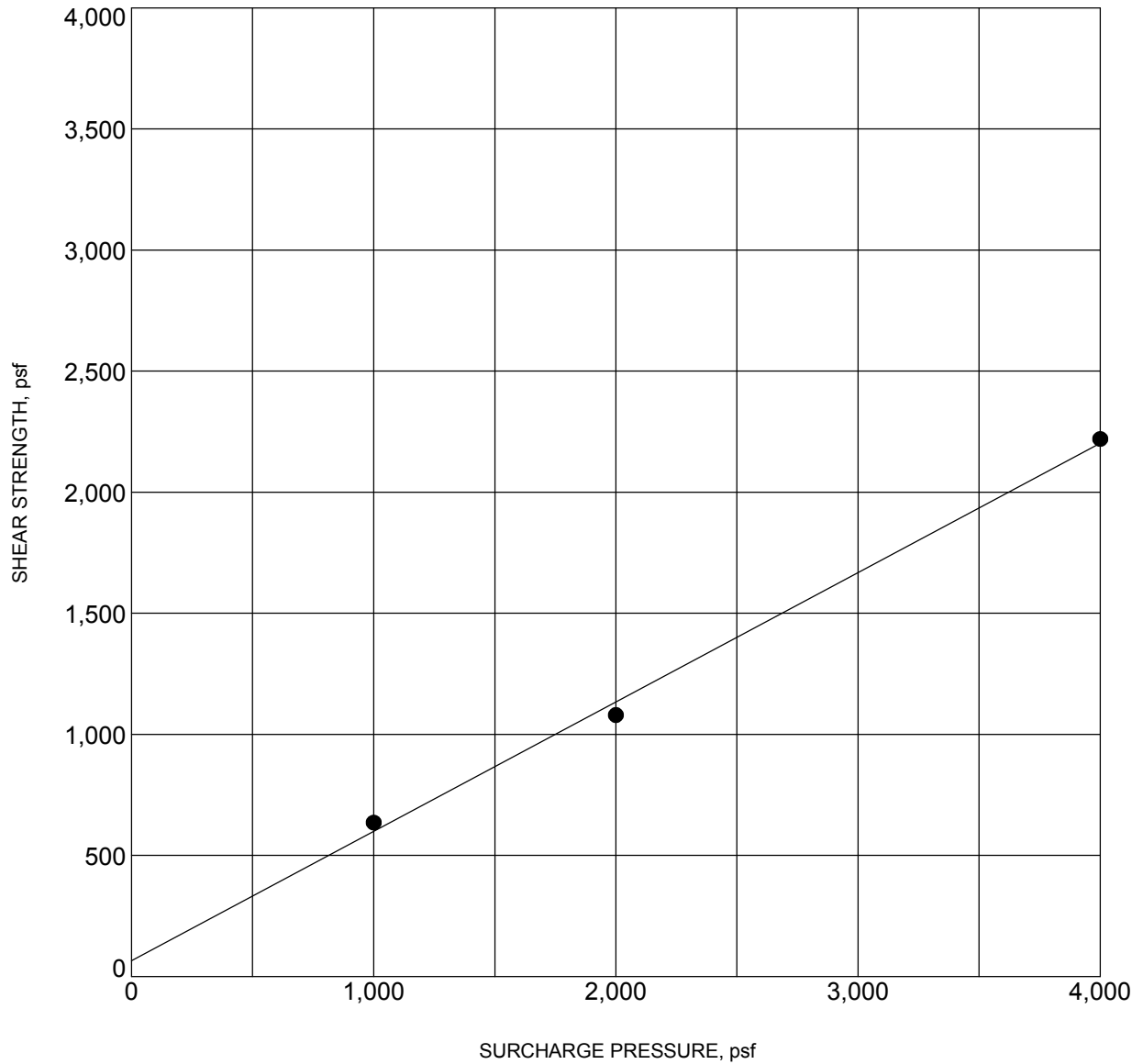


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Project No.  
**22-81-206-01**

Drawing No.  
**B-2**



BORING NO. :	<b>BH-05</b>	DEPTH (ft) :	<b>8.0-9.5</b>
DESCRIPTION :	<b>SILTY SAND (SM)</b>		
COHESION (psf) :	<b>70</b>	FRICTION ANGLE (degrees):	<b>28</b>
MOISTURE CONTENT (%) :	<b>5.0</b>	DRY DENSITY (pcf) :	<b>103.0</b>

NOTE: Ultimate Strength.

