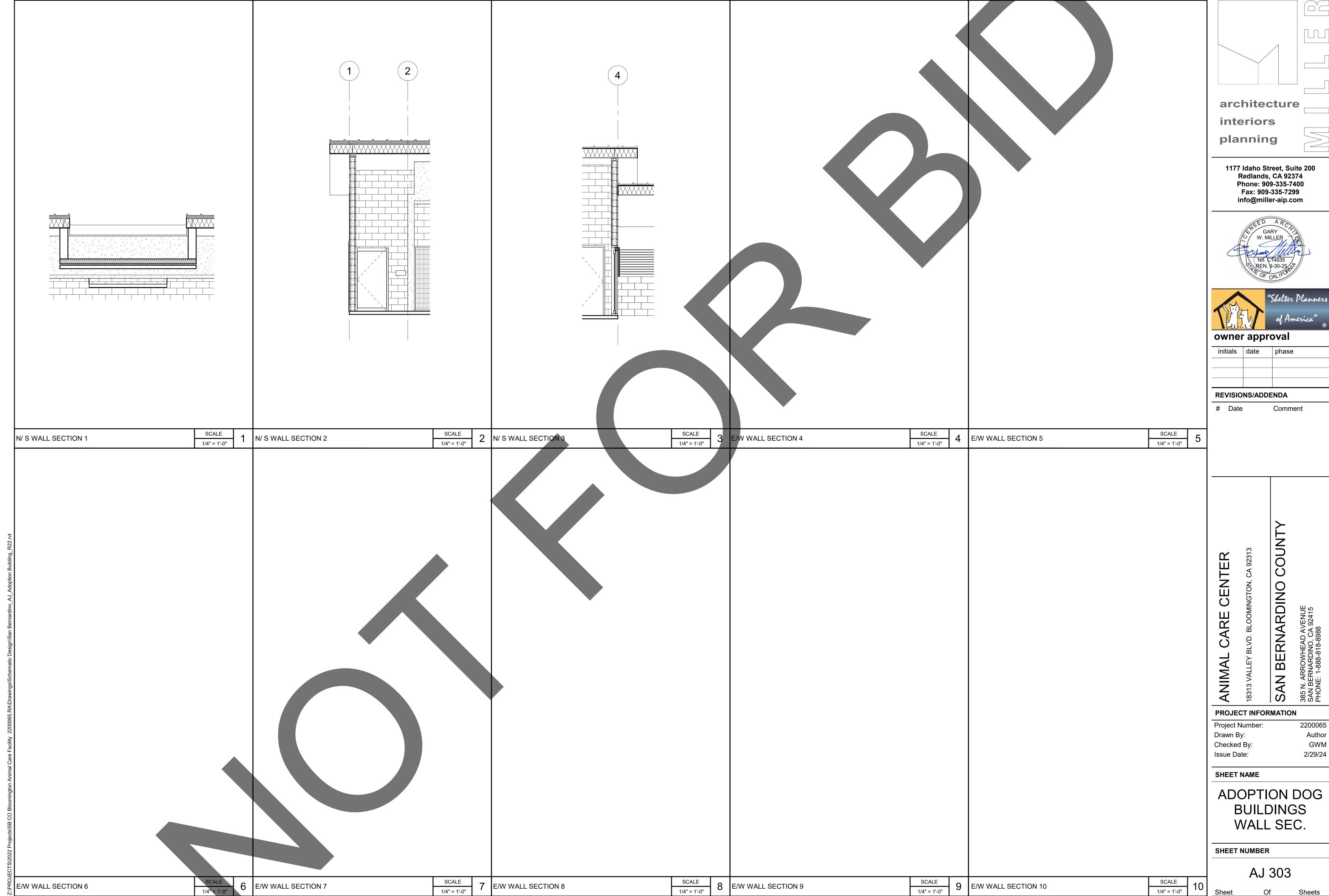
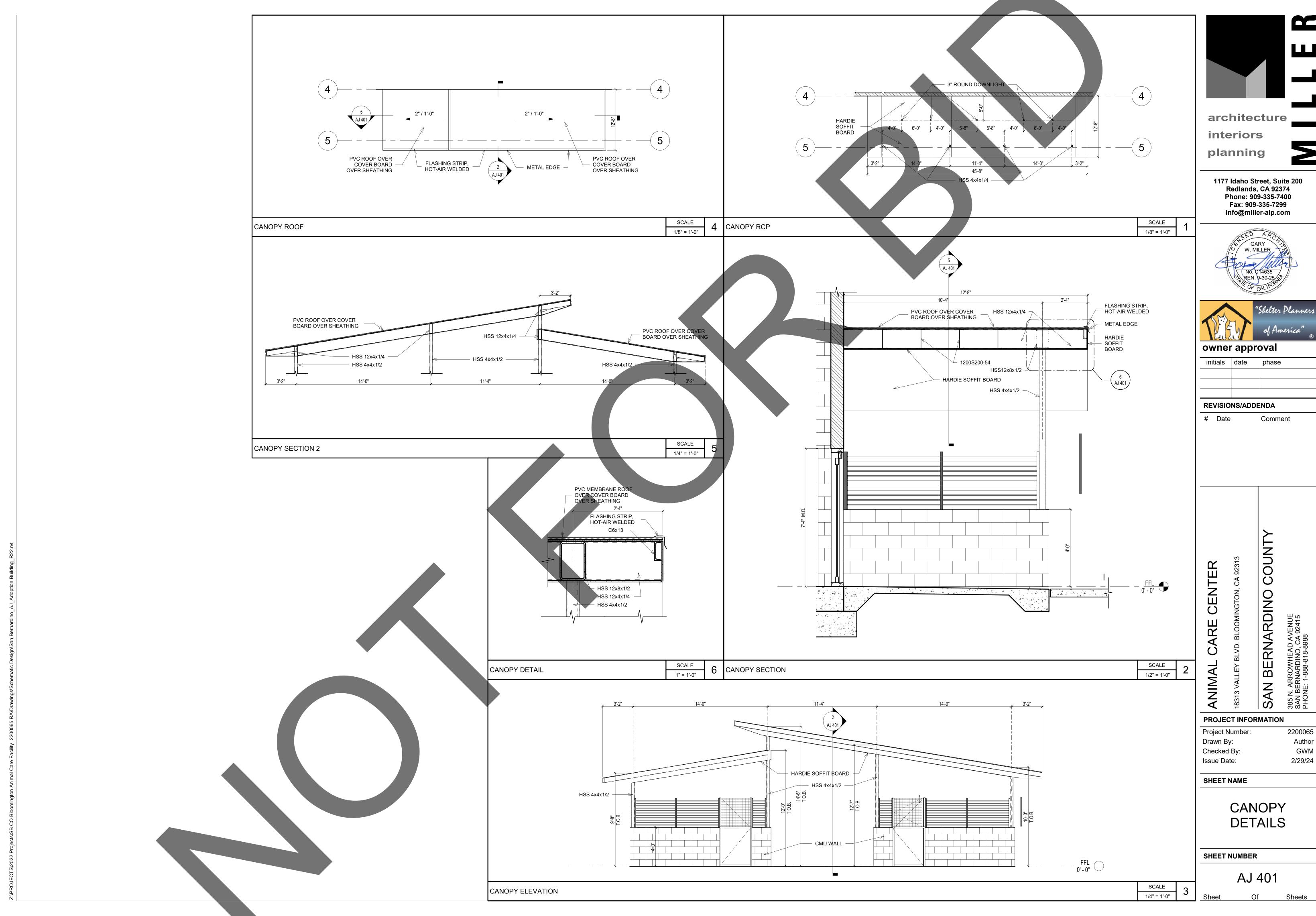


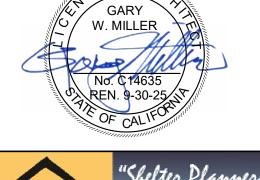
initials	date	phase





architecture

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initials	date	phase

SAN

2200065

Author

GWM

2/29/24

## PROJECT INFORMATION

Project Number: Drawn By: Checked By: Issue Date:

SHEET NAME

SCHEDULES

SHEET NUMBER

AJ 601

3

 $\mathsf{AL}$ 

**GLAZING NOTES** 

A. U-FACTOR: 0.00

B. SHGC: 0.00 C. VT: 0.00

16' - 0" | 4' - 0"

**WINDOW** 

1. TEMPERED GLASS

2. WIRE GLASS3. DUAL GLAZING

4. SINGLE GLAZING

ALUMINUM **HOLLOW METAL** 

**EXISTING** 

PAINTED

5. FIRE PROTECTION RATED GLAZING

VINYL FRAME MATERIAL

FACTORY APPLIED

**EXISTING FINISH** 

**GLAZING:** 

**FRAME** 

MATERIAL:

DENOTES TEMPERED GLASS

**STOREFRONT NOTES:** 

CUSTOM WINDOW SEE WINDOW SCHEDULE

PRE-MANUFACTURED WINDOWS SEE WINDOW SCHEDULE

9/A-502 | 10/A-50 |

1. ALL EXTERIOR GLAZING TO COMPLY WITH THE FOLLOWING TITLE 24 REQUIREMENTS:

2. ALL EXTERIOSOLARBAN 70XL SOLEXIA 1-INCH INSULATED LOW-E VITRO OR EQUAL

WINDOW TYPES

(T) TEMPERED GLAZING

1/4" = 1'-0"

FLOOR LINE

3" = 1'-0"

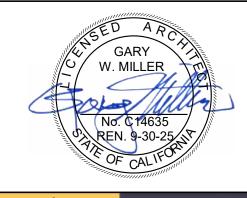
6" = 1'-0"

6" = 1'-0"

Of

Sheet

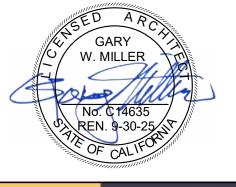
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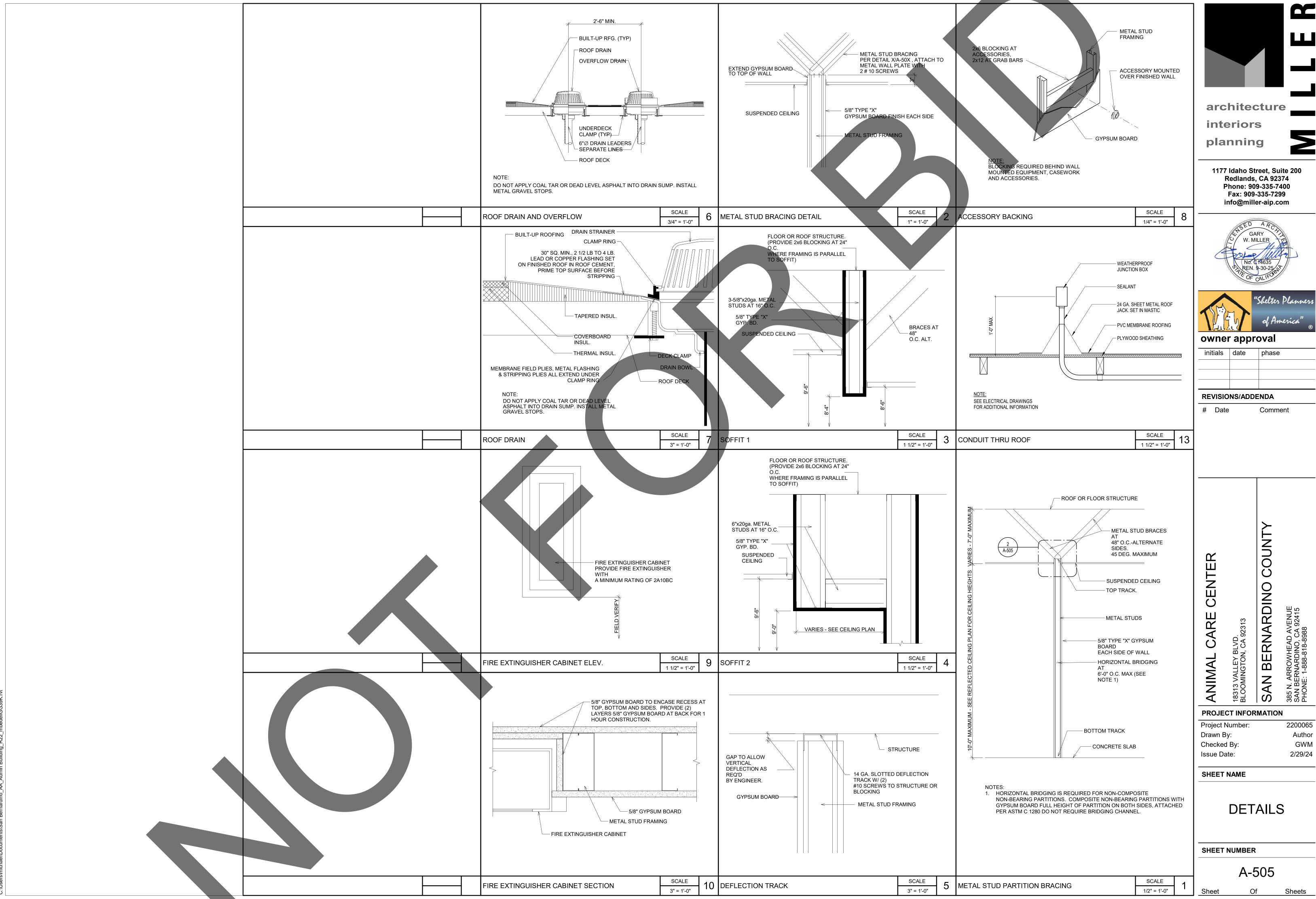
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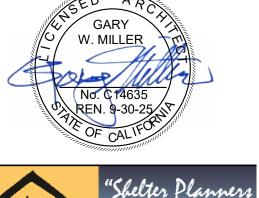
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initials	date	phase			
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#### **DESIGN CRITERIA AND LOADS**

1. STRUCTURE HAS BEEN DESIGNED TO COMPLY WITH: CALIFORNIA BUILDING CODE 2022 AND SUPPLEMENTS

( AÇI 318-19 ) ACI 530-13 AISC 360-16

AISC 341-16, INCLUDING SUPPLEMENTS AWS D1.1 AISI S100, S240

NDS-18 AND SDPWS-15 TMS 402/602-16 OCCUPANCY RISK CATEGORY SEISMIC: SEISMIC DESIGN CATEGORY

IMPORTANCE FACTOR 1.0 SOIL CLASSIFICATION PER SITE CLASS 1.560 a 0.604 g1.040 q 0.685 g

BUILDING	SEISMIC FORCE RESISTING SYSTEM	R	Cd	$\Omega_{O}$	ρ
SA	LIGHT-FRAME C.F.S. SHEAR WALLS WITH W.S.P./ STEEL SPECIAL MOMENT FRAMES	6.5 / 8	4 / 5.5	3/3	1.3
SB	LIGHT-FRAME C.F.S. SHEAR WALLS WITH W.S.P.	6.5	4	3	1.3
SC	SPECIAL REINFORCED CMU SHEAR WALL	5.0	3.5	2.5	1.3
SD	LIGHT-FRAME C.F.S. SHEAR WALLS WITH W.S.P.	6.5	4	3	1.3
SE, SF, SG, SH	SPECIAL REINFORCED CMU SHEAR WALL	5.0	3.5	2.5	1.3
SI	LIGHT-FRAME C.F.S. SHEAR WALLS WITH W.S.P.	6.5	4	3	1.3
SJ	SPECIAL REINFORCED CMU SHEAR WALL	5.0	3.5	2.5	1.3

4. WIND: BASIC WIND SPEED IMPORTANCE FACTOR EXPOSURE CLASS LIVE LOADS: TYPICAL ROOF TYPICAL FLOOR **PARTITION LOAD** HANDRAILS

20 PSF (REDUCIBLE) 50 PSF (REDUCIBLE) 15 PSF (UNREDUCIBLE)

1 (120 MPH

MAX OF SIMULTANEOUS VERT AND HORIZ THRUST 50 PLF APPLIED AT THE TOP OF THE RAILING OR 200 LBS IN ANY DIRECTION SEE S-103 FOR LOADING PLANS