## DIRECT SHEAR TEST RESULTS

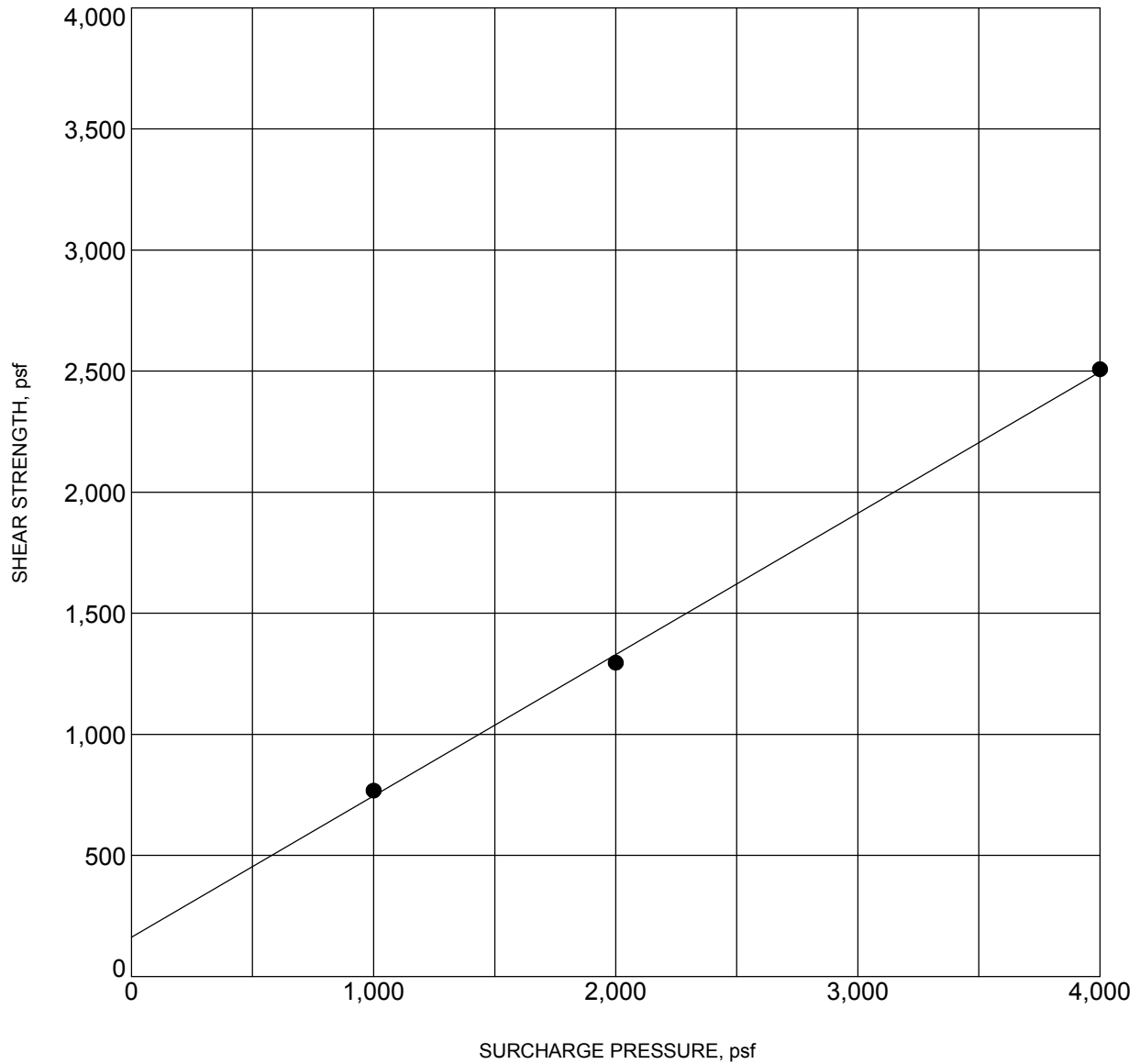


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Project No.  
**22-81-206-01**

Drawing No.  
**B-3**



BORING NO. :	<b>BH-08</b>	DEPTH (ft) :	<b>3.0-4.5</b>
DESCRIPTION :	<b>SILTY SAND (SM)*</b>		
COHESION (psf) :	<b>160</b>	FRICTION ANGLE (degrees):	<b>30</b>
MOISTURE CONTENT (%) :	<b>4.0</b>	DRY DENSITY (pcf) :	<b>117.0</b>

\*Remolded to 90% of laboratory maximum dry density.

NOTE: Ultimate Strength.

## DIRECT SHEAR TEST RESULTS



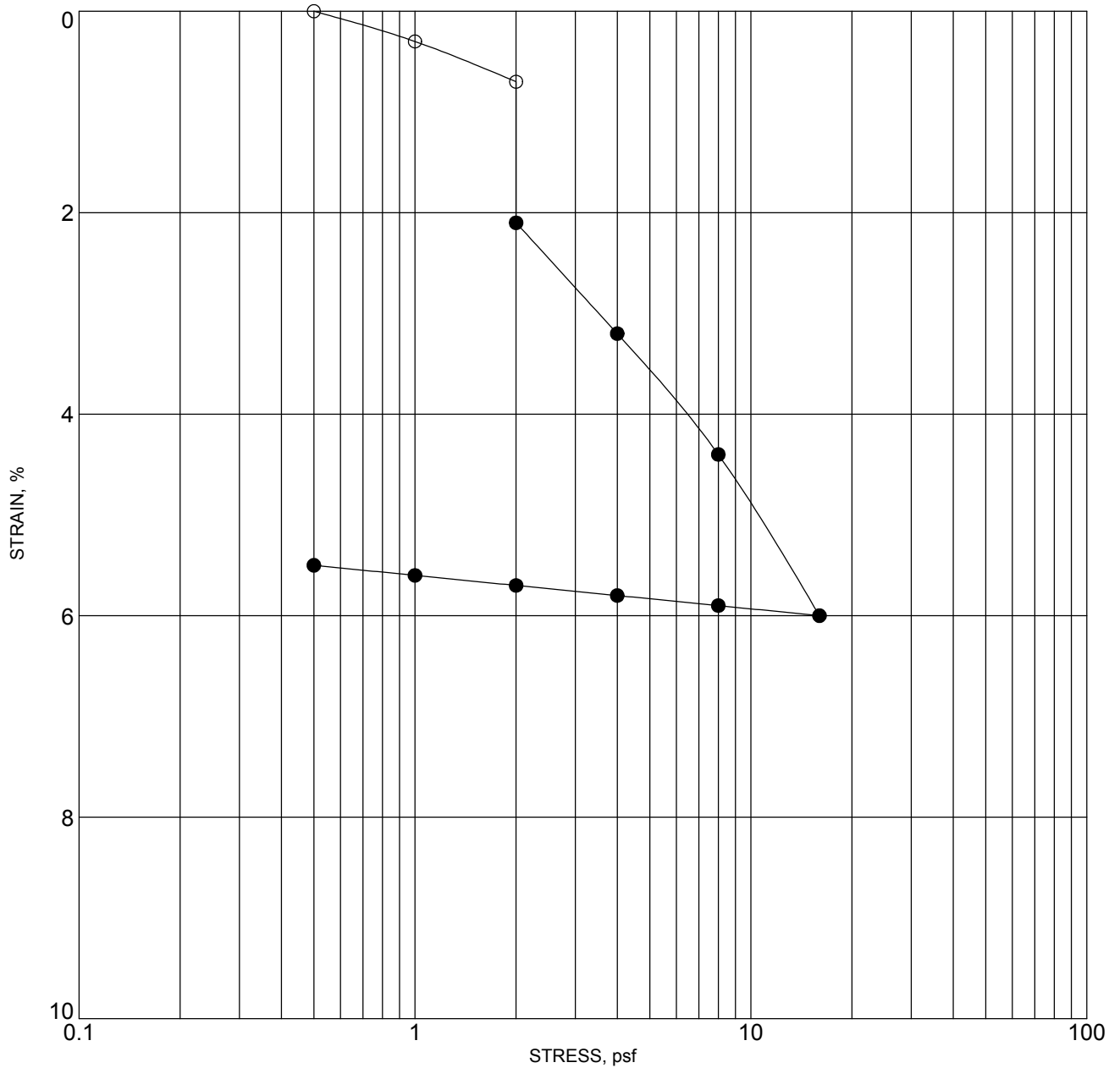
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Project No.  
**22-81-206-01**

Drawing No.  
**B-4**





BORING NO. :		<b>BH-01</b>		DEPTH (ft) :		<b>3.5-5.0</b>	
DESCRIPTION :				<b>SILTY SAND (SM)</b>			
MOISTURE CONTENT (%)		DRY DENSITY (pcf)		PERCENT SATURATION		VOID RATIO	
INITIAL	<b>4</b>	<b>95.1</b>		<b>14</b>		<b>0.734</b>	
FINAL							