NOTE: LIVE LOADS SHALL BE POSTED AS REQUIRED PER SECTION 1603.3 OF

#### **GENERAL**

- ALL WORK SHALL COMPLY WITH TITLE 24 OF THE CALIFORNIA BUILDING CODE, LATEST EDITION, AND ALL OTHER LOCAL OR STATE AGENCIES HAVING JURISDICTION ON THIS PROJECT.
- NEITHER THE PROFESSIONAL ACTIVITIES OF THE ENGINEER. NOR THE PRESENCE OF THE ENGINEER OR THEIR EMPLOYEES AND SUBCONSULTANTS AT THE CONSTRUCTION SITE. SHALL RELIEVE THE CONTRACTOR AND ANY OTHER ENTITY OF THEIR OBLIGATIONS, DUTIES AND RESPONSIBILITIES INCLUDING BUT NOT LIMITED TO, CONSTRUCTION MEANS, METHODS, SEQUENCE, TECHNIQUES OR PROCEDURES NECESSARY FOR PERFORMING, SUPERINTENDING OR COORDINATING ALL PORTIONS OF THE CONSTRUCTION WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS AND ANY HEALTH OR SAFETY PRECAUTIONS REQUIRED BY ANY REGULATORY AGENCIES. THE ENGINEER AND THEIR PERSONNEL HAVE NO AUTHORITY TO EXERCISE ANY CONTROL OVER ANY CONSTRUCTION CONTRACTOR OR OTHER ENTITY OR THEIR EMPLOYEES IN CONNECTION WITH THEIR WORK OR ANY HEALTH OR SAFETY PRECAUTIONS. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE JOBSITE SAFETY. THE ENGINEER AND THE ENGINEER'S CONSULTANTS SHALL BE MADE ADDITIONAL INSUREDS UNDER THE CONTRACTOR'S GENERAL LIABILITY INSURANCE POLICY.
- ALL DRAWINGS ARE CONSIDERED TO BE A PART OF THE CONTRACT DOCUMENTS. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE REVIEW AND COORDINATION OF ALL DRAWINGS PRIOR TO THE START OF CONSTRUCTION. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT PRIOR TO THE START OF CONSTRUCTION SO A CLARIFICATION CAN BE ISSUED. ANY WORK PERFORMED IN CONFLICT WITH THE CONTRACT DOCUMENTS OR ANY CODE REQUIREMENTS SHALL BE CORRECTED BY THE CONTRACTOR AT THEIR OWN EXPENSE AND AT NO EXPENSE TO THE
- ALL DIMENSIONS AND SITE CONDITIONS SHALL BE VERIFIED BY THE CONTRACTOR AT THE JOBSITE PRIOR TO CONSTRUCTION, START OF SHOP DRAWINGS, START OF CONSTRUCTION, AND/OR FABRICATION OF MATERIALS, IF DISCREPANCIES ARE ENCOUNTERED, OR CONDITIONS DEVELOP THAT ARE NOT COVERED BY THE CONTRACT DOCUMENTS, THE ARCHITECT SHALL BE NOTIFIED FOR CLARIFICATION.
- CONTRACTOR SHALL PROVIDE AND BE RESPONSIBLE FOR THE PROTECTION AND REPAIR OF ADJACENT EXISTING SURFACES AND AREAS WHICH MAY BE DAMAGED AS A RESULT OF NEW WORK. STRUCTURAL DRAWINGS INCLUDE DESIGN REQUIREMENTS AND DIMENSIONS FOR
- STRUCTURAL INTEGRITY BUT DO NOT SHOW ALL DETAIL DIMENSIONS TO FIT INTRICATE ARCHITECTURAL AND MECHANICAL DETAILS. CONTRACTOR SHALL SO CONSTRUCT THE WORK SO IT WILL CONFORM TO THE CLEARANCES REQUIRED BY ARCHITECTURAL, MECHANICAL AND ELECTRICAL DESIGN. ALL SYMBOLS AND ABBREVIATIONS USED ON THE DRAWINGS ARE CONSIDERED TO BE
- CONSTRUCTION STANDARDS. IF CLARIFICATION IS REQUIRED, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT PRIOR TO PROCEEDING WITH THE WORK. . DO NOT SCALE DRAWINGS. PRINTED DIMENSIONS HAVE PRECEDENCE OVER SCALED
- DRAWINGS AND LARGE-SCALE OVER SMALL-SCALE DRAWINGS. CONTRACTOR TO DETERMINE FINAL DIMENSION WITH ARCHITECT. TYPICAL DETAILS SHALL APPLY TO SITUATIONS OCCURRING ON THE PROJECT THAT ARE
- THE SAME OR SIMILAR TO THOSE SPECIFICALLY REFERENCED. WHERE NO DETAILS ARE GIVEN, CONSTRUCTION SHALL BE AS SHOWN FOR SIMILAR WORK. 10. THE CONTRACT DOCUMENTS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT
- INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE AND SAFETY OF WORKMEN DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING AND SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT, ETC. OBSERVATION VISITS TO THE SITE BY THE ARCHITECT OR STRUCTURAL ENGINEER SHALL NOT INCLUDE INSPECTION OR APPROVAL OF THE ABOVE ITEMS AND DOES NOT IN ANY WAY RELIEVE THE CONTRACTOR OF THEIR RESPONSIBILITIES FOR THE ABOVE. SEE ARCHITECTURAL, ELECTRICAL AND MECHANICAL DRAWINGS FOR DETAILS,
- CONDITIONS, PITS, TRENCHES, PADS, DEPRESSIONS, ROOF/FLOOR OPENINGS, STAIRS, SLEEVES, ITEMS TO BE EMBEDDED OR ATTACHED TO STRUCTURAL ELEMENTS, ETC., NOT SHOWN ON THE STRUCTURAL DRAWINGS.
- 12. ESTABLISH AND VERIFY ALL OPENINGS AND INSERTS FOR MECHANICAL, ELECTRICAL AND PLUMBING WITH APPROPRIATE TRADE CONTRACTORS. OPENING SIZES AND LOCATIONS SHOWN FOR DUCTS, PIPE, INSERTS AND OTHER PENETRATIONS WHEN SHOWN ARE FOR GENERAL INFORMATION ONLY AND SHALL BE VERIFIED PRIOR TO FORMING.
- 13. NO HOLES, NOTCHES, BLOCKOUTS, ETC. ARE ALLOWED IN STRUCTURAL ÉLEMENT UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS OR APPROVED BY THE STRUCTURAL ENGINEER. 14. BEFORE SUBMITTING A PROPOSAL FOR THIS WORK, EACH BIDDER SHALL VISIT
- PREMISES AND BECOME FULLY ACQUAINTED WITH THE EXISTING CONDITIONS. TEMPORARY CONSTRUCTION REQUIRED, QUANTITIES AND TYPE OF EQUIPMENT, ETC. THE BID SHALL INCLUDE ALL SUMS REQUIRED TO DO THE WORK WITHIN THE EXISTING
- 15. SHOP DRAWINGS SHALL BE REVIEWED AND COORDINATED PRIOR TO SUBMITTING TO THE ARCHITECT. EACH SHOP DRAWING SUBMITTED SHALL BE STAMPED INDICATING REVIEW BY THE CONSTRUCTION MANAGER/GENERAL CONTRACTOR AND REVIEW BY THE ARCHITECT SHALL NOT BEGIN UNTIL THIS IS COMPLETE. WORK SHALL NOT BEGIN WITHOUT REVIEW BY THE ARCHITECT/STRUCTURAL ENGINEER.
- 16. SHOP DRAWINGS SHALL BE REVIEWED BY THE ARCHITECT/STRUCTURAL ENGINEER FOR GENERAL CONFORMANCE WITH DESIGN CONCEPT ONLY. NOTATIONS MADE BY THE ARCHITECT/STRUCTURAL ENGINEER ON THE SHOP DRAWINGS DO NOT RELIEVE THE CONTRACTOR FROM COMPLYING WITH THE REQUIREMENTS OF THE DRAWINGS.

#### CAST-IN-PLACE CONCRETE

1. ALL CONCRETE WORK SHALL CONFORM TO THE AMERICAN CONCRETE INSTITUTE PUBLICATIONS: ACI 117, ACI 301, ACI 305.1, ACI 306.1, ACI 308.1, ACI 318 AND SP-066,

2. CONCRETE MATERIALS SHALL CONFORM TO: ASTM C150, TYPE I OR II CEMENT ASTM C618, TYPE C OR F FLY ASH FINE AND COARSE AGGREGATE ASTM C33 LIGHTWEIGHT AGGREGATE ASTM C330 POTABLE WATER

AIR-ENTRAINING ADMIXTURE

**UNLESS NOTED OTHERWISE** 

WATER-REDUCING ADMIXTURE

3.	CONCRETE STRENGTHS SHALL CO	NFORM TO:			
	INTENDED USE	28-DAY STRENGTH (PSI)	MAX W/C RATIO	A/E	SLUI
	FOUNDATIONS	4000	0.45	N/A	1"-4
	SLAB-ON-GRADE	4000	0.5	N/A	4"-6

ASTM C260

ASTM C494

4000

0.45 N/A

- LIGHTWEIGHT CONCRETE SHALL HAVE A DRY DENSITY OF 107-116 PCF.
- DRYPACK SHALL BE 1:3-1/2 PORTLAND CEMENT TO SAND WITH A MINIMUM 28-DAY STRENGTH OF 7000 PSI. GROUT SHALL BE 1:3:2 PORTLAND CEMENT TO SAND TO PEA GRAVEL WITH A
- MINIMUM 28-DAY STRENGTH OF 7000 PSI. SLAB ON GRADE CONSTRUCTION:

LOCATE SAW CUT CONTROL JOINTS ALONG COLUMN LINES WITH INTERMEDIATE JOINTS SPACED PER THE TABLE BELOW, UNO. SLAB PANELS SHALL HAVE A MAXIMUM LENGTH TO WIDTH RATIO OF 1.5:1. PROVIDE ADDITIONAL CONTROL JOINTS AT ALL RE-ENTRANT CORNERS.

THICKNESS (IN)	MAX JOINT SPACING (FT)
4	12
5	13
6	15

- CROSS REFERENCE ARCHITECTURAL AND STRUCTURAL DRAWINGS TO ASSURE PROPER DIMENSIONS AND PLACEMENT OF ALL ANCHOR BOLTS, INSERTS. NOTCHES, EDGES OF WALLS/GRADE BEAMS AND PIERS.
- UNO, ALL FOOTINGS SHALL BE CENTERED UNDER WALLS, PIERS OR COLUMNS. SANDBLAST ALL EXISTING CONCRETE SURFACES OLDER THAN 28 DAYS AGAINST WHICH CONCRETE IS TO BE PLACED, UNLESS DIRECTED OTHERWISE IN WRITING BY THE STRUCTURAL ENGINEER.
- PROVIDE SLEEVES FOR PLUMBING AND ELECTRICAL PENETRATIONS THROUGH CONCRETE BEFORE PLACING. SECURE SUCH SLEEVES TO PREVENT MOVEMENT DURING PLACING OPERATIONS. REFER TO MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATIONS OF PENETRATIONS.
- 12. CORE DRILLING CONCRETE IS NOT PERMITTED UNLESS NOTED OTHERWISE OR APPROVED IN WRITING BY THE ARCHITECT. NOTIFY THE ARCHITECT IN ADVANCE
- OF CONDITIONS NOT SHOWN ON THE DRAWINGS 13. CONFIRM WITH ARCHITECT THAT MATERIALS TO BE EMBEDDED ARE SUITABLE
- FOR EMBEDMENT IN CONCRETE. 14. THE OUTSIDE DIAMETER OF EMBEDDED CONDUIT OR PIPE SHALL NOT EXCEED 1/3 OF THE STRUCTURAL SLAB THICKNESS, INCLUDING AT CROSS-OVERS, AND SHALL BE PLACED BETWEEN THE TOP AND BOTTOM REINFORCING WITH A MINIMUM 3" CLEAR COVER. CONDUIT OR PIPE RUNNING PARALLEL TO EACH OTHER SHALL BE SPACED AT LEAST 8" APART AND NO MORE THAN 2 RUNS STACKED VERTICALLY IN THE SLAB. CONDUIT OR PIPE SHALL NOT BE EMBEDDED IN SLAB THICKNESSES LESS THAN 6 INCHES.
- 15. DO NOT PLACE PIPES, DUCTS, REGLETS OR CHASES IN STRUCTURAL CONCRETE WITHOUT APPROVAL OF THE STRUCTURAL ENGINEER THROUGH THE ARCHITECT
- 16. NO ALUMINUM SHALL BE ALLOWED IN THE CONCRETE WORK UNLESS COATED TO PREVENT ALUMINUM-CONCRETE REACTION.
- 17. WATERSTOPS SHALL BE A FLEXIBLE BENTONITE PVC PRODUCT. ACCEPTABLE PRODUCTS INCLUDE: CETCO WATERSTOP-RX AND GREENSTREAK SWELLSTOP WESTEC BARRIER TECHNOLOGIES TPE-R WATERSTOP AND GREENSTREAK PVC
- 18. PROJECTING CORNERS OF BEAMS, WALLS, COLUMNS, ETC., SHALL BE FORMED WITH A 3/4 INCH CHAMFER, UNLESS NOTED OTHERWISE ON ARCHITECTURAL
- 19. SLOPE SLABS TO DRAINS OR FOR POSITIVE DRAINAGE IF NO DRAINS ARE PRESENT, AND PROVIDE DEPRESSIONS WHERE SHOWN ON THE STRUCTURAL AND/OR ARCHITECTURAL DRAWINGS, WITHOUT REDUCING THE THICKNESS O SLAB INDICATED. FOR SLAB-ON-GRADE DEPRESSIONS GREATER THAN 1 INC REFER TO DETAILS FOR ADDITIONAL REINFORCING.
- 20. INTERNALLY VIBRATE ALL CAST-IN-PLACE CONCRETE EXCEPT SLABS-ON WHICH NEED ONLY BE VIBRATED AROUND UNDER FLOOR DUCTS AND O EMBEDDED ITEMS. VIBRATE TOPS OF COLUMNS.
- 21. PROVIDE VERTICAL CONTROL JOINTS IN EXPOSED CONCRETE WALLS AT A MINIMUM UNIFORM SPACING NOT TO EXCEED 25 FEET PER ACI 224.3. COORDIN JOINT LOCATIONS WITH ARCHITECTURAL DRAWINGS.
- 22. CONCRETE SHALL NOT BE PERMITTED TO DROP MORE THAN 5 FEET. 23. CONCRETE SLABS SHALL BE CURED BY KEEPING CONTINUOUSLY WET FOR 7 DAYS. FORMS FOR CONCRETE WALLS SHALL BE LEFT IN PLACE FOR 7 DAYS OR MAY BE STRIPPED AFTER 3 DAYS AND COATED WITH AN APPROVED CURING
- 24. NOTIFY THE ARCHITECT/STRUCTURAL ENGINEER 48 HOURS MINIMUM PRIOR TO ALL POURS
- 25. THE DESIGN AND ENGINEERING OF FORMWORK, AS WELL AS ITS CONSTRUCTION, SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. FORMS SHALL BE DESIGNED TO HAVE SUFFICIENT STRENGTH TO SAFELY WITHSTAND THE LOADS RESULTING FROM PLACEMENT AND VIBRATION OF THE CONCRETE, AND SHALL ALSO BE DESIGNED FOR SUFFICIENT RIGIDITY TO MAINTAIN SPECIFIED TOLERANCES. CONTRACTOR SHALL SUBMIT DETAILED FORMWORK SHOP DRAWINGS TO THE ARCHITECT TO BE REVIEWED FOR GENERAL COMPLIANCE
- WITH THE DESIGN CONCEPT ONLY. 26. THE STRUCTURAL STEEL FRAME WILL DEFLECT WHILE CONCRETE IS BEING PLACED. THIS WILL RESULT IN THE NEED TO ADJUST SCREEDS AFTER CONCRETE HAS BEEN PLACED TO PRODUCE A LEVEL SURFACE. ADDITIONAL CONCRETE WILL BE REQUIRED, AND IS ANTICIPATED, AT NO EXTRA COST.
- 27. NO CONCRETE SHALL BE PLACED ONTO OR AGAINST SUBGRADES CONTAINING FREE WATER, FROST, ICE OR SNOW.
- 28. DURING WINTER CONSTRUCTION, ALL FOOTINGS SHALL BE PROTECTED FROM FROST PENETRATION UNTIL THE BUILDING IS ENCLOSED AND TEMPORARY HEAT IS
- 29. THE CONCRETE CONTRACTOR SHALL FURNISH MIX DESIGN SHOP DRAWINGS FOR
- ENERAL CONTRACTOR TO PROVIDE SHOP DRAWINGS FOR SIZE, LOCATION AND HEIGHT OF MECHANICAL EQUIPMENT PADS ON CONCRETE SLAB ON STEEL DECK ND SLAB-ON-GRADE.

#### HIGH LIFT GROUTED CONSTRUCTION

- WHERE HIGH LIFT GROUTING IS USED, CONFORM TO THE SPECIFICATIONS AND
- THE CALIFORNIA BUILDING CODE. CLEANOUT OPENINGS SHALL BE PROVIDED AT THE BOTTOM OF EACH POUR OF GROUT. ANY OVERHANGING MORTAR OR OTHER DEBRIS SHALL BE REMOVED FROM THE INSIDES OF CELL WALLS
- THE FOUNDATION OR OTHER HORIZONTAL CONSTRUCTION JOINTS SHALL BE CLEANED OF ALL LOOSE MATERIAL AND MORTAR DROPPINGS BEFORE EACH
- THE CLEANOUTS SHALL BE SEALED BEFORE GROUTING. ALL CELLS SHALL BE AN APPROVED ADMIXTURE REDUCING EARLY WATER LOSS AND PRODUCING AN

#### **DESIGN-BUILD SUBMITTAL ITEMS**

DESIGN, FABRICATION AND INSTALLATION OF DESIGN-BUILD SUBMITTAL ITEMS SHALL CONFORM TO ALL PROJECT REQUIREMENTS. SUPPLIER SHALL SUBMIT COMPLETE DRAWINGS AND CALCULATIONS SIGNED BY AN ENGINEER, REGISTERED IN THE STATE OF THE PROJECT, TO THE GOVERNING AGENCY FOR REVIEW AND PROVAL PRIOR TO FABRICATION.

- 2. DEFERRED SUBMITTAL ITEMS INCLUDE:
- A. STEEL STAIRS B. PREFABRICATED METAL PANEL CLADDING SUPPORT AND ANCHORAGE OF FIRE & LIFE SAFETY EQUIPMENT, UNO

EXPANSION ACTION SHALL BE USED IN THE GROUT.

- D. SOLAR PANELS
- E. GLASS RAIL SYSTEM F. STOREFRONT SYSTEM

#### **REINFORCING STEEL**

**ASTM A1064** 

Fy = 60 KSI

- 1. ALL REINFORCING STEEL SHALL BE DETAILED AND PLACED IN CONFORMANCE WITH THE AMERICAN CONCRETE INSTITUTE "ACI DETAILING MANUAL" (SP-066) EXCEPT AS OTHERWISE SHOWN, NOTED OR SPECIFIED.
- 2. CONCRETE REINFORCING STEEL SHALL BE HIGH STRENGTH NEW BILLET STEEL CONFORMING TO THE FOLLOWING STANDARDS: ASTM A615, GR60 DEFORMED BARS DEFORMED BARS IN SFRS ASTM A706, GR 60 Fy = 60 KSIWELDED WIRE REINFORCING **ASTM A1064** Fv = 65 KSI **EPOXY-COATED BARS** ASTM A775 Fy = 60 KSIGALVANIZED-COATED BARS ASTM A767 Fv = 60 KSI
- 3. MINIMUM CONCRETE COVER SHALL BE PROVIDED AS FOLLOWS TO THE **OUTERMOST REINFORCING BARS:**

STEEL WIRE

- CAST AGAINST AND PERMANENTLY IN CONTACT WITH GROUND 3" EXPOSED TO WEATHER OR IN CONTACT WITH GROUND #6 BARS OR LARGER
  - #5 BARS OR SMALLER NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND SLABS, JOISTS AND WALLS WITH #14 AND #18 BARS SLABS, JOISTS AND WALLS WITH #11 BARS OR SMALLER 3/4"
- BEAMS, COLUMNS, PEDESTALS AND TENSION TIES 4. BAR SPLICES SHALL BE PROVIDED WHERE INDICATED ON THE DRAWINGS. ALL SPLICES SHALL BE CLASS 'B' AS DEFINED IN ACI 318. IF SPLICE LENGTH IS NOT CIVEN ON THE DRAWINGS, PROVIDE LAP LENGTH (IN INCHES) AS FOLLOWS

GIV	EN ON THE DR	AVVINGS, PROV	IH (IN INCHES)	AS FULLOWS.			
	3000 PSI C	ONCRETE	4000 PSI C	ONCRETE	5000 PSI CONCRETE		
BAR SIZE	OTHER	ТОР	OTHER	ТОР	OTHER	ТОР	
#3	22	28	19	25	17	22	
#4	29	38	25	33	23	29	
#5	36	47	31	41	28	36	
#6	43	56	37	49	34	44	
#7	63	81	54	71	49	63	
#8	72	93	62	81	56	72	
#9	81	105	70	91	63	81	
#10	90	116	78	101	69	90	
#11	98	128	85	111	76	99	

- LAP LENGTHS ASSUME CLEAR SPACING BETWEEN BARS OF 2 BAR DIAMETERS, AND A MINIMUM COVER OF 1 BAR DIAMETER. FOR DEVELOPMENT LENGTHS, DIVIDE BY 1.3. TOP BARS ARE DEFINED AS HORIZONTAL BARS WITH MORE THAN 1'-0" ( FRESH CONCRETE BELOW.
- 5. ALL REINFORCING IN CONCRETE USED FOR THE CONTAINMENT OF WATER SHALL BE HOT-DIP GALVANIZED OR EPOXY-COA
- 6. USE LOW HYDROGEN ELECTRODES, GRADE E-90, FOR WELDING OF REINFORCING . PROVIDE ADEQUATE TIES FOR ALL REINFORCING BARS AND STIF
- CONCRETE SLABS AND BEAMS. ANCHOR BOLTS, DOWELS, REINFORCING STEEL, INSERTS, ETC., SHALL BE SECURELY TIED IN PLACE PRIOR TO POURING CONCRETE. CONCRETE BLOCKS SHALL ONLY BE USED TO SUPPORT R
- SUPPORTS FOR REINFORCEMENT SHALL HAVE CLASS 2 P TECTION AS DEFINED IN THE CRSI MANUAL OF STANDARD PRACTICE, UNC 9. SUPPORTS FOR COATED REINFORCEMENT SHALL HAVE CLASS 1 PROTECTION AS
- DEFINED IN THE CRSI MANUAL OF STANDARD PRACTICE, UNO. 10. CONTINUOUS REINFORCING SHALL BE LAPPED AT MIDSPAN FOR TOP BARS AND DIRECTLY OVER THE
- SUPPORT FOR BOTTOM BARS. REINFORCING (WWR) SHALL BE LAPPED 2 PANELS AT EDGES AND ENDS. TWEEN FOOTINGS AND WALLS OR COLUMNS SHALL BE THE SAME GRADE, SIZE AND SPACING
- JMBER AS THE VERTICAL REINFORCING, RESPECTIVELY, UN TOP BARS IN GRADE BEAMS SHALL BE CONTINUOUS OVER SUPPORTS AND LAP AT MID-SPAN WEEN SUPPORTS. WHERE GRADE BEAMS ARE SIMPLE SPAN, TOP BARS SHALL BE CONTINUOUS FOR
- L LENGTH AND HOOKED DOWN AT EACH E BOTTOM BARS IN GRADE BEAMS SHALL BE CONTINUOUS BETWEEN SUPPORTS AND LAP OVER
- EINFORCING IN WALL FOOTINGS BETWEEN COLUMNS SHALL EXTEND INTO COLUMN FOOTINGS A INIMUM OF 2 FEET. INFORCING IN FOOTINGS AND GRADE BEAMS SHALL BE ACCURATELY PLACED, SPACED, SUPPORTED
- D SECURED BEFORE PLACING CONCRETE TING OF REINFORCING WHICH CONFLICTS WITH EMBEDDED OBJECTS IS NOT ACCEPTABLE. FORCING BARS SHALL BE BENT COLD, AND NO METHOD OF FABRICATION SHALL BE USED WHICH JLD BE INJURIOUS TO THE MATERIAL. HEATING OF BARS FOR BENDING IS NOT PERMITTED.
- D WELDING OR BENDING OF REINFORCING IS NOT PERMITTED EXCEPT AS INDICATED ON THE OR AS APPROVED BY THE STRUCTURAL ENGINEER. . SUBMIT SHOP DRAWINGS FOR FABRICATION AND PLACEMENT OF REINFORCING STEEL. INCLUDE SCHEDULES AND DIAGRAMS OF BENT BARS AND SHOW ARRANGEMENT OF REINFORCEMENT.
- STRUCTURAL ENGINEER'S REVIEW WILL BE FOR COMPLIANCE WITH DESIGN REQUIREMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING DIMENSIONS AND QUANTITIES.

#### **MASONRY**

MINIMUM 28-DAY COMPRESSIVE STRENGTHS FOR MASONRY CONSTRUCTION

	DESIGN ASSEMBLY STRENGTH, f'm	2000 PSI
	INDIVIDUAL CONCRETE MASONRY UNITS	2800 PSI
	MORTAR	1800 PSI
	GROUT	2000 PSI
2.	MASONRY MATERIALS SHALL CONFORM TO	THE FOLLOWING STANDARDS:
	CONCRETE MASONRY UNITS (CMU)	ASTM C90, GRADE N-1
	MORTAR	ASTM C270, TYPE S
	GROUT	ASTM C476
	REINFORCING STEEL	ASTM A615, GR 60
	PLATE AND BENT BAR ANCHORS	ASTM A36
	SHEET METAL ANCHORS AND TIES	ASTM A1008
	WIRE MESH TIES	ASTM A1064
	WIRE TIES AND ANCHORS	ASTM A951

- ANCHOR BOLTS ASTM A307, GRADE A 3. BAR SPLICES SHALL BE PROVIDED WHERE INDICATED ON THE DRAWINGS. IF SPLICE LENGTH IS NOT GIVEN ON THE DRAWINGS, PROVIDE LAP LENGTHS (IN
- INCHES) AS FOLLOWS EXCEPT BARS LARGER THAN #9 SHALL BE MECHANICALLY 4. LOAD BEARING MASONRY SHALL HAVE FULL HEIGHT 9 GAUGE MINIMUM
- HORIZONTAL REINFORCEMENT NOT TO EXCEED 16" OC VERTICALLY. 5. ALL LOAD BEARING MASONRY WALLS TO HAVE FULL BED, HEAD AND COLLAR . ALL CELLS SHALL BE FILLED WITH GROUT, UNLESS NOTED OTHERWISE.
- PROVIDE A MINIMUM OF 1 INCH GROUT BETWEEN MAIN REINFORCING AND/OR BOLTS AND MASONRY UNIT FACE. VERTICAL REINFORCEMENT SHALL BE
- CENTERED IN WALL, UNO. 8. CELLS SHALL BE IN VERTICAL ALIGNMENT. DOWELS IN FOOTINGS SHALL BE SET TO
- ALIGN WITH CORES CONTAINING REINFORCING STEEL. 9. ALL CELLS CONTAINING REINFORCING SHALL BE FILLED SOLID WITH GROUT, AND ALSO WHERE NOTED ON THE DRAWINGS. 10. STACK BOND LAID MASONRY SHALL HAVE VERTICAL REINFORCEMENT AT

12. REFER TO ARCHITECTURAL DRAWINGS FOR SURFACE AND HEIGHT OF UNITS

- MAXIMUM 16" OC SPACING. 11. COORDINATE ANY UNIDENTIFIED PIPE OR DUCT PASSING THROUGH STRUCTURAL MASONRY WALLS, UNLESS NOTED OR DETAILED SPECIFICALLY.
- LAYING PATTERN AND JOINT TYPE. ALL BLOCK SHALL BE RUNNING BOND, UNO. 13. THE LOAD BEARING CONCRETE MASONRY WALLS FOR THIS PROJECT WERE DESIGNED TO SPAN VERTICALLY AND BE BRACED BY THE ROOF AND FLOOR FRAMING ELEMENTS OF THE STRUCTURE. DURING CONSTRUCTION, THE MASONRY CONTRACTOR SHALL PROVIDE LATERAL BRACING UNTIL THE ROOF STRUCTURE IS INSTALLED AS RECOMMENDED BY ACI 530 TMS 402/602 AND THE "STANDARD PRACTICE FOR BRACING MASONRY WALLS UNDER CONSTRUCTION", PREPARED BY THE COUNCIL FOR MASONRY WALL BRACING. THIS BRACING IS TO PREVENT UNNECESSARY STRESS OR DAMAGE TO THE MASONRY WALLS FROM WIND LOADS, WHICH CAN OCCUR WHILE THE WALLS ARE NOT PROPERLY BRACED
- BY THE ROOF AND FLOOR STRUCTURE. ackslash 14. THE MASONRY CONTRACTOR SHALL FURNISH SHOP DRAWINGS OF PRODUCT DATA, REINFORCEMENT DETAILS, AND MIX DESIGNS FOR ARCHITECT/STRUCTURAL ENGINEER'S REVIEW BEFORE FABRICATION.

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Y	SJ-203	SUPPORT BUILDING ROOF PLAN
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	S-700	TYPICAL EXTERIOR METAL STUD DETAILS
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	S-709	TYPICAL METAL STUD DETAILS
	S-710	TYPICAL METAL STUD DETAILS
	S-711	TYPICAL FRAMING DETAILS

STRUCTURAL SHEET INDEX

#### **FOUNDATION**

1. FOUNDATION DESIGN IS BASED ON SOILS REPORTS BY: CONVERSE CONSULTANTS, DATED: JANUARY 18, 2022, CONVERSE PROJECT NO. 22-81-206-01

**EQUIPMENT ANCHORAGE DETAILS** 

GRAND TOTAL: 64

- FOUNDATION TYPE: SPREAD FOOTING 3. DESIGN ALLOWABLES: A. SOIL BEARING: 2.500 PSF (DL+LL), 3.325 PSF (WIND/EQ) BASED ON MIN 18" WIDE WALL
- FOOTING AND 18" SQUARE PAD FOOTING (18" MINIMUM DEPTH) W/ INCREASE OF 100 PSF PER ADD'L FOOT OF WIDTH AND 500 PSF PER ADD'L FOOT OF DEPTH TO MAXIMUM 3,500 PSF B. LATERAL BEARING: 220 PSF/FT (PASSIVE PRESSURE, MAX 2,500 PSF) C. COEFFICIENT OF FRICTION: 0.35
- 4. FOLLOW RECOMMENDATIONS IN SOIL REPORT FOR ALL FOUNDATION WORK THE SOILS ENGINEER SHALL VERIFY CONDITION AND/OR ADEQUACY OF ALL EXCAVATIONS, SUB GRADES, FILLS AND BACK FILLS. NO REINFORCEMENT OR CONCRETE SHALL BE PLACED IN ANY EXCAVATION OR ON ANY SUBGRADE OR FILL UNTIL THAT WORK HAS BEEN REVIEWED AND APPROVED IN WRITING BY THE SOILS ENGINEER.
- $\sqrt{1}$   $\{$  6.  $\}$ ALL FOOTINGS SHALL BEAR ON COMPACTED STRUCTURAL FILL. THE TOP OF FOOTING ELEVATIONS ARE SHOWN ON THE PLANS. WHERE SOFT OR LOOSE MATERIAL IS FOUND AT BOTTOM OF FOOTING ELEVATIONS, THE SOFT OR LOOSE MATERIAL SHALL BE REMOVED AND REPLACED WITH COMPACTED STRUCTURAL FILL AS DIRECTED BY THE SOILS ENGINEER.
- THE EARTH AT CONTRACTOR'S OPTION REQUIRE THE FOLLOWING PRECAUTIONS: A. SIDES OF EXCAVATION MUST BE VERTICAL (OVER POURING AND MUSHROOMING NOT ALLOWED)

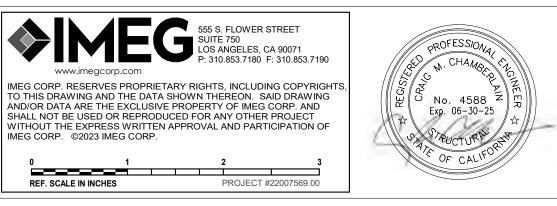
THE SIDES OF FOUNDATIONS SHOWN STRAIGHT ARE FORMED. FOUNDATIONS POURED AGAINST

- CONTRACTOR SHALL BE RESPONSIBLE FOR CLEAN UP OF SOIL SLOUGHING BEFORE, DURING, AND AFTER POUR. 8. CONTRACTOR TO PROVIDE FOR DE-WATERING OF EXCAVATION FOR EITHER SURFACE WATER,
- 9. BACK FILL OVER EXCAVATED FOOTINGS WITH CONCRETE OF SAME DESIGN STRENGTH AS FOOTING CONCRETE OR COMPACTED STRUCTURAL FILL, AS DIRECTED OTHERWISE BY THE SOILS ENGINEER.

GROUND WATER OR SEEPAGE IF REQUIRED.