NOTE: SOLID CIRCLES INDICATE READINGS AFTER ADDITION OF WATER

## CONSOLIDATION TEST RESULTS

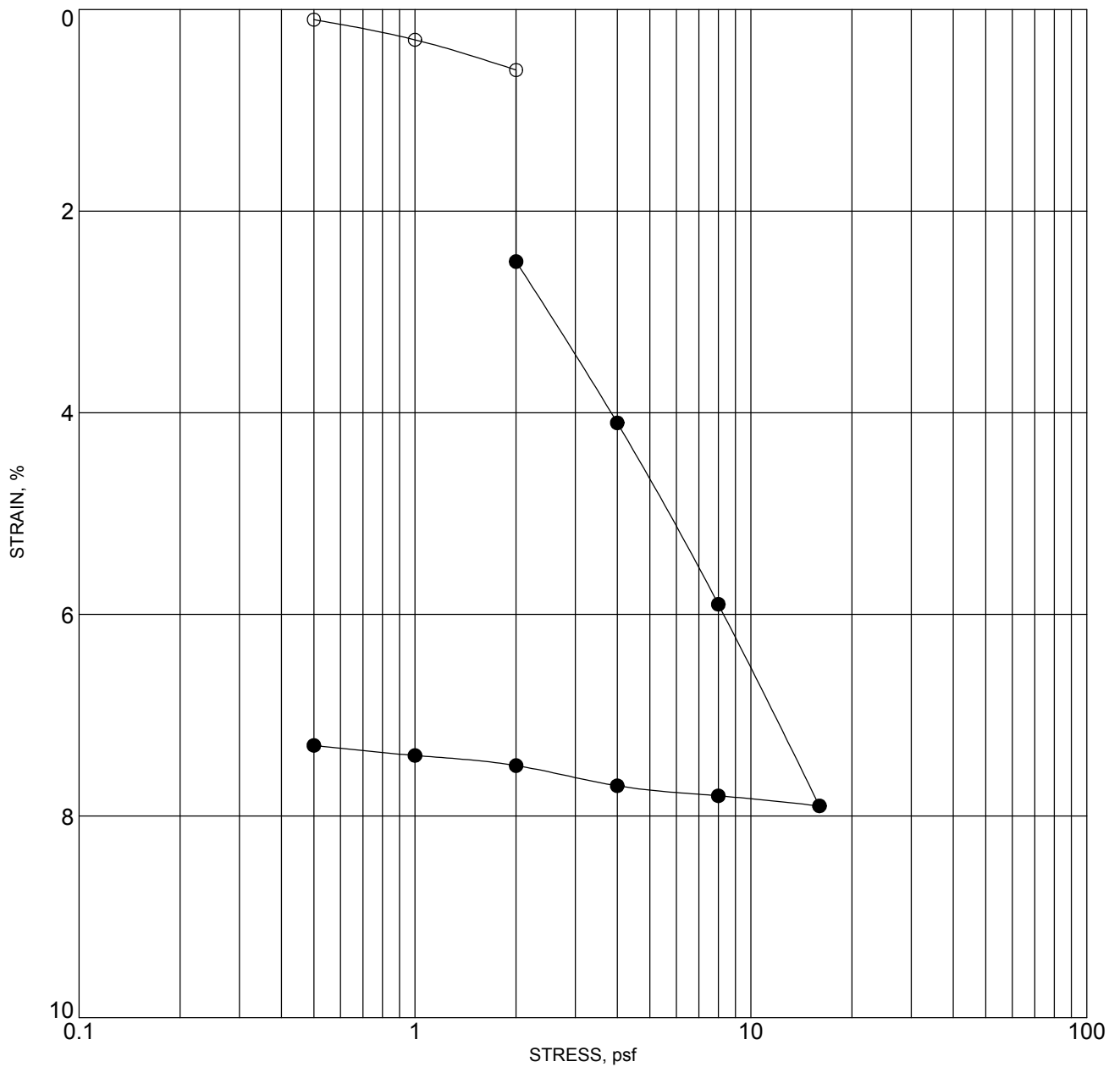


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Project No.  
**22-81-206-01**

Drawing No.  
**B-5**



BORING NO. :		<b>BH-04</b>		DEPTH (ft) :		<b>4.0-5.5</b>	
DESCRIPTION :				<b>SILTY SAND (SM)</b>			
MOISTURE CONTENT (%)		DRY DENSITY (pcf)		PERCENT SATURATION		VOID RATIO	
INITIAL	<b>4</b>	<b>94.0</b>		<b>7</b>		<b>1.459</b>	
FINAL							

NOTE: SOLID CIRCLES INDICATE READINGS AFTER ADDITION OF WATER

## CONSOLIDATION TEST RESULTS



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Project No.  
**22-81-206-01**

Drawing No.  
**B-6**

# Appendix C

## Liquefaction and Settlement Analysis



## APPENDIX C

### LIQUEFACTION AND SETTLEMENT ANALYSIS

The subsurface data obtained from the boring BH-03 was used to evaluate the liquefaction potential and associated dry seismic settlement when subjected to ground shaking during earthquakes.

A simplified liquefaction hazard analysis was performed using the program SPTLIQ (InfraGEO Software, 2021) using the liquefaction triggering analysis method by Boulanger and Idriss (2014). A modal earthquake magnitude of M 8.1 was selected for the site based on the results of seismic disaggregation analysis using the USGS interactive online tool (<https://earthquake.usgs.gov/hazards/interactive/>).

A peak ground acceleration ( $PGA_M$ ) of 0.727g for the MCE design event, where g is the acceleration due to gravity, was selected for this analysis. The PGA was based on the 2022 CBC seismic design parameters presented in Section 7.2, *CBC Seismic Design Parameters*.

The results of our analyses are presented on Plates of Appendix C and summarized in the following table.

**Table No. C-1, Estimated Dynamic Settlements**

Location	Groundwater Current Depth (feet bgs)	Groundwater Historical Depth (feet bgs)	Dry Seismic Settlement (inches)	Liquefaction Induced Settlement (inches)
BH-03	> 50.0	>50.0	1.44	Negligible

Based on our analysis, we anticipate the site has the potential for up to 1.44 inches of dry seismic settlement. The differential settlement resulting from dynamic loads is anticipated to be 0.72 inches over a horizontal distance of 40 feet. The structural engineer should consider this in the design.