STRUCTURAL ENGINEER'S APPROVAL

- 10. STEP CONTINUOUS FOOTINGS AT VARYING ELEVATIONS PER TYPICAL DETAIL. SLOPING OF
- 11. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE LOCATION AND PROTECTION OF ALL EXISTING UTILITIES, EXISTING STRUCTURES, ETC., WHETHER INDICATED OR NOT, WHICH MAY BE AFFECTED BY THE CONSTRUCTION PROCESS 12. UTILITY LINES SHALL NOT BE PLACED THROUGH OR BELOW FOUNDATIONS WITHOUT THE
- 13. SLABS ON GRADE SHALL BE SUPPORTED ON NATURAL GRADE OR COMPACTED STRUCTURAL FILL ACCORDING TO THE RECOMMENDATIONS OF THE SOILS REPORT. 14. THE SLOPE BETWEEN THE LOWER EDGES OF ADJACENT FOUNDATIONS SHALL NOT EXCEED 45
- DEGREES WITH THE HORIZONTAL, UNLESS INDICATED OTHERWISE IN THE DRAWINGS. MAINTAIN A 1:1 SLOPE FROM BOTTOM EDGE OF ANY EXCAVATION. 15. DURING BACKFILLING OPERATIONS, FOUNDATION WALL BACKFILL SHALL NOT BE UNBALANCED
- BY MORE THAN TWO FEET ON EITHER SIDE AT ANY TIME. 16. THE CONTRACTOR SHALL PROVIDE FOR THE DESIGN AND INSTALLATION OF ALL CRIBBING. SHEETING AND SHORING ETC. REQUIRED FOR CONSTRUCTION OF THE PROJECT AND SHALL BE SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS AND UTILITIES.







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**REVISIONS/ADDENDA** 

06/12/24 PLAN CHECK RESUBMITTAL

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**PROJECT INFORMATION** 

Project Number: 22007569.00 Drawn By: Checked By:

06/12/2024

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**GENERAL NOTES** 

**SHEET NUMBER** 

Of Sheet

#### **COLD-FORMED STEEL FRAMING (CFSF) SYSTEM**

- MATERIAL, DESIGN AND MANUFACTURE SHALL BE IN ACCORDANCE WITH THE "STANDARD FOR COLD-FORMED STEEL FRAMING - GENERAL PROVISIONS" OF THE AMERICAN IRON AND STEEL INSTITUTE
- STEEL STUDS, HEADERS, TRACKS, AND OTHER ELEMENTS USED FOR THIS PROJECT ARE SIZED BASED ON SSMA. ELEMENTS OF EQUAL OR GREATER CAPACITY MAY BE EXCHANGED.
- 3. STRUCTURAL CFSF SHALL BE SUPPLIED BY A CURRENT MEMBER OF THE STEEL STUD MANUFACTURERS ASSOCIATION.
- COLD-FORMED STRUCTURAL STUDS SHALL CONFORM TO THE FOLLOWING STANDARDS
  - ROLLED SECTIONS, CONNECTION MATERIAL, AND STIFFENER PLATES 18 GAUGE AND THINNER ASTM A653, GR 33 Fv = 33 KSI 16 GAUGE AND THICKER ASTM A653, GR 50  $F_V = 50 \text{ KSI}$ Fy = 36 KSIASTM A36
  - CONNECTION MATERIAL >3/16" ASTM F1554, GR 36 ANCHOR RODS Fv = 36 KSIASTM A307 Fv = 10 KSIHOT-DIP COATING ASTM A924, G60 **ELECTRO-PLATE COATING** ASTM A591 ALUMINUM-ZINC COATING ASTM A792, GR 40

ASTM C955 AND ASTM C1007

AWS 5.1. E60XX

- STRUCTURAL COLD FORM STEEL FRAMING IS DEFINED AS THE FOLLOWING: A. ANY COLD FORMED FRAMING THICKER THAN 20 GA (33 MIL)
- B. ANY EXTERIOR COLD FORMED FRAMING C. ALL OTHER STEEL STUD FRAMING IS NON-STRUCTURAL AND NOT A PART OF THE STRUCTURAL PACKAGE
- STRUCTURAL CFSF IS PERFORMANCE SPECIFIED. DESIGN INFORMATION INCLUDED IN THESE DOCUMENTS IS TO BE CONSIDERED A GUIDELINE FOR BIDDING PURPOSES ONLY. STUD DEPTH IS REQUIRED TO MEET THOSE INDICATED ON THE PLANS, CONNECTION DETAILS ARE ONLY AN INDICATION OF SUGGESTED SUPPORT AND SLIP JOINT ORIENTATION. GAUGE, SECTION, MATERIAL, BRACING, CONNECTIONS, STIFFENERS, AND SIMILAR DETAILS ARE THE RESPONSIBILITY OF THE MANUFACTURER BASED ON LOADS GIVEN ON THE PLANS.
- PROVIDE TRACKS, BLOCKING, HEADERS, CLIP ANGLES, BRIDGING, SHOES, REINFORCEMENTS, FASTENERS AND ACCESSORIES TO PROVIDE A COMPLETE METAL FRAME SYSTEM IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.
- BEARING STUDS MUST BE FABRICATED WITH FULL STUD END SEATED AGAINST
- TRACK WEB. DO NOT USE STUD THAT HAS BEEN CUT AT PUNCHOUT. 9. FRAMING FABRICATOR SHALL ENSURE PUNCHOUT ALIGNMENT WHEN
- ASSEMBLING FRAMING AND FIELD CUTTING TO LENGTH.
- 10. TRACK SHALL BE THE SAME SIZE AND GAUGE AS THE STUD. 11. UNIFORM BEARING SURFACE SHALL BE PROVIDED WITH A MAXIMUM 1/4 INCH GAP BETWEEN BOTTOM TRACK AND THE FOUNDATION OR SLAB. IT SHALL BE ACCOMPLISHED BY THE USE OF LOAD BEARING SHIMS AND/OR GROUT PROVIDED BETWEEN THE UNDERSIDE OF THE WALL BOTTOM TRACK AND THE TOP OF THE FOUNDATION OR SLAB AT EACH STUD.
- 12. IN THE EVENT A TRACK BUTT JOINT OCCURS WITHIN A PANEL, ABUTTING PIECES OF TRACK SHALL BE BUTT WELDED OR SPLICED TOGETHER. NO SUCH SPLICES SHALL OCCUR AT ANY HEAD OR SILL CONDITION.
- 13. FRAME WALL OPENINGS LARGER THAN 2'-0" SQUARE WITH DOUBLE STUD AT EACH JAMB OF FRAME EXCEPT WHERE MORE THAN 2 ARE SHOWN OR INDICATED. INSTALL RUNNER TRACKS AND JACK STUDS ABOVE AND BELOW WALL OPENINGS. ANCHOR TRACKS TO JAMB STUDS BY WELDING, AND SPACE JACK STUDS SAME AS FULL HEIGHT STUDS OF WALL. SECURE STUD SYSTEM WALL OPENING FRAME IN
- 14. INSTALL HORIZONTAL BRIDGING IN STUD SYSTEM NOT MORE THAN 5'-0" ON CENTER. FASTEN AT EACH STUD INTERSECTION.
- 15. UNLESS OTHERWISE NOTED, ATTACH MATERIALS BY BOLTING OR SCREW FASTENERS.
- A. SCREW CONNECTIONS:

INSTALLATION

ELECTRODES FOR ARC WELDING

- 1) SCREWS LARGER THAN SPECIFIED MAY BE USED, PROVIDED THE MINIMUM SPACING AND EDGE DISTANCE REQUIREMENTS ARE MET 2) SCREWS SHALL BE FULLY DRIVEN AND HAVE A MINIMUM PENETRATION OF THREE THREADS THROUGH THE LAST MATERIAL JOINED. 3) SCREWS SHALL HAVE A PROTECTIVE COATING COMPLYING WITH
- RECOGNIZED DESIGN STANDARDS FOR THE PROJECT ENVIRONMENTAL CONDITIONS.
- **BOLT CONNECTIONS:** 1) BOLTS SHALL MEET OR EXCEED THE REQUIREMENTS OF ASTM A307 AND SHALL BE INSTALLED WITH NUTS AND WASHERS AT SPACING PER RECOGNIZED DESIGN STANDARD.
- 1) ALL WELDED CONNECTIONS ARE TO BE PERFORMED IN ACCORDANCE WITH AMERICAN WELDING SOCIETY (AWS) D1.3 FOR WELDING SHEET
- STEEL IN STRUCTURES. 16. UNLESS INDICATED OTHERWISE ON THE DRAWINGS OR IN SHEAR WALL PANELS. FASTEN GYPSUM BOARD WITH #6 x 1" BUGLE HEAD SCREWS AT 12" OC AT ALL
- SUPPORTS AND EDGES. 17. WEB CRIPPLING BASED ON MINIMUM 10" UNPUNCHED STEEL AT BOTH ENDS.
- 18. ALL FRAMING COMPONENTS SHALL BE CUT SQUARELY FOR ATTACHMENT TO PERPENDICULAR MEMBERS.
- 19. ALL FIELD CUTTING OF STUDS SHALL BE DONE BY SAWING. 20. PREFABRICATED COLD FORM TRUSSES:
- A. DESIGN, FABRICATE, TRANSPORT AND ERECT COLD FORM TRUSSES IN ACCORDANCE WITH AISI S214 STANDARDS AND MANUFACTURER'S RECOMMENDATIONS.
- B. DESIGN FOR LOADS. IN ADDITION TO MEMBER WEIGHTS. AS GIVEN IN THE
- DESIGN CRITERIA NOTES AND AS NOTED ON THE DRAWINGS. PRE-FABRICATED PRE-ENGINEERED TRUSSES ARE PERFORMANCE SPECIFIED. DESIGN INFORMATION INCLUDED IN THESE DOCUMENTS IS TO BE
- CONSIDERED SCHEMATIC. SECTION, BRACING, CONNECTIONS, AND SIMILAR DETAILS ARE THE RESPONSIBILITY OF THE MANUFACTURER BASED ON LOADS GIVEN ON THE PLANS AND SPECIFICATIONS.
- D. TRUSS CONTRACTOR SHALL BE RESPONSIBLE FOR SUPPLYING THE PROPERLY SIZED ANCHORAGE FOR TRUSS-TO-TRUSS CONNECTIONS. ALL PERMANENT AND TEMPORARY BRACING AND FASTENING AT BEARINGS SHALL BE DESIGNED BY THE TRUSS MANUFACTURER, UNO.
- . CONSTRUCTION SHALL NOT BEGIN UNTIL SHOP DRAWINGS AND CALCULATIONS HAVE BEEN REVIEWED BY THE ARCHITECT/STRUCTURAL ENGINEER. SUBMIT COMPLETE TECHNICAL INFORMATION ON ALL COLD-FORMED STEEL STRUCTURAL MEMBERS, INCLUDING SECTION PROPERTIES, ALLOWABLE DESIGN STRESSES, DESCRIPTION OF CONNECTIONS AND FINISHES. DO NOT PROCEED WITH INSTALLATION UNTIL SUBMITTALS HAVE BEEN REVIEWED AND RETURNED.

#### **MECHANICAL SYSTEMS AND SUSPENDED CEILINGS**

- HVAC DUCTWORK SHALL BE INSTALLED PER ASCE 7-16 SECTION 13.6.6 UNDER EXCEPTION 2. THE FOLLOWING CONDITIONS ARE APPLICABLE:
- A. PROVISIONS ARE MADE TO AVOID IMPACT WITH OTHER DUCTS OR
- MECHANICAL COMPONENTS OR TO PROTECT THE DUCTS IN THE EVENT OF SUCH IMPACT. B. THE DISTRIBUTION SYSTEM IS POSITIVELY ATTACHED TO THE STRUCTURE.
- C. HVAC DUCTS HAVE A CROSS SECTIONAL AREA OF LESS THAN 6 SQ. FT. AND WEIGH LESS SUSPENDED CEILING SHALL BE INSTALLED PER ASCE 7-16 SECTION 13.5.6.2.2 AND SHALL
- COMPLY WITH THE FOLLOWING:
- A. THE WIDTH OF PERIMETER CLOSURE ANGLE OR CHANNEL SHALL NOT BE LESS THAN 2" UNLESS QUALIFIED PERIMETER SUPPORTING CLIPS ARE USED.

#### STRUCTURAL OBSERVATION

THE OWNER SHALL EMPLOY THE ENGINEER RESPONSIBLE FOR THE STRUCTURAL DESIGN TO PERFORM STRUCTURAL OBSERVATION AS DEFINED IN IBC SECTION 1704. OBSER DEFICIENCIES SHALL BE REPORTED IN WRITING TO THE OWNER'S REPRESENTATIVE, SPEC INSPECTOR, CONTRACTOR AND THE BUILDING OFFICIAL THE STRUCTURAL OBSERVER SUBMIT TO THE BUILDING OFFICIAL A WRITTEN STATEMENT THAT THE SITE VISITS HAVE BEEN MADE AND IDENTIFYING ANY REPORTED DEFICIENCIES WHICH, TO THE BEST OF THE STRUCTURAL OBSERVER'S KNOWLEDGE, HAVE NOT BEEN RESOLVED.

- STRUCTURAL OBSERVATION SHALL BE PERFORMED FOR THE FOLLOWING CONSTRUCTION STAGES:
- A. PRIOR TO CONCRETE POUR(S) AT: 1) FIRST PLACEMENT @ FOUNDATIONS
- 2) FIRST PLACEMENT @ GROUND LEVEL SLABS
- 3) FIRST PLACEMENT @ ABOVE GROUND WALLS & COLUM B. PRIOR TO COVER OF:
- 1) WALL FRAMING SYSTEMS 2) FLOOR FRAMING SYSTEMS 3) ROOF FRAMING SYSTEMS

#### **STEEL**

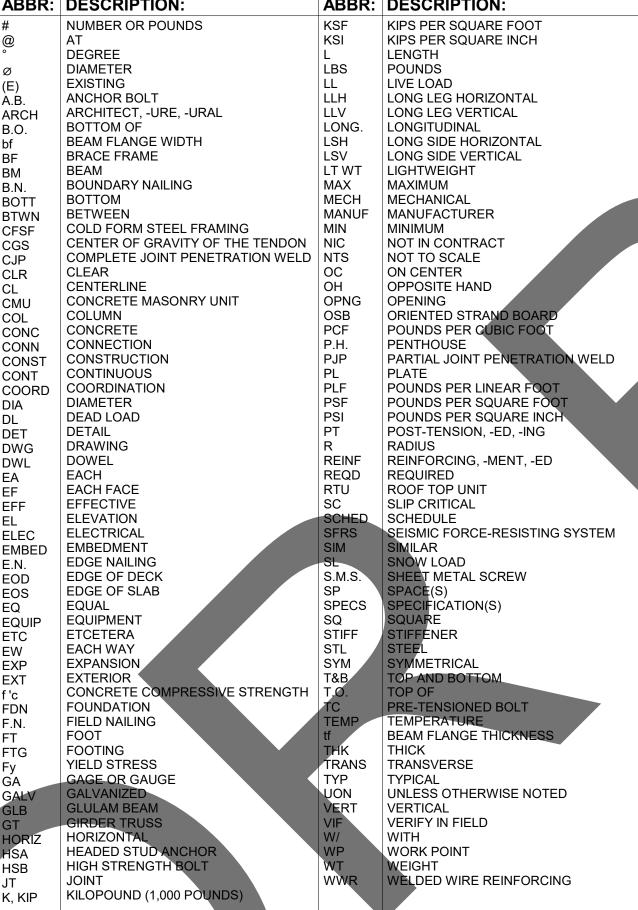
- 1. STRUCTURAL STEEL SHALL BE DETAILED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) "DETAILING FOR STEEL CONSTRUCTION" AND FABRICATED AND ERECTED IN ACCORDANCE WITH THE
- "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" 2. STRUCTURAL STEEL SHALL CONFORM TO ASTM STANDARDS AS NOTED BELOW: WIDE FLANGE SHAPES ASTM A992 Fy = 50 KSIOTHER ROLLED SHAPES Fy = 36 KSI ASTM A36 PIPE SECTIONS ASTM A53, GR B Fy = 35 KSI
  - HSS SECTIONS, ROUND ASTM A500, GR C Fy = 46 KSI Fy = 46 KSI HSS SECTIONS, SQ/RECT ASTM A500. GR B BASE AND CONNECTION PLATES ASTM A36 Fv = 36 KSIASTM F1554, GR 36 Fv = 36 KSI ANCHOR RODS Fv = 120 KSI HIGH STRENGTH BOLTS ASTM F3125, GR A325 HIGH STRENGTH BOLTS ASTM F3125, GR A490 Fv = 150 KSIHIGH STRENGTH TWIST-OFF BOLTS ASTM F3125, GR F1852 Fv = 120 KSI HIGH STRENGTH TWIST-OFF BOLTS ASTM F3125, GR F2280 Fv = 150 KSI
- HEAVY HEX NUTS ASTM A563 WASHERS ASTM F436 **HEADED STUDS** ASTM A108, TYPE B ELECTRODES FOR ARC WELDING AWS 5.1, E70XX
- 3. HIGH STRENGTH BOLTS SHALL BE INSTALLED IN ACCORDANCE WITH AISC "SPECIFICATIONS FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS" REFER TO DETAILS FOR BOLT SIZE AND MATERIAL ASTM DESIGNATION.
- 4. USE TENSION-CONTROL, "TWIST-OFF", BOLTS FOR ALL HIGH STRENGTH BOLTS REQUIRING FULL TENSION AS INDICATED ON THE DRAWINGS. 5. ALL HIGH STRENGTH BOLTS SHALL CONFORM TO ASTM F3125, GRADE A325N, UNO. FOR ALL DRAG STRUT BOLTS, HIGH STRENGTH BOLTS SHALL CONFORM TO ASTM
- F3125, GRADE A490SC. 6. ALL BOLTED CONNECTIONS SHALL BE BEARING TYPE, UNLESS NOTED
- OTHERWISE 7. STANDARD BOLT HOLES IN STEEL SHALL BE 1/16 INCH LARGER IN DIAMETER THAN NOMINAL SIZE OF BOLT USED, UNO.
- 8. BOLTS IN SLOTTED HOLES SHALL BE LOCATED IN THE CENTER OF THE HOLE AFTER FIELD ASSEMBLY IS COMPLETE, UNLESS DETAILED OTHERWISE 9. ALL WELDS SHALL CONFORM TO THE AMERICAN WELDING SOCIETY "STRUCTURAL WELDING CODE - STEEL" (AWS D1.1), "STRUCTURAL WELDING CODE - SEISMIC
- SUPPLEMENT" (AWS D1.8). AND BE MADE WITH APPROVED ELECTRODES. 10. WELD LENGTHS INDICATED ON THE DRAWINGS ARE THE NET EFFECTIVE LENGTH REQUIRED. WHERE FILLET WELD SYMBOL IS GIVEN WITHOUT INDICATION OF SIZE USE MINIMUM SIZE WELDS AS SPECIFIED IN AISC 360, SECTION J2.4 AND CHAPTER 22 OF THE CALIFORNIA BUILDING CODE.
- 11. USE BACKING FOR ALL FULL PENETRATION WELDS. ALL FULL AND/OR PARTIAL
- PENETRATION WELDS SHALL BE FULLY DETAILED ON THE SHOP DRAWINGS. 12. WELD ACCESS HOLES SHALL BE FABRICATED IN ACCORDANCE WITH THE
- **RECOMMENDATIONS OF AWS D1.1.** 13. ALL WELDING OF STRUCTURAL STEEL SHALL BE PERFORMED BY CERTIFIED WELDERS WITH EXPERIENCE AND CERTIFICATION IN THE TYPES OF WELDING CALLED FOR. WELDERS SHALL HAVE BEEN RECENTLY QUALIFIED AS PRESCRIBED
- IN "QUALIFICATION PROCEDURES" OF THE AMERICAN WELDING SOCIETY (AWS). 14. FIELD CONNECTIONS SHALL BE WELDED OR BOLTED. SHOP CONNECTIONS SHALL BE WELDED, UNO. WELDS INDICATED WITH A SHOP WELD SYMBOL MAY BE MADE IN THE FIELD WITH THE APPROVAL OF THE STRUCTURAL ENGINEER. LOCATIONS OF ALL FIELD WELDS SHALL BE CLEARLY SHOWN ON THE SHOP DRAWINGS. WELDS SHALL BE DESIGNED TO BE FULLY EQUIVALENT IN STRENGTH TO BOLTED CONNECTIONS DETAILED TO MINIMIZE BENDING IN THE CONNECTION.
- A. SHALL BE FABRICATED IN ACCORDANCE WITH AWS D1.1 AND WITHIN THE TOLERANCES SET FORTH IN AWS D1.1.
- B. SHALL BE NELSON GRANULAR FLUX-FILLED SHEAR CONNECTOR OR ANCHOR STUDS (OR APPROVED EQUIVALENT). STUDS SHALL BE MANUFACTURED OF
- COLD ROLLED STEEL WHICH CONFORMS TO ASTM A108. C. STUDS SHALL BE AUTOMATICALLY END WELDED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS IN SUCH A MANNER AS TO PROVIDE COMPLETE FUSION BETWEEN THE END OF THE STUD AND THE PLATE. THERE SHOULD BE NO POROSITY OR EVIDENCE OF LACK OF FUSION BETWEEN THE WELDED END OF THE STUD AND THE PLATE. THE STUD SHALL DECREASE IN LENGTH DURING WELDING APPROXIMATELY 1/8" FOR 5/8"ø AND SMALLER AND 3/16" FOR LARGER THAN 5/8"ø. WELDING SHALL BE DONE ONLY BY QUALIFIED
- WELDERS APPROVED BY THE INSPECTION AGENCY. 16. ALL STEEL EXPOSED TO WEATHER SHALL BE HOT-DIP GALVANIZED AFTER FABRICATION. ABRADED AREAS TO BE TOUCHED UP WITH GALVALOY. ALL HOLLOW SECTIONS SHALL HAVE WELDED CAP PLATES TO SEAL EXPOSED ENDS.
- 17. PROVIDE ONE SHOP COAT OF PAINT ON STRUCTURAL STEEL EXPOSED TO PUBLIC 18. REFER TO DRAWINGS FOR DETAIL OF DECK OPENINGS. REFER TO
- ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS, ETC. FOR EXAC
- SIZE, LOCATION, AND COUNT OF REQUIRED OPENINGS. 19. CUTS, HOLES, OPENINGS, ETC., REQUIRED IN STRUCTURAL STEEL MEMBERS FOI THE WORK OF OTHER TRADES SHALL BE SHOWN ON THE SHOP DRAWING BURNING OF HOLES AND CUTS IN THE FIELD SHALL NOT BE ALLOWED, WRITTEN AUTHORIZATION FROM THE STRUCTURAL ENGINEER. NO HOLES S BE CUT IN STRUCTURAL STEEL BY OTHER TRADES UNLESS SHOWN ON STRUCTURAL DRAWINGS OR APPROVED IN WRITING BY THE STRUCTURA
- ENGINEER. 20. FURNISH AND INSTALL MISCELLANEOUS STEEL (CURBS, HANGERS, EXPANSION JOINT ANGLES, STRUTS, ETC.) AS CALLED FOR OR AS NECESSARY PER ARCHITECTURAL AND MECHANICAL/ELECTRICAL DRAWINGS.
- 21. NON-SHRINK GROUT FOR BASE AND BEARING PLATES SHALL BE A PRE-MIXED, NON-METALLIC, NON-CORROSIVE, NON-STAINING PRODUCT CONTAINING SELECTED SILICA SAND, PORTLAND CEMENT, SHRINKAGE COMPENSATING AGENTS AND PLASTICIZING/WATER REDUCING AGENTS. MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE 7000 PSI.
- 22. THE STRUCTURAL STEEL FABRICATOR SHALL FURNISH SHOP DRAWINGS OF ALL STRUCTURAL STEEL FOR ARCHITECT/STRUCTURAL ENGINEER'S REVIEW BEFORE

#### STEEL DECK

- STEEL DECK SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE SPECIFICATIONS OF
- THE STEEL DECK INST JSTICAL DECK REQUIRED WHERE INDICATED]: 2. ROOF DECKING [ACO

- A. TYPE B DECK 1 1/2" DEEP X GA, AS INDICATED, GALVANIZED G60 ASTM A 653 DESIGNATION SS GRADE 33 OR 1.) MINIMUM ATTACHMENT REQUIREMENTS:
  - a. INTERIOR BEARING SUPPORTS-5/8" DIA. PUDDLE WELDS AT [4] PER 36" WIDE SHEET. b. AT SEAMS: VSC2 SIDELAPS @ 12" OC.
- c. AT PARALLEL EDGE: 5/8" DIA. PUDDLE WELDS @ [12"] OC.
  d. AT EDGE CLOSURE TO FULL WIDTH SHEET: No. 10 SCREWS @ 4" OC.
  e. EXTERIOR BEARING SUPPORTS: 5/8" DIA. PUDDLE WELDS AT [4] PER 36" WIDE SHEET. FLOOR DECKING:
- A. TYPE W DECK-DEPTH AND GAGE AS INDICATED, GALVANIZED G60 VENTED COMPOSITE TYPE, ASTM A 653 DESIGNATION SS GRADE 33 OR ASTM A924. 1.) MINIMUM ATTACHMENT REQUIREMENTS:
  - a. INTERIOR BEARING SUPPORTS: 5/8" DIAMETER PUDDLE WELDS OR POWDER ACTUATED FASTENERS AT 4 PER 36" WIDE SHEET.
  - b. SEAMS: VSC2 SIDELAPS @ 12" OC.c. PARALLEL EDGE: 5/8" DIA. PUDDLE WELDS @ 18" OC.
  - H. EDGE CLOSURE TO FULL WIDTH SHEET: No. 10 SCREWS @ 4" OC.
- e. EXTERIOR BEARING SUPPORTS 5/8" DIA. PUDDLE WELDS AT 4 PER 36" WIDE SHEET. B. PROVIDE 14 GA. MIN. EDGE CLOSURE PIECES, REFER TO DETAILS.
- C. SHEAR STUD CONNECTORS MAY BE SUBSTITUTED FOR WELDS ON A ONE TO ONE BASIS. D. CONCRETE FILL THICKNESS SHOWN ON FRAMING PLANS AND DETAIL SHEETS ARE MINIMUM THICKNESS. NO ALLOWANCES HAVE BEEN SHOWN FOR ADDITIONAL CONCRETE FILL REQUIRED. TO COMPENSATE FOR BEAM OR DECK DEFLECTIONS AND TO MAINTAIN SURFACE TOLERANCES
- SPECIFIED. METAL DECK PANELS ARE TO BE THREE-SPAN CONTINUOUS WHERE EVER POSSIBLE. AT COMPOSITE DECK SPANS WHERE BEAM CENTERLINES ARE EQUAL TO OR GREATER THAN 11'-6" 3 SPAN DECK CONDITION ONLY OR PROVIDE SHORING IF LESS THAN 3 SPAN CONDITION IS USED. SURE PLATES, FLASHING ETC. IF NOT NOTED, SHALL BE 16-GAGE MINIMUM
- ONFORMING TO THE SAME REQUIREMENTS AS THE DECK. SEE METAL DECK DETAILS. MINIMUM BEARING OF DECKING ON SUPPORTS SHALL BE 2".
- SUPPORT SHALL BE PROVIDED AT ALL EDGES OF OPENINGS IN STEEL DECKS. REFER TO DETAILS. MISCELLANEOUS ITEMS SUCH AS SUSPENDED CEILING SYSTEMS AND LIGHT FIXTURES SHALL NOT BE HUNG DIRECTLY FROM STEEL DECK. EXCEPT USING SPECIFIC METHODS SHOWN IN THE TYPICAL STRUCTURAL DETAILS.

#### STRUCTURAL ABBREVIATION KEY ABBR: DESCRIPTION: ABBR: DESCRIPTION: NUMBER OR POUNDS KIPS PER SQUARE FOOT KSI KIPS PER SQUARE INCH DEGREE LENGTH LBS DIAMETER POUNDS EXISTING LIVE LOAD ANCHOR BOLT LONG LEG HORIZONTAL À.B. ARCHITECT, -URE, -URAL LLV LONG LEG VERTICAL ARCH LONG. BOTTOM OF LONGITUDINAL B.O. LSH BEAM FLANGE WIDTH LONG SIDE HORIZONTA LSV BRACE FRAME LONG SIDE VERTICAL BF LT WT LIGHTWEIGHT BM BOUNDARY NAILING MAX B.N. MAXIMUM воттом MECH MECHANICAL BOTT BTWN **BETWEEN** MANUF MANUFACTURER COLD FORM STEEL FRAMING CFSF MINIMUM CENTER OF GRAVITY OF THE TENDON | NIC NOT IN CONTRACT CGS COMPLETE JOINT PENETRATION WELD NTS CJP NOT TO SCALE CLEAR ON CENTER CLR OC CENTERLINE OH OPPOSITE HAND CONCRETE MASONRY UNIT OPNG **OPENING** CMU COL OSB ORIENTED STRAND BO COLUMN CONC CONCRETE POUNDS PER GUB P.H. CONNECTION PENTHOUSE CONN PARTIAL JOINT PENETRATION WELD PJP CONSTRUCTION CONST CONT CONTINUOUS PLATE PLF POUNDS PER LINEAR FOO COORDINATION COORD PSF PSI POUNDS PER SQUARE FO DIA DIAMETER DL DET DWG DEAD LOAD POUNDS PER SQUARE INCH POST-TENSION, -ED, -ING DETAIL **DRAWING** RADIUS DWL EA REINF REINFORCING, -MENT, -ED DOWEL EACH REQD REQUIRED EACH FACE RTU ROOF TOP UNIT EF EFFECTIVE SLIP CRITICAL EFF ELEVATION **SCHEDULE** SEISMIC FORCE-RESISTING SYSTEM ELECTRICAL ELEC **EMBEDMENT** SIM SIMILAR **EMBED EDGE NAILING** NOW LOAD E.N. S.M.S. ET METAL SCREW EDGE OF DECK EOD EOS EDGE OF SLAB EQ EQUIP SPECS EQUAL FICATION(S) EQUIPMENT SQ ETC ETCETERA STIFF EW EXP EACH WAY STL SYM **EXPANSION** MMETRICAL EXT EXTERIOR **ESSIVE STRENGTH** CONCRETE CO f'c PRE-TENSIONED BOLT FDN FOUNDATION F.N. FIELD NAILING BEAM FLANGE THICKN FOOT FOOTING THICK FTG YIELD STRESS TRANSVERSE GAGE OR GAUGE **TYPICAL UNLESS OTHERWISE NOTED** VERTICAL GLUI AM BEAN **VERIFY IN FIELD** HORIZON WITH **HEADED STUD ANCHOR** WORK POINT





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Architect: Miller Architecture

Frame

Steel Moment Frame

Steel Braced Frame

Masonry Wall Frame

CA Registration:

Diaphragm

Congrete

Steel Deck

Mond W

Other:

Land Use Services Department

**Building & Safety Division** 

Structural Observation Program

And Designation Of The

Structural Observer

Job Address: 8313 VALLEY BLVD, BLOOMINGTON, CA 92313 Fermil No. ACCRNR-2024-00011

Structural Observation

(Only checked items are required)

Irm of Individual to be responsible for Structural Observation

Concrete

Masonry.

Wood

Other:

Cold-form framing

I, the Owner of the project, declare that the above listed firm or individual is hired by me to be the

DECLARATION BY ARCHITECT OR ENGINEER OF RECORD (required if the Structural Observer is

I the Architect or Engineer of Record for the Project, declare that the above listed firm or individual is

SAN BERNARDINO

Description of Work

Engineer: IMEG Corp.

Foundation

Footing, Stem Walls,

Stepping, Retaining

Foundation, Hillside

Special Anchors

**DECLARATION BY OWNER** 

different from Architect or Engineer of Record).

designated by me to be responsible for the Structural Observation.