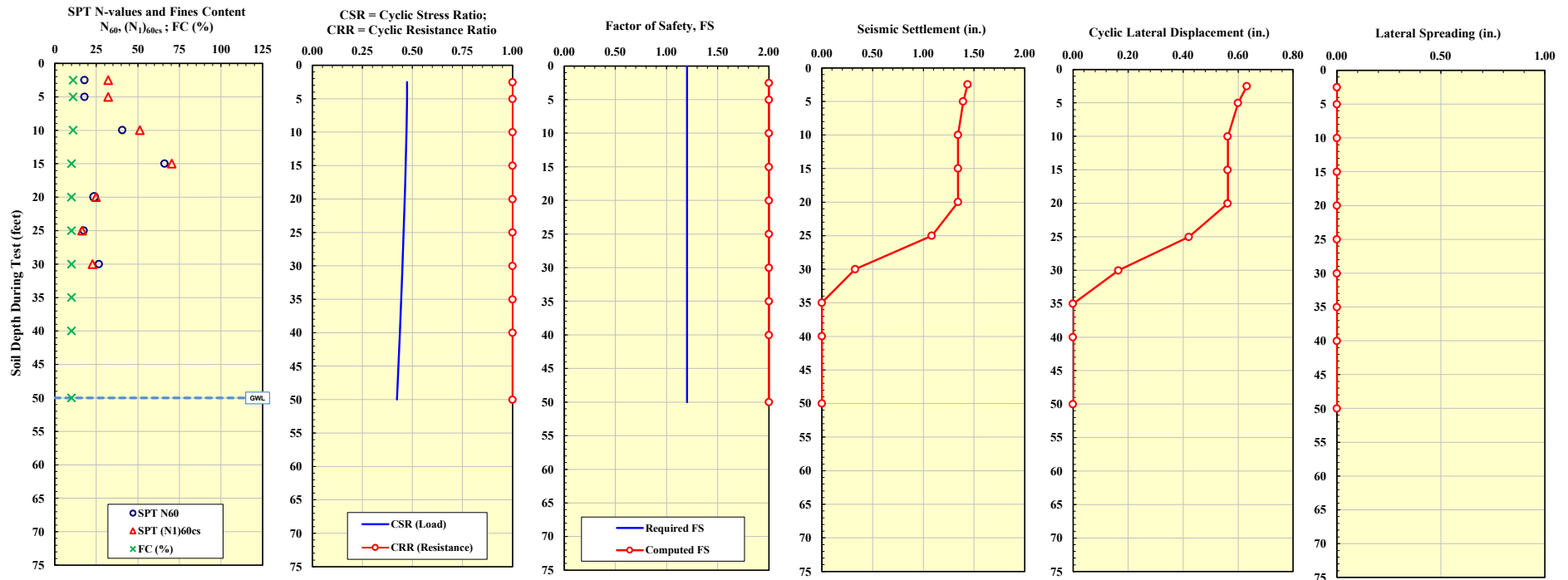




# SIMPLIFIED LIQUEFACTION HAZARDS ASSESSMENT USING STANDARD PENETRATION TEST (SPT) DATA

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PROJECT INFORMATION	
Project Name	Bloomington Animal Shelter
Project No.	22-81-206-01
Project Location	18313 Valley Boulevard, Bloomington Area of San Bernardino County, California
Analyzed By	Sk Syfur Rahman
Reviewed By	Hashmi S. Quazi



<b>Analysis Methods Used ==&gt;&gt;&gt;</b>	<b>Liquefaction Triggering:</b> Boulanger-Idriss (2014)	<b>Seismic Settlements:</b> Above GWL: Pradel (1998) Below GWL: Ishihara and Yoshimine (1992)	<b>Cyclic Lateral Displacements:</b> Pradel (1998) Tokimatsu and Asaka (1998)	<b>Lateral Spreading:</b> Zhang et al. (2004)
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# Appendix D

## Percolation Testing





## APPENDIX D

### PERCOLATION TESTING

Percolation testing was performed at two locations (PT-01 and PT-02) on December 9, 2022, in general accordance with the San Bernardino County Technical Guidance Document for the Preparation of Conceptual/Preliminary and/or Project Water Quality Management Plans, Appendix VII, Infiltration Rate Evaluation Protocol and Factor of Safety Recommendations (San Bernardino County, 2013) for using a percolation testing method to estimate infiltration rates.

Upon completion of drilling the test holes, approximately 2-inch-thick gravel layer was placed at the bottom of each hole and a 3.0-inch diameter perforated pipe was installed above the gravel to the ground surface. The boring annulus around the pipe was filled with gravel. The purpose of the pipe and gravel was to reduce the potential for erosion and caving due to the addition of water to the hole.

Each test hole was presoaked by filling with water to at least 5 times the radius of the test hole. Percolation testing was conducted the day following presoaking. More than 6 inches of water seeped away from the test holes in less than 25 minutes for 2 consecutive measurements, meeting the criteria for testing as “sandy soil”. During testing, the water level and total depth of the test hole were measured from the top of the pipe every 10 minutes for one hour. Following the completion of percolation testing, the pipe was removed from each test hole and the percolation test hole was backfilled with cutting soils and compacted.

Percolation rates describe the movement of water horizontally and downward into the soil from a boring. Infiltration rates describe the downward movement of water through a horizontal surface, such as the floor of a retention basin. Percolation rates are related to infiltration rates but are generally higher and require conversion before use in design. The percolation test data was used to estimate infiltration rates using the Porchet Inverse Borehole Method, in accordance with the San Bernardino County guidelines. A factor of safety of 2 was applied to the measured infiltration rates to account for subsurface variations, uncertainty in the test method, and future siltation. The infiltration structure designer should determine whether additional design-related safety factors are appropriate.

The measured percolation test data, calculations and estimated infiltration rates are shown on Plates No. 1 and 4. The estimated and design infiltration rates at the test holes are presented in the following table.



**Table D-1, Estimated Infiltration Rates**

Percolation Test	Approx. Depth of Boring* (feet)	Predominant Soil Types (USCS)	Average Infiltration Rate (inches/hour) (FOS 2)
PT-01	5.3	Silty Sand (SM)	1.82
PT-02	10.2	Silty Sand (SM)	6.30

Based on the calculated infiltration rate during the final respective intervals in each test, a design infiltration rate of 1.82 and 6.30 (inches/hour) can be used for depth of 5 feet and 10 feet respectfully for selected percolation testing locations. Please note that infiltration rates may change if the soil type and location of the proposed system changes. If that is the case, then additional percolation testing should be performed in the required location.



**Estimated Infiltration Rate from Percolation Test Data, PT-01**

Project Name	Bloomington Animal Shelter
Project Number	22-81-206-01
Test Number	PT-01
Test Location	Southeast of site
Personnel	Stephen McPherson
Presoak Date	12/8/2022
Test Date	12/9/2022

Shaded cells contain calculated values.