Mat Foundation

Z Calsson, Piles

Grade Beams

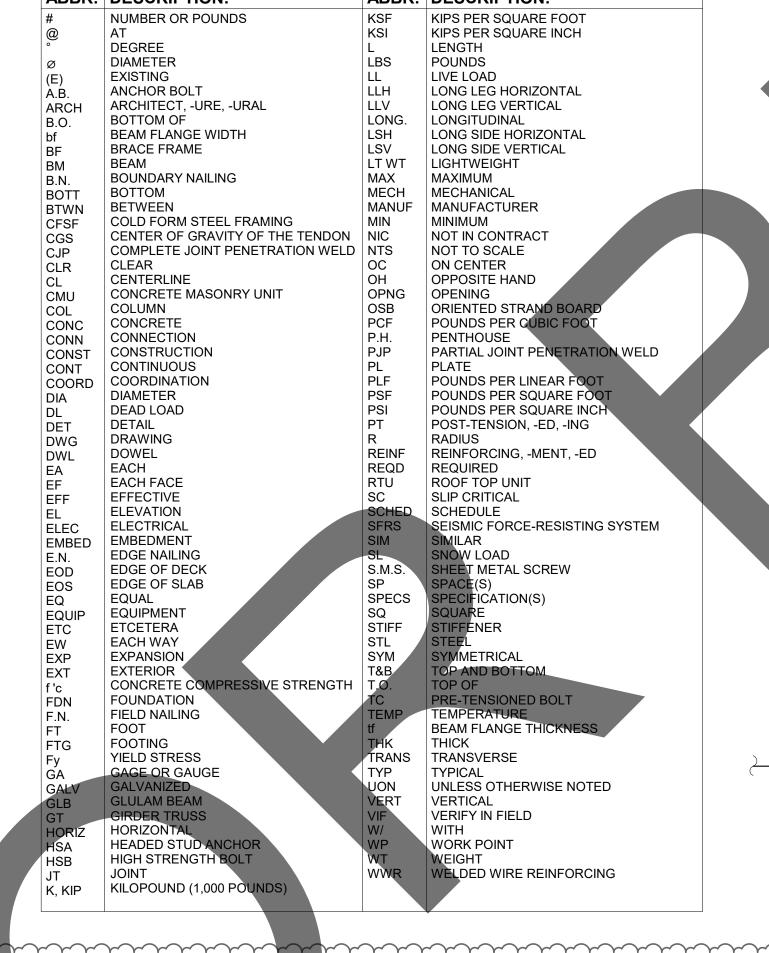
Piera

Other:

Structural Observer

REV 05/18





www.SBCounty.gov

#### INDICATES COMPLETE PENETRATION WELDED MOMENT CONNECTION, SEE TYPICAL DETAILS. TYPICAL FRAME BRACE DETAIL (FRAME PERPENDICULAR TO BEAM) SEE 3/S-502 INDICATES BEARING WALL ABOVE INDICATES BEARING WALL BELOW. SEE 1/S-700 AND 1/S-702 FOR WALL STUDS AND WALL HEADERS UNO \sw#\ (#' - #") DENOTES METAL STUD WALL WITH SINGLE SIDED PLYWOOD \_\_\_\_ SHEATHING WITH LENGTH, PER SCHEDULE 2/S-706 SW# (#' - #") DENOTES METAL STUD WALL WITH DOUBLE SIDED PLYWOOD \_\_\_\_ SHEATHING WITH LENGTH, PER SCHEDULE 2/S-706 **\_\_\_\_** DENOTES METAL STUD SHEAR WALL BELOW `-----<u>-</u> DENOTES QUANTITY WHERE MORE THAN ONE DRAGS IS REQUIRED - DENOTES DRAG MARK PER SCHEDULE ON (#) (#) XX'-XX"

CONCRETE/FOUNDATION LEGEND

" THICK CONCRETE SLAB

W/ #X@X"OC EW T&B

STEEL LEGEND

INDICATES TO SLAB ELEVATION.

REFERENCE SEE FOUNDATION PLANS

INDICATES STEP IN SLAB/FRAMING

INDICATES CHANGE IN SLOPE

SCHEDULE ON PLAN.

SEE DETAIL 2/S-504

INDICATES CONCRETE SLAB AND SPAN DIRECTION, FOR

INDICATES CONCRETE WALL ON CONTINUOUS FOOTING.

INDICATES START OF STEEL HSS COLUMN AT FLOOR

PLAN LEVEL. FOR COLUMN SIZE, REFER TO COLUMN

INDICATES TYPICAL BOLTED COLLECTOR BEAM CONNECTION.

INDICATES TOP OF STEEL ELEVATION

NDICATES FOOTING MARK. SEE SCHEDULE ON SHEET 1/S-303



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- DENOTES REQUIRED DRAG LENGTH

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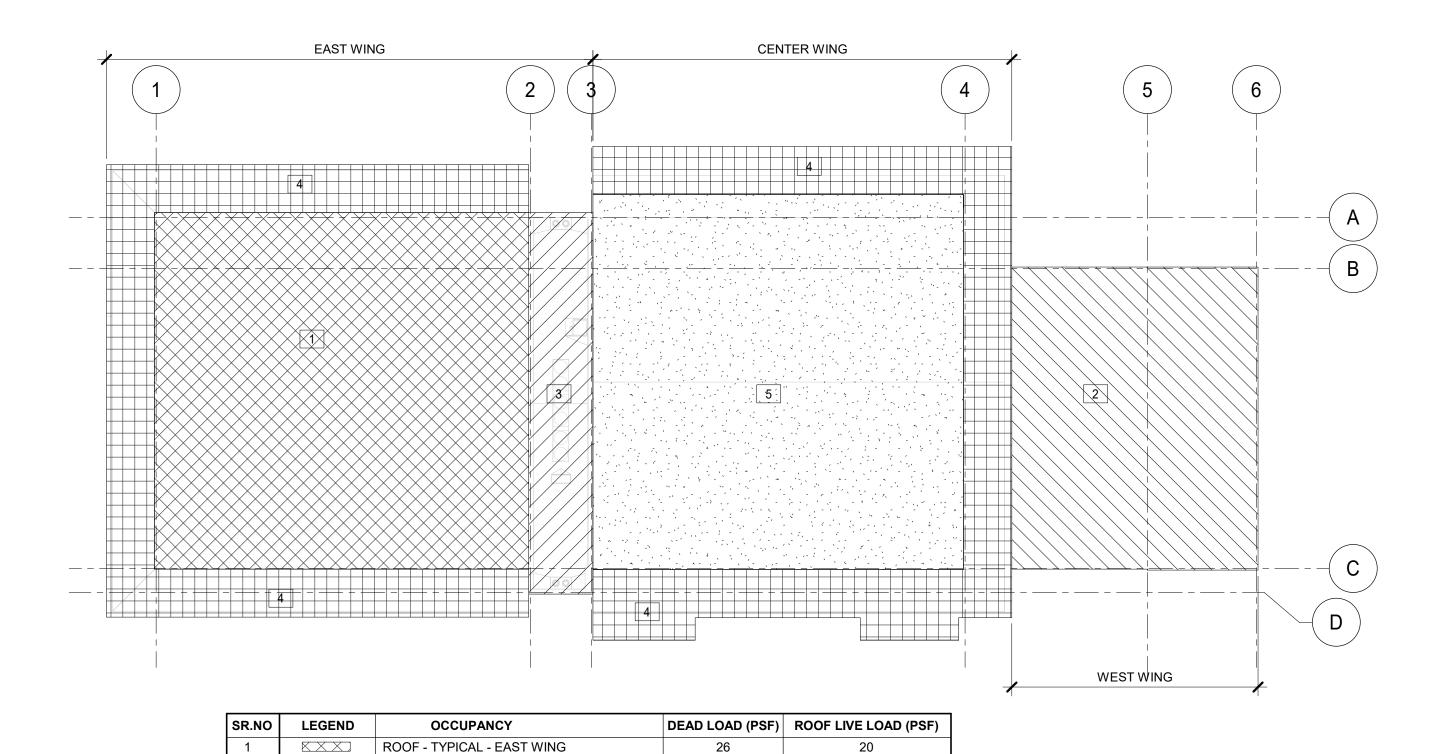
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**GENERAL NOTES** 

No. 4588 <sup>2</sup> Exp. 06-30-25

**SHEET NUMBER** 

Of Sheet



ROOF - TYPICAL - WEST WING

ROOF - TYPICAL - CENTER WING

ADMIN BUILDING - ROOF LEVEL LOADING DIAGRAM

**ROOF - MECHANICAL ROOF - OVERHANG** 

- DEAD LOAD & ROOF LIVE LOAD

OCCUPANCY DEAD LOAD (PSF) | FLOOR LIVE LOAD (PSF) FLOOR - TYPICAL

#### ADMIN BUILDING - FIRST FLOOR LOADING DIAGRAM - DEAD LOAD & FLOOR LIVE LOAD

# WIND LOADING CRITERIA - COMPONENTS & CLADDING: VULT = 120 MPH 1.0 BASIC WIND SPEED IMPORTANCE FACTOR INTERNAL PRESSURE COEFFICIENT TOPOGRAPHIC FACTOR Kzt =

B = BUILDING WIDTH
L = BUILDING LENGTH
h = BUILDING HEIGHT
Z = HEIGHT ABOVE GROUND

- 1. WIND PRESSURES GIVEN ARE BASED ON ASSUMED TRIBUTARY AREA AS INDICATED ABOVE. CURTAIN WALL DESIGNER RESPONSIBLE FOR DETERMINING ACTUAL WIND PRESSURE IN ACTUAL EFFECTIVE AREA OF
- 2. PRESSURES SHOWN ARE APPLIED NORMAL TO SURFACE. PLUS SIGNS SIGNIFY PRESSURES ACTING TOWARD THE SURFACE. NEGATIVE SIGNS SIGNIFY PRESSURES AWAY FROM SURFACE.
- 3. WIND PRESSURES SHOWN ARE STRENGTH LEVEL

	F	ROOF PR		E - (PSF) I CE AREA		SF	ROOF PRESSURE - (PSF) FOR 20 SF SURFACE AREA					
Z (ft)	Zoi	Zone 1 Zone 2		Zoi	Zone 3		Zone 1		Zone 2		Zone 3	
	Positive	Negative	Positive	Negative	Positive	Negative	Positive	Negative	Positive	Negative	Positive	Negativ
0-60	25.5	-67.5	25.5	-85.5	25.5	-112.5	24.6	-63.8	24.6	-80.7	24.6	-103.0
	` ,					SURE - (PSF) FOR WALL PRESSURE - (PSF) FOR FACE AREA 50 SF SURFACE AREA				,		
Z (ft)		Zone 4	Zo	ne 5	Zoi	ne 4	Zone 5 Zone 4		ne 4	Zone 5		
	Positive	Negative	Positive	Negative	Positive	Negative	Positive	Negative	Positive	Negative	Positive	Negativ
0-60	43.5	-46.2	43.5	-54.3	40.2	-42.9	40.2	-47.6	37.3	-40.0	37.3	-41.9
	R	OOF OV		G - (PSF) CE AREA		SF	R	OOF OVE		G - (PSF) CE AREA		SF
			1 Zone 2		Zone 3		Zone 1		Zone 2		Zone 3	

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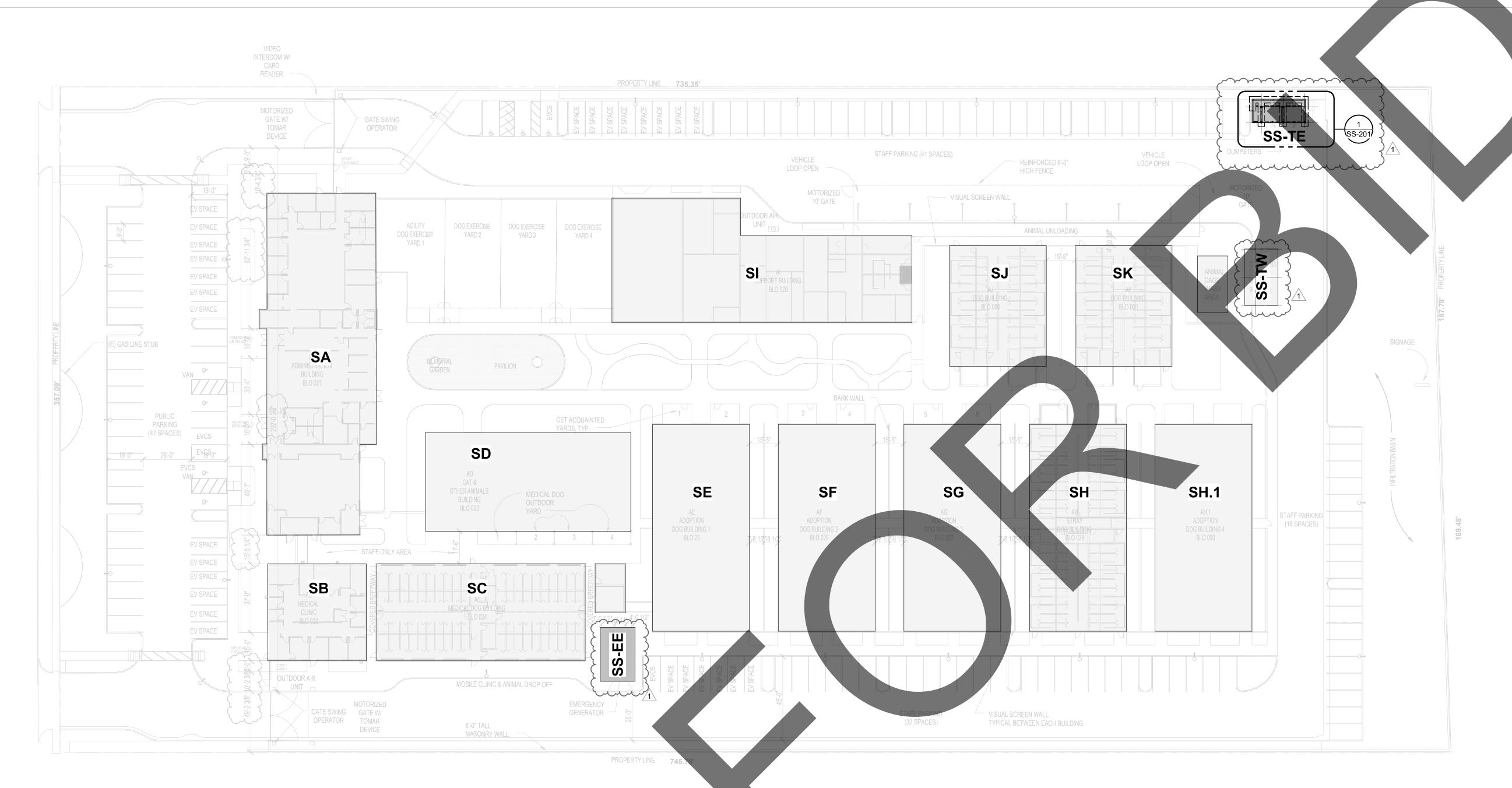
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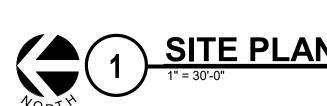
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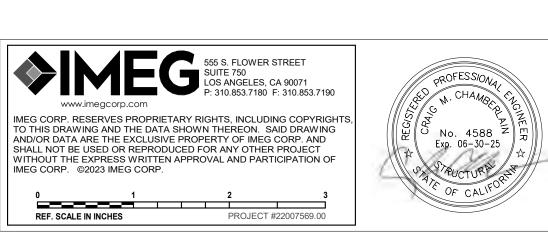
LOADING DIAGRAMS

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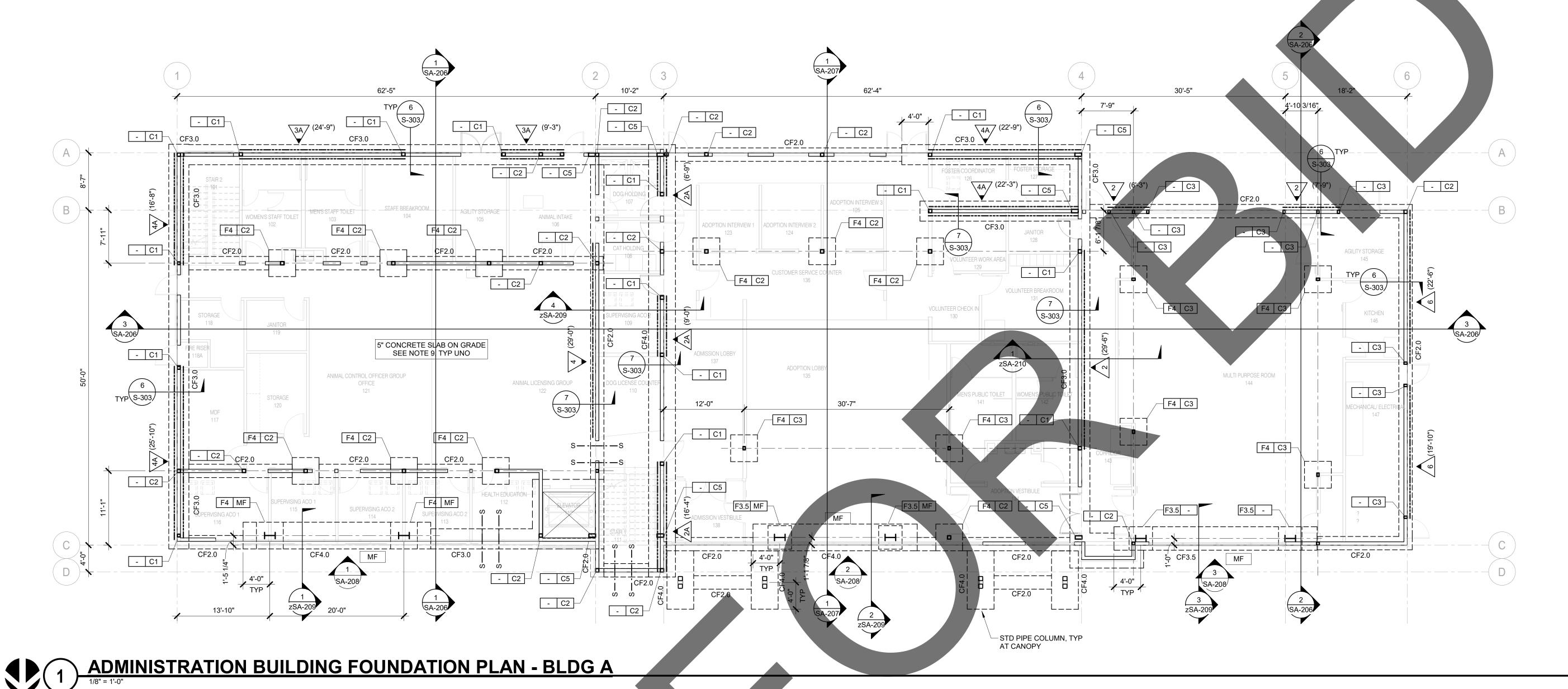
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SHEET NAME

SITE PLAN

SHEET NUMBER

SS-200



**FOUNDATION PLAN NOTES** 

1. SEE SHEET **\$-100** SERIES FOR STRUCTURAL NOTES. SEE SHEET **\$-300** SERIES FOR TYPICAL CONCRETE DETAILS. SEE SHEET **\$-500** SERIES FOR TYPICAL STEEL DETAILS. SEE SHEET **\$-700** SERIES FOR TYPICAL COLD FORM STEEL DETAILS.