Test Hole Radius, r (inches)	4
Total Depth of Test hole, D <sub>T</sub> (inches)	62.5
Inside Diameter of Pipe, I (inches)	2.88
Outside Diameter of Pipe, O (inches)	3.13
Factor of Safety (FOS), F	2

Interval No.	Time Interval, Δt (min)	Initial Depth to Water, D <sub>0</sub> (inches)	Final Depth to Water, D <sub>f</sub> (inches)	Elapsed Time (min)	Initial Height of Water, H <sub>0</sub> (inches)	Final Height of Water, H <sub>f</sub> (inches)	Change in Height of Water, ΔH (inches)	Average Head Height, H <sub>avg</sub> (inches)	Infiltration Rate, I <sub>t</sub> (inches/hr)	Infiltration Rate with FOS, I <sub>f</sub> (inches/hr)
				0						0
1	25.00	11.40	40.80	25.00	51.10	21.70	29.40	36.40	3.68	1.84
2	25.00	5.88	37.44	50.00	56.62	25.06	31.56	40.84	3.54	1.77
3	10.00	8.40	24.72	60.00	54.10	37.78	16.32	45.94	4.09	2.04
4	10.00	8.40	24.00	70.00	54.10	38.50	15.60	46.30	3.88	1.94
5	10.00	8.40	23.64	80.00	54.10	38.86	15.24	46.48	3.77	1.89
6	10.00	8.40	23.40	90.00	54.10	39.10	15.00	46.60	3.70	1.85
7	10.00	8.40	23.16	100.00	54.10	39.34	14.76	46.72	3.64	1.82
8	10.00	8.40	23.16	110.00	54.10	39.34	14.76	46.72	3.64	1.82

**Recommended Design Infiltration Rate (inches/hr) 1.82**

Infiltration calculations are based on the Porchet Inverse Borehole Method presented in Riverside County BMP Design Handbook, Appendix A, Infiltration Testing (Riverside County, 2011)

$$H_0 = D_T - D_0$$

$$H_f = D_T - D_f$$

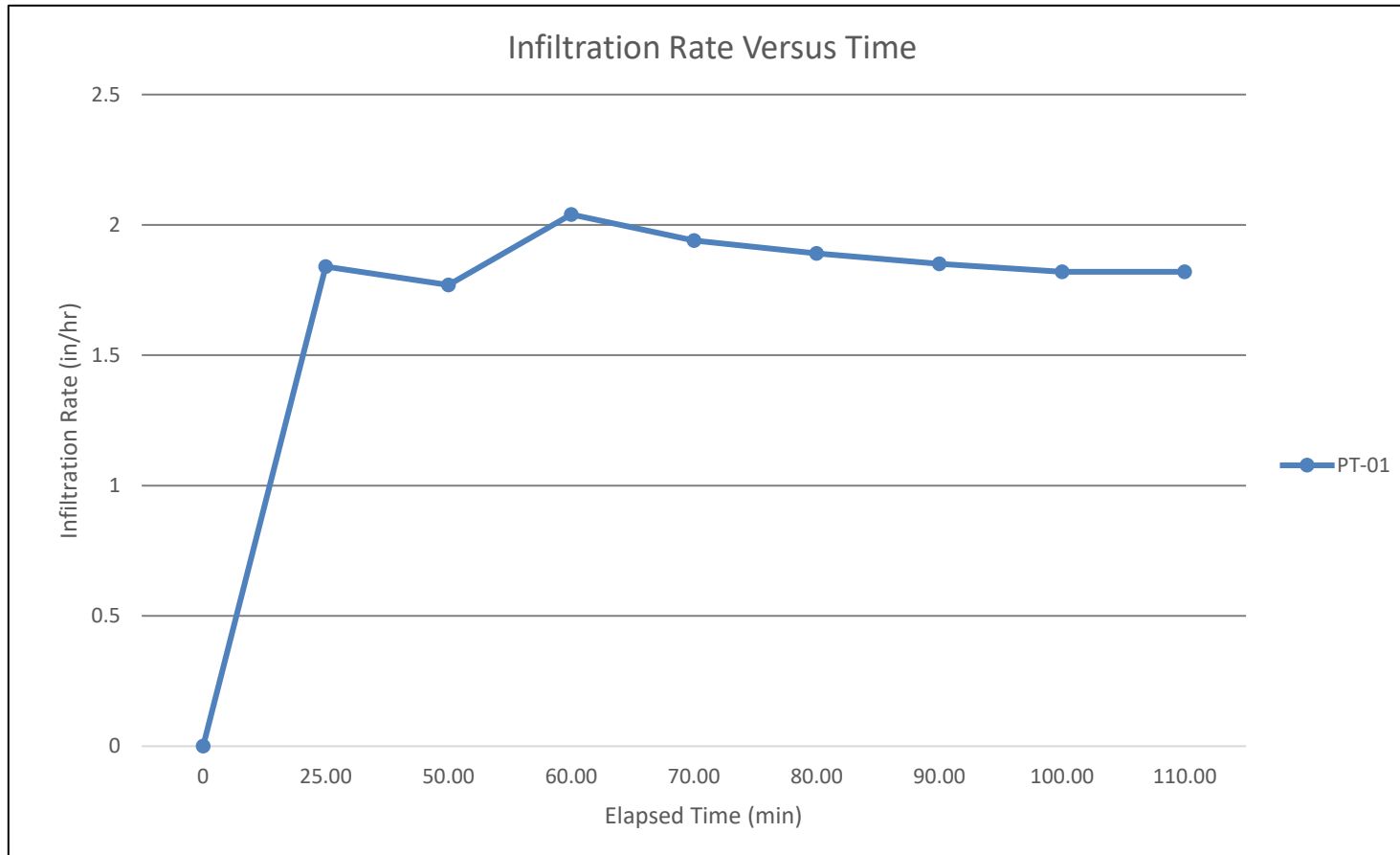
$$\Delta H = H_0 - H_f$$

$$H_{avg} = (H_0 + H_f) / 2$$

$$I_t = (\Delta H * (60 * r)) / (\Delta t * (r + (2 * H_{avg})))$$

### Infiltration Rate versus Time, PT-01

Project Name	Bloomington Animal Shelter
Project Number	22-81-206-01
Test Number	PT-01
Test Location	Southeast of site
Personnel	Stephen McPherson
Presoak Date	12/8/2022
Test Date	12/9/2022



**Estimated Infiltration Rate from Percolation Test Data, PT-01**

Project Name	Bloomington Animal Shelter
Project Number	22-81-206-01
Test Number	PT-02
Test Location	Southwest of site
Personnel	Stephen McPherson
Presoak Date	12/8/2022
Test Date	12/9/2022

Shaded cells contain calculated values.