2. TOP OF SLAB ON GRADE = 0'-0" UNO

3. TOP OF FOOTING SHALL BE 1'-0" BELOW TOP OF SLAB OR FINISH GRADE, UNO.

4. S.A.D. FOR DIMENSIONS, ELEVATIONS, SLOPES, CURBS, STEPS, AND PADS NOTED ON PLAN.

5. COORDINATE LOCATION OF SLAB STEPS AND DEPRESSIONS WITH ARCHITECTURAL DRAWINGS.

6. CONTRACTOR TO VERIFY ALL DIMENSIONS AND NOTIFY ARCHITECT OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION.

7. ALL FOUNDATION EXCAVATIONS MUST BE INSPECTED AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF REINFORCING STEEL.

8. PRIOR TO THE CONTRACTOR REQUESTING A BUILDING DEPARTMENT INSPECTION, THE SOILS ENGINEER SHALL ADVISE THE BUILDING OFFICIAL IN WRITING THAT:

A. THE BUILDING PAD WAS PREPARED IN ACCORDANCE WITH THE SOILS REPORT,

B. THE UTILITY TRENCHES HAVE BEEN PROPERLY BACKFILLED AND COMPACTED, AND

C. THE FOUNDATION EXCAVATIONS COMPLY WITH THE INTENT OF THE SOILS REPORT

9. TYPICAL SLAB ON GRADE: 5" THICK W/ #4 AT 18" O.C. EA WAY FOR UNDERLAYMENT SEE 4/S-301

10. ———— DENOTES CONTINUOUS FOOTING. SEE SCHEDULE 1/S-303 FOR FOOTING SIZE AND REINFORCEMENT \_ \_ \_ \_ WFX

11. S——- S DENOTES STEPPED FOOTING. SEE DETAIL 5/S-

12. CONTRACTOR SHALL COORDINATE AND LOCATE ALL DUCT, PIPE, CONDUIT, ETC PENETRATIONS THRU WALLS AND FOOTINGS AND PROVIDE THE ASSOCIAT FRAMING AND FOUNDATION CONDITIONS PER THE TYPICAL DETAILS.

DENOTES LOAD BEARING METAL STUD FRAMED WALL PER 1/S-700 AND 1/S-702

14. SW# (#' - #") DENOTES METAL STUD SHAR WALL, PER 2/S-706

#### **FOUNDATION PLAN NOTES (CONT)**

DENOTES NON-LOAD BEARING METAL STUD WALL PER 1/S-702

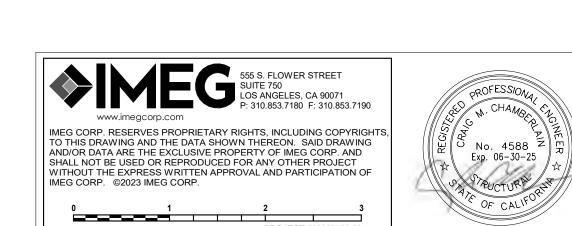
DENOTES HOLDOWN PER 5/S-706

COLUMN SCHEDULE - BUILDING A			
MARK	SIZE		
C1	HSS6X6X3/8		
C2	HSS6X6X1/4		
C3	HSS6X4X1/4		
C4	HSS4X4X3/8		
C5	HSS12X6X1/2		
7			
MF	MOMENT FRAME COLUMN PER ELEVATION		

#### NOTES:

1. REFER TO DETAILS 5/S-301 FOR ANCHOR BOLT AND BASE PLATE INFORMATION, UNO.

2. REFER TO DETAILS 2/S-304 AND 3/S-304 FOR FOOTING INFORMATION AT STEEL COLUMNS, UNO.





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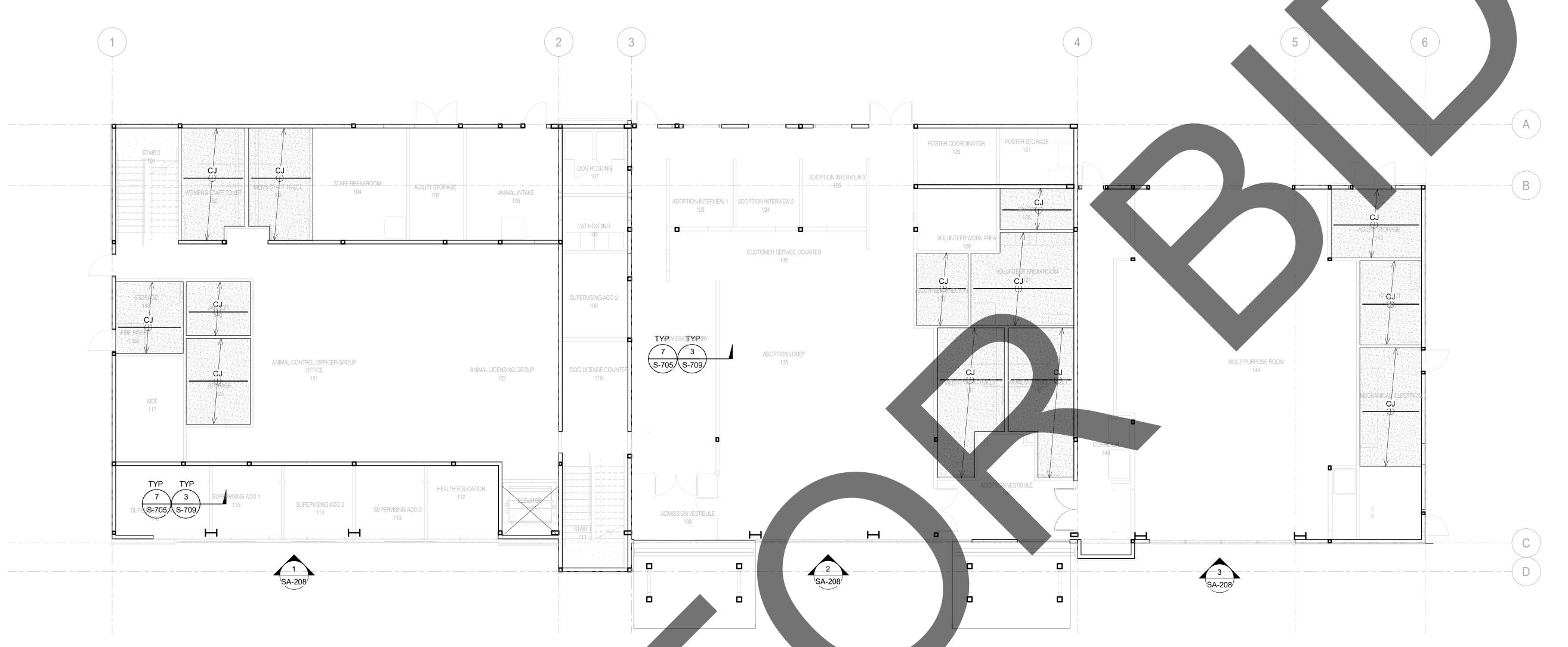
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ADMINISTRATION BUILDING **FOUNDATION PLAN** 

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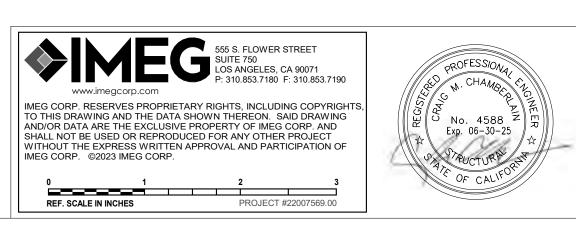
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#### **CEILING PLAN NOTES**

- 1. SEE SHEET S-100 SERIES FOR STRUCTURAL NOTES. SEE SHEET S-704 SERIES FOR TYPICAL CEILING DETAILS.
- 2. ALL DIMENSIONAL INFORMATION SHOWN IS BASED ON THE ARCHITECTURAL DRAWINGS. FOR ANY DIMENSIONAL INFORMATION NOT SHOWN REFER TO THE ARCHITECTURAL DRAWINGS.
- 3. MECHANICAL ACCESS W/3/4" PLYWOOD OVER CHEILING JOISTS SEE ARCHITECTURAL DRAWINGS FOR EXTENT





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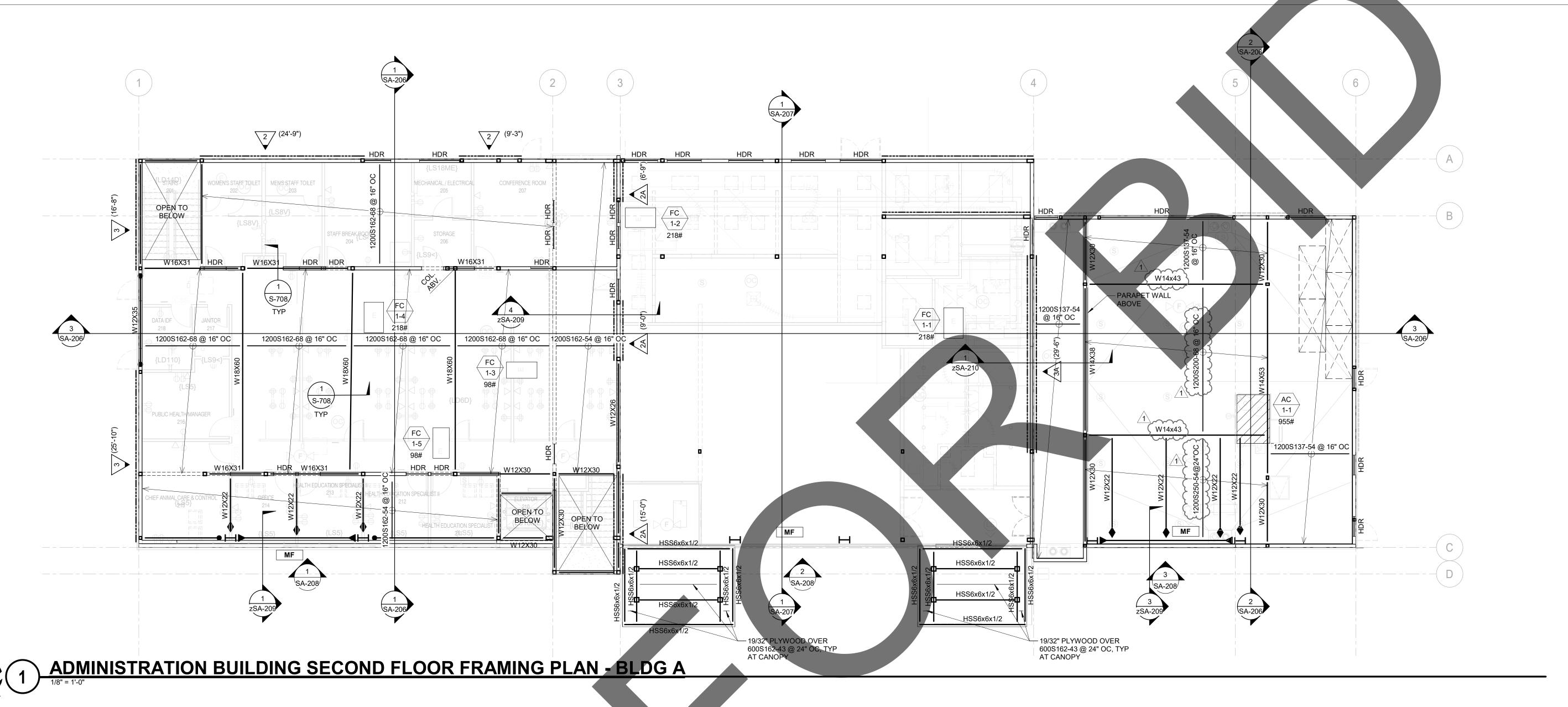
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ADMINISTRATION BUILDING SECOND FLOOR **CEILING PLAN** 

SHEET NUMBER



#### **ROOF PLAN NOTES**

1. SEE SHEET **S-XXX** SERIES FOR STRUCTURAL NOTES. SEE SHEET **S-XXX** SERIES FOR TYPICAL DETAILS.

2. ALL DIMENSIONAL INFORMATION SHOWN IS BASED ON THE ARCHITECTURAL DRAWINGS. FOR ANY DIMENSIONAL INFORMATION NOT SHOWN REFER TO THE ARCHITECTURAL DRAWINGS.

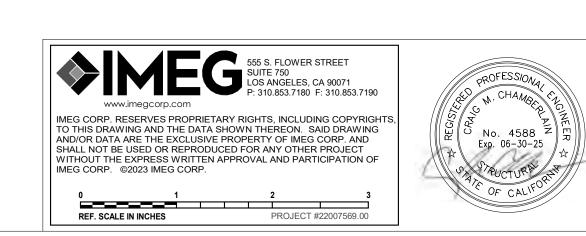
3. BEAM TO BE EQUALLY SPACED BETWEEN SUPPORTS UNO.