Test Hole Radius, r (inches)	4
Total Depth of Test hole, D <sub>T</sub> (inches)	122.75
Inside Diameter of Pipe, I (inches)	2.88
Outside Diameter of Pipe, O (inches)	3.13
Factor of Safety (FOS), F	2

Interval No.	Time Interval, Δt (min)	Initial Depth to Water, D <sub>0</sub> (inches)	Final Depth to Water, D <sub>f</sub> (inches)	Elapsed Time (min)	Initial Height of Water, H <sub>0</sub> (inches)	Final Height of Water, H <sub>f</sub> (inches)	Change in Height of Water, ΔH (inches)	Average Head Height, H <sub>avg</sub> (inches)	Infiltration Rate, I <sub>t</sub> (inches/hr)	Infiltration Rate with FOS, I <sub>f</sub> (inches/hr)
				0						0
1	25.00	12.00	120.60	25.00	110.75	2.15	108.60	56.45	8.92	4.46
2	25.00	14.76	118.44	50.00	107.99	4.31	103.68	56.15	8.56	4.28
3	10.00	15.60	97.80	60.00	107.15	24.95	82.20	66.05	14.50	7.25
4	10.00	13.92	94.92	70.00	108.83	27.83	81.00	68.33	13.82	6.91
5	10.00	18.00	94.20	80.00	104.75	28.55	76.20	66.65	13.32	6.66
6	10.00	12.60	91.68	90.00	110.15	31.07	79.08	70.61	13.07	6.53
7	10.00	16.80	91.68	100.00	105.95	31.07	74.88	68.51	12.74	6.37
8	10.00	14.40	90.36	110.00	108.35	32.39	75.96	70.37	12.60	6.30

**Recommended Design Infiltration Rate (inches/hr) 6.30**

Infiltration calculations are based on the Porchet Inverse Borehole Method presented in Riverside County BMP Design Handbook, Appendix A, Infiltration Testing (Riverside County, 2011)

$$H_0 = D_T - D_0$$

$$H_f = D_T - D_f$$

$$\Delta H = H_0 - H_f$$

$$H_{avg} = (H_0 + H_f) / 2$$

$$I_t = (\Delta H * (60 * r)) / (\Delta t * (r + (2 * H_{avg})))$$

### Infiltration Rate versus Time, PT-01

Project Name	Bloomington Animal Shelter
Project Number	22-81-206-01
Test Number	PT-02
Test Location	Southwest of site
Personnel	Stephen McPherson
Presoak Date	12/8/2022
Test Date	12/9/2022

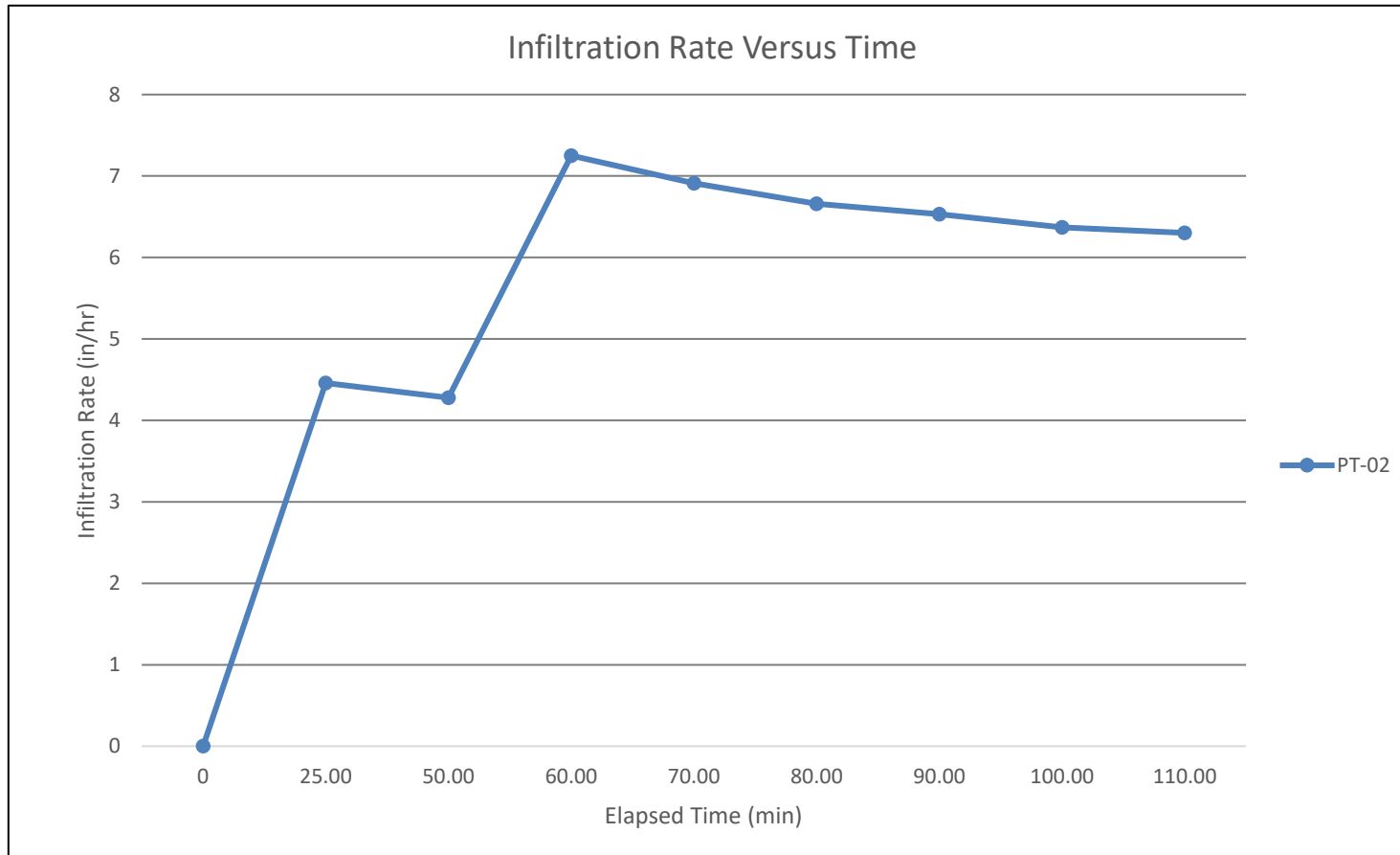


Plate No.

4

**Worksheet H: Factor of Safety and Design Infiltration Rate and Worksheet**

Factor Category		Factor Description	Assigned Weight (w)	Factor Value (v)	Product (p) $p = w \times v$
A	Suitability Assessment	Soil assessment methods	0.25	2	0.50
		Predominant soil texture	0.25	1	0.25
		Site soil variability	0.25	1	0.25
		Depth to groundwater / impervious layer	0.25	1	0.25
		Suitability Assessment Safety Factor, $S_A = \sum p$			
B	Design	Tributary area size	0.25	1	0.25
		Level of pretreatment/ expected sediment loads	0.25	3	0.75
		Redundancy	0.25	3	0.75
		Compaction during construction	0.25	1	0.25
		Design Safety Factor, $S_B = \sum p$			
Combined Safety Factor, $S_{Total} = S_A \times S_B$				2.50	
Observed Infiltration Rate, inch/hr, $K_{observed}$ (corrected for test-specific bias)				1.82	
Design Infiltration Rate, in/hr, $K_{DESIGN} = K_{Observed} / S_{Total}$				0.73	
<b>Supporting Data</b>					
Briefly describe infiltration test and provide reference to test forms: See soils report in Appendix 6.4					

**Note:** The minimum combined adjustment factor shall not be less than 2.0 and the maximum combined adjustment factor shall not exceed 9.0.

Calculator for Corrugated Steel Pipe Storage System

	Quantity	Units	
Pipe Diameter:	6	ft	
Total Linear Feet:	966	ft	966
Gravel Depth on Bottom:	0.5	ft	
Gravel Depth on Top:	1	ft	
Porosity of Gravel:	0.40	Decimal %	
Depth vs Volume Increment	0.5	ft	
Bed Length	325	ft	
Bed width	30	ft	
Bottom Elevation	1097.67		

	Elevation	System Depth (ft)	Water Depth in Pipe (ft)	θ	X-Sect Area of Water in Pipe (ft <sup>2</sup> )	Volume of Water in Pipe (ft <sup>3</sup> )	Volume of Water in Gravel (ft <sup>3</sup> )	Total Volume (ft <sup>3</sup> )	Total Volume (ac-ft)
Depth 0	1097.67	0	0	0.00	0.00	0.00	0.00	0.00	0.00
Depth 1	1098.17	0.5	0	0.00	0.00	0.00	1,950.00	1950	0.0448
Depth 2	1098.67	1	0.5	67.11	1.13	1,087.13	3,465.15	4552	0.1045
Depth 3	1099.17	1.5	1	96.38	3.10	2,992.17	4,653.13	7645	0.1755
Depth 4	1099.67	2	1.5	120.00	5.53	5,339.72	5,664.11	11004	0.2526
Depth 5	1100.17	2.5	2	141.06	8.25	7,969.70	6,562.12	14532	0.3336
Depth 6	1100.67	3	2.5	160.81	11.15	10,771.98	7,391.21	18163	0.4170
	1100.77	3.1	2.6	164.68	11.74	11,344.99	7,552.00	18897	0.4338
Depth 7	1101.17	3.5	3	180.00	14.14	13,656.50	8,187.40	21844	0.5015
Depth 8	1101.67	4	3.5	160.81	17.12	16,541.03	8,983.59	25525	0.5860
Depth 9	1102.17	4.5	4	141.06	20.02	19,343.31	9,812.68	29156	0.6693
Depth 10	1102.67	5	4.5	120.00	22.75	21,973.28	10,710.69	32684	0.7503
Depth 11	1103.17	5.5	5	96.38	25.18	24,320.84	11,721.66	36043	0.8274
Depth 12	1103.67	6	5.5	67.11	27.15	26,225.88	12,909.65	39136	0.8984
Depth 13	1104.17	6.5	6	0.00	28.27	27,313.01	14,424.80	41738	0.9582
Depth 14	1104.67	7	6	0.00	28.27	27,313.01	16,374.80	43688	1.0029
Depth 15	1105.17	7.5	6	0.00	28.27	27,313.01	18,324.80	45638	1.0477