4. FOR COLUMN SIZES, SEE FOUNDATION PLAN.

— DENOTES MECHANICAL UNIT PER MEP DWGS

XXX# — DENOTES UNIT MAX OPER WT
W/ CURB AND ALL ATTACHMENTS INCLUDED

- DENOTES METAL STUD SHEAR WALL BELOW





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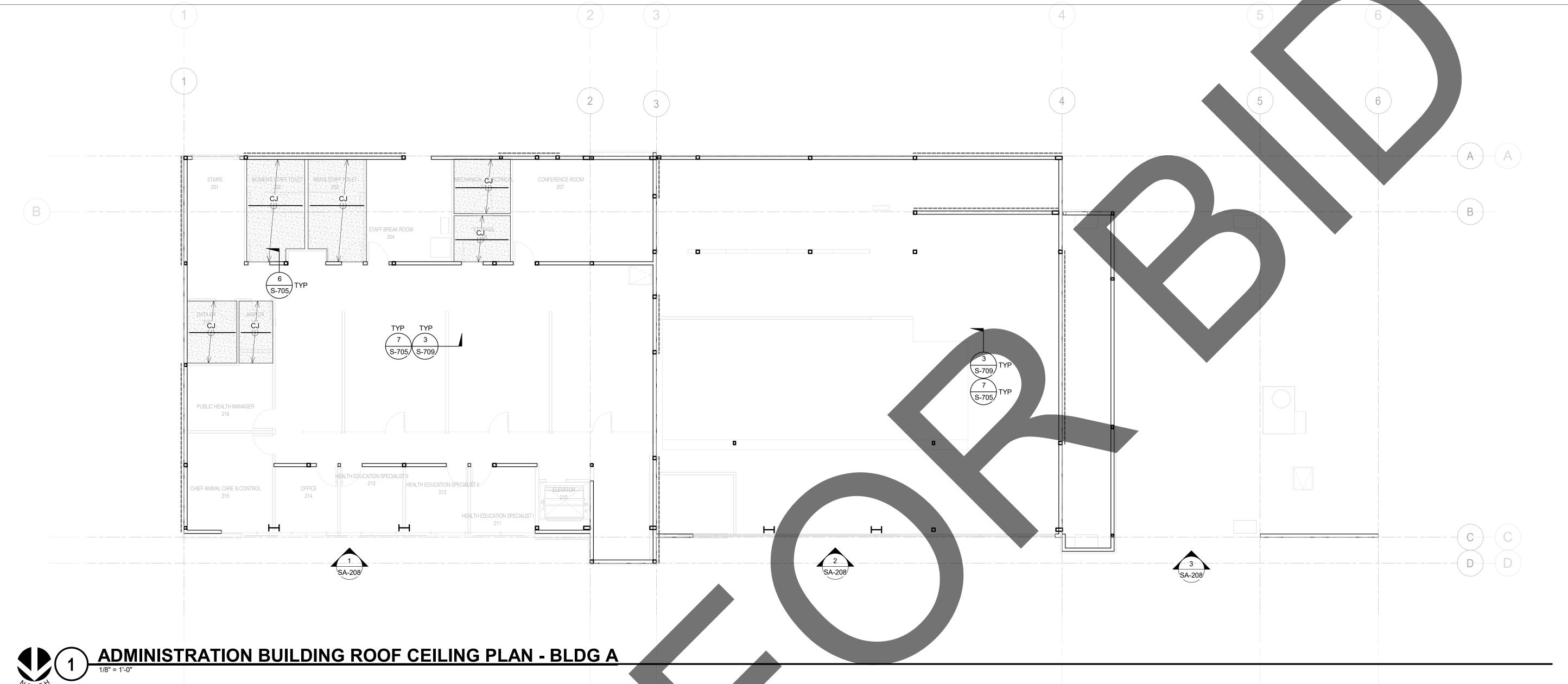
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ADMINISTRATION BUILDING SECOND FLOOR FRAMING PLAN

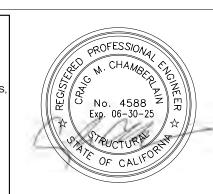
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#### **CEILING PLAN NOTES**

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- 3. MECHANICAL ACCESS W/3/4" PLYWOOD OVER CHEILING JOISTS SEE ARCHITECTURAL DRAWINGS FOR EXTENT







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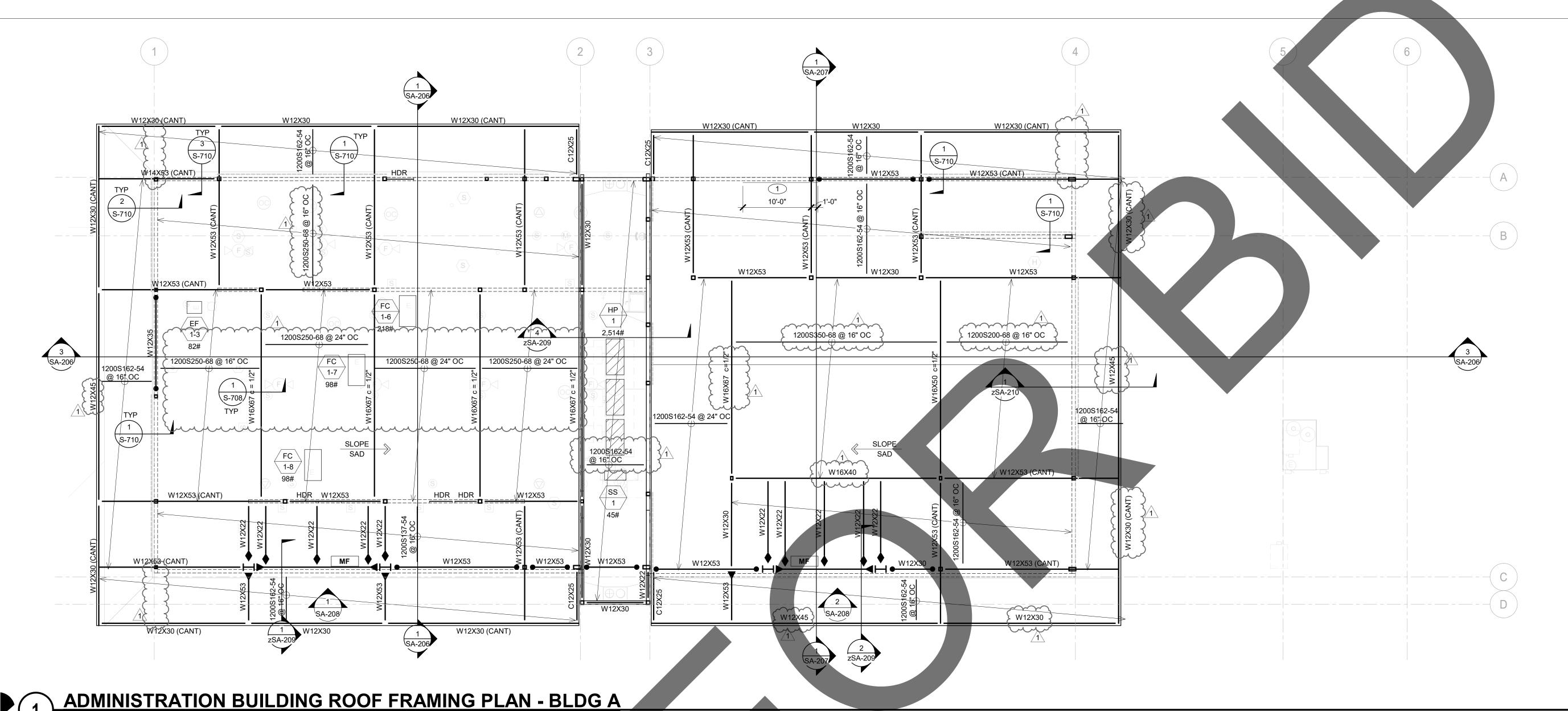
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ADMINISTRATION **BUILDING ROOF CEILING PLAN** 

SHEET NUMBER



#### **ROOF PLAN NOTES**

1. SEE SHEET **S-XXX** SERIES FOR STRUCTURAL NOTES. SEE SHEET **S-XXX** SERIES FOR TYPICAL DETAILS.

2. ALL DIMENSIONAL INFORMATION SHOWN IS BASED ON THE ARCHITECTURAL DRAWINGS. FOR ANY DIMENSIONAL INFORMATION NOT SHOWN REFER TO THE ARCHITECTURAL DRAWINGS.

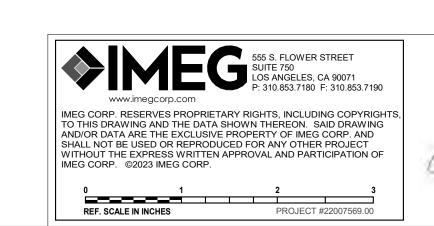
3. BEAM TO BE EQUALLY SPACED BETWEEN SUPPORTS UNO.

4. FOR COLUMN SIZES, SEE FOUNDATION PLAN.

— DENOTES MECHANICAL UNIT PER MEP DWGS

XXX# — DENOTES UNIT MAX OPER WT
W/ CURB AND ALL ATTACHMENTS INCLUDED

- DENOTES METAL STUD SHEAR WALL BELOW





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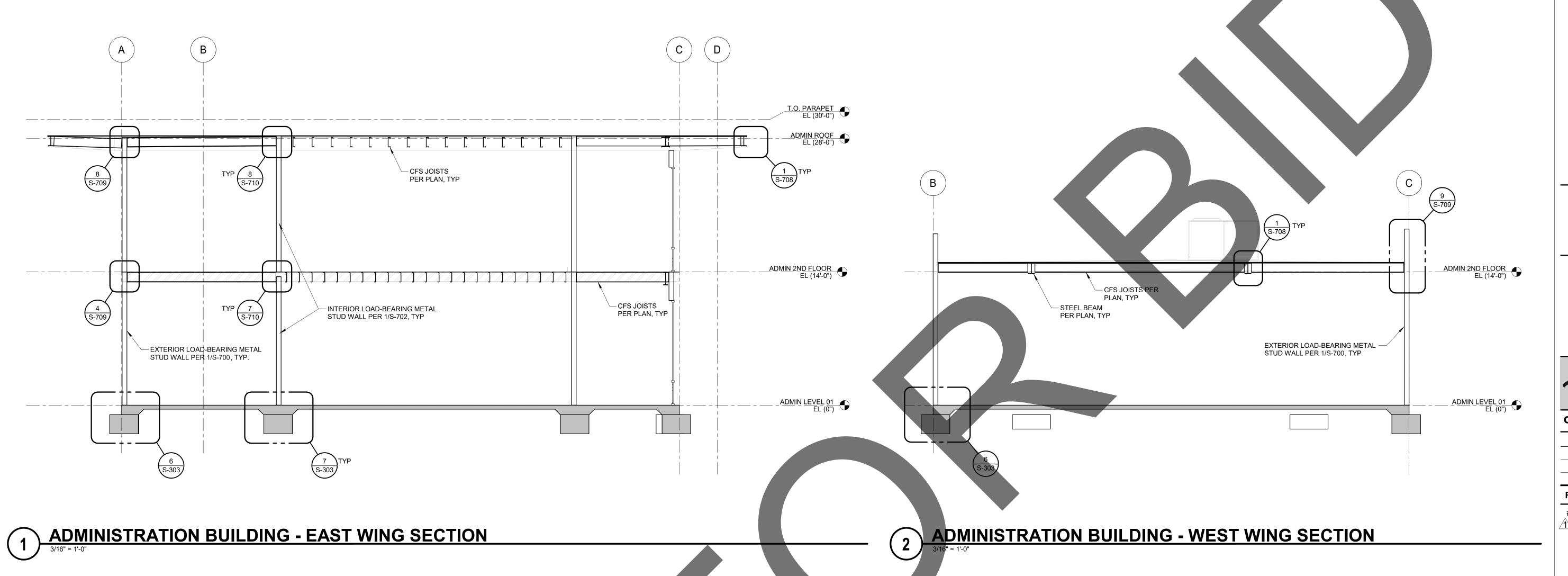
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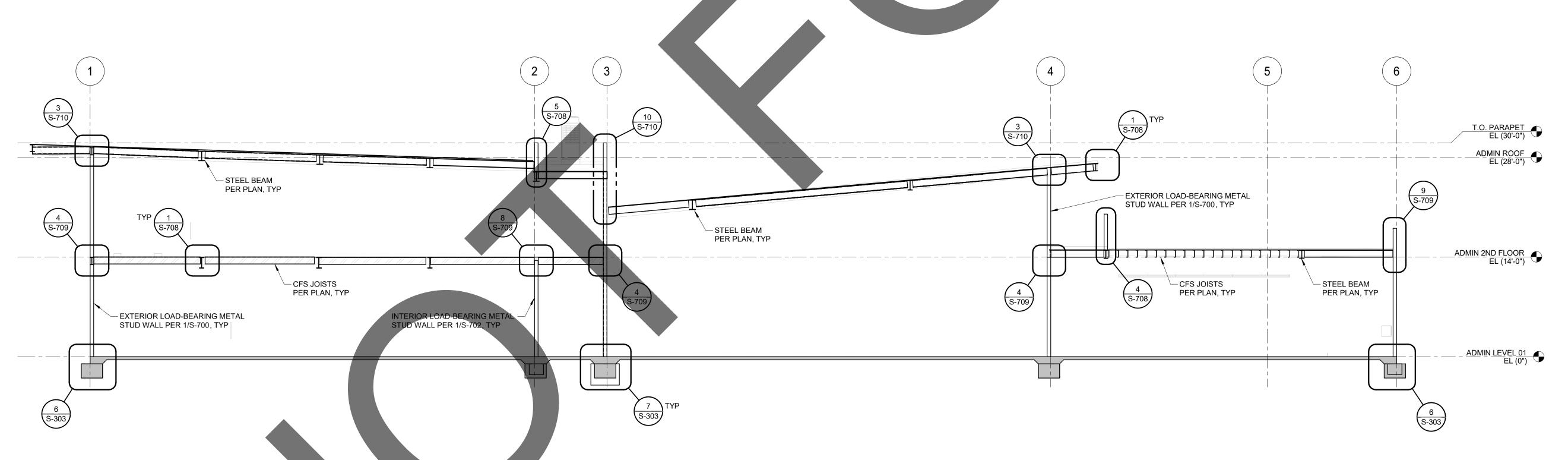
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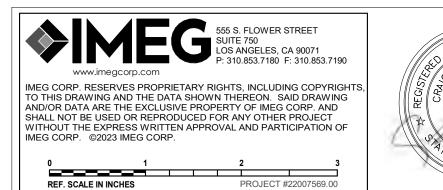


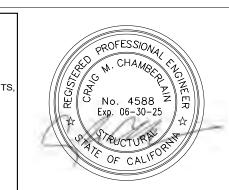




ADMINISTRATION BUILDING - EAST WING SECTION

1/8" = 1'-0"







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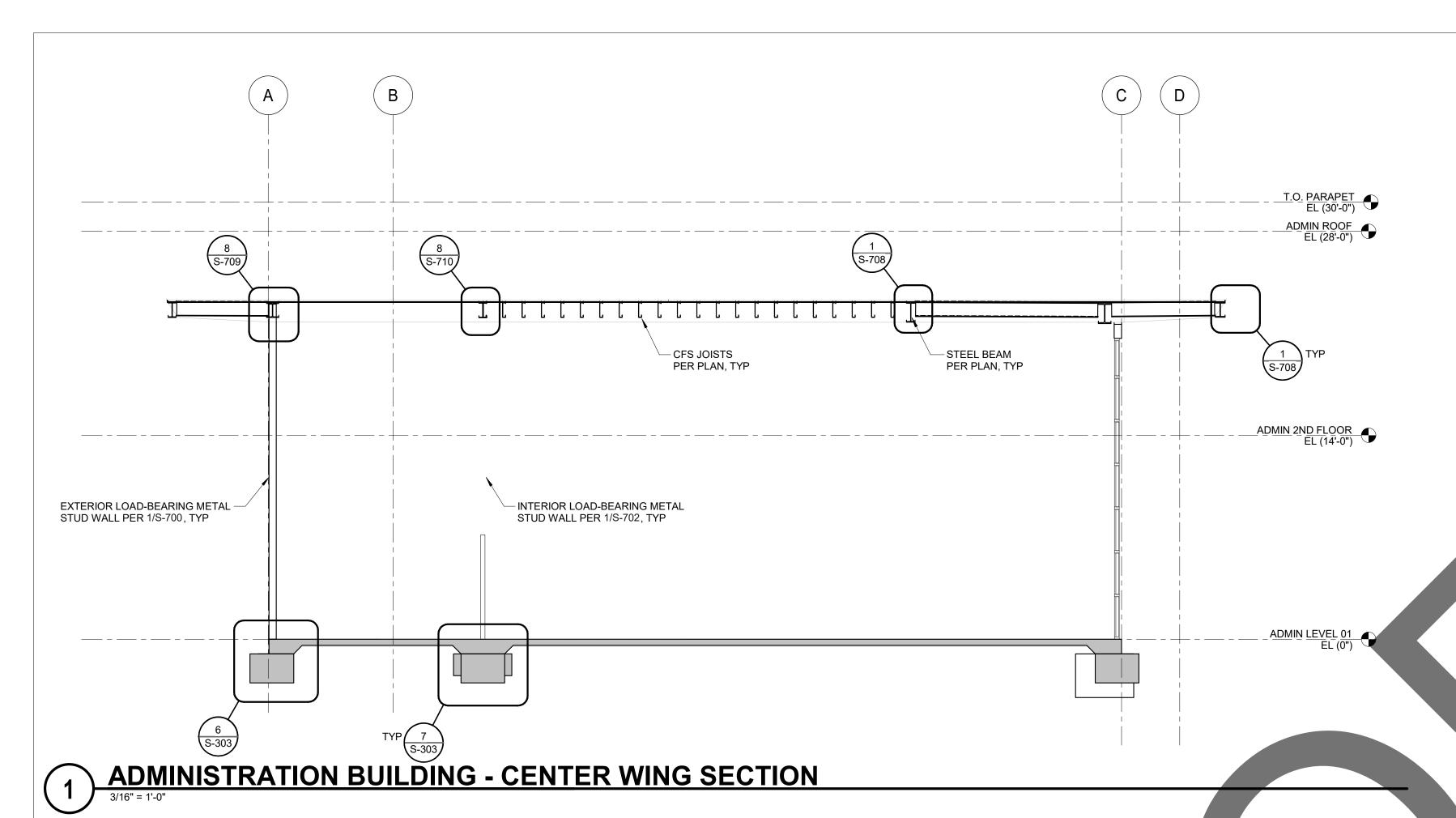
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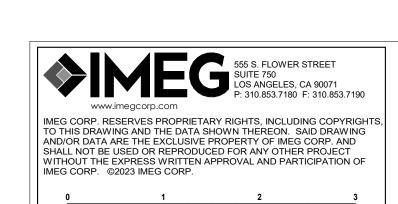
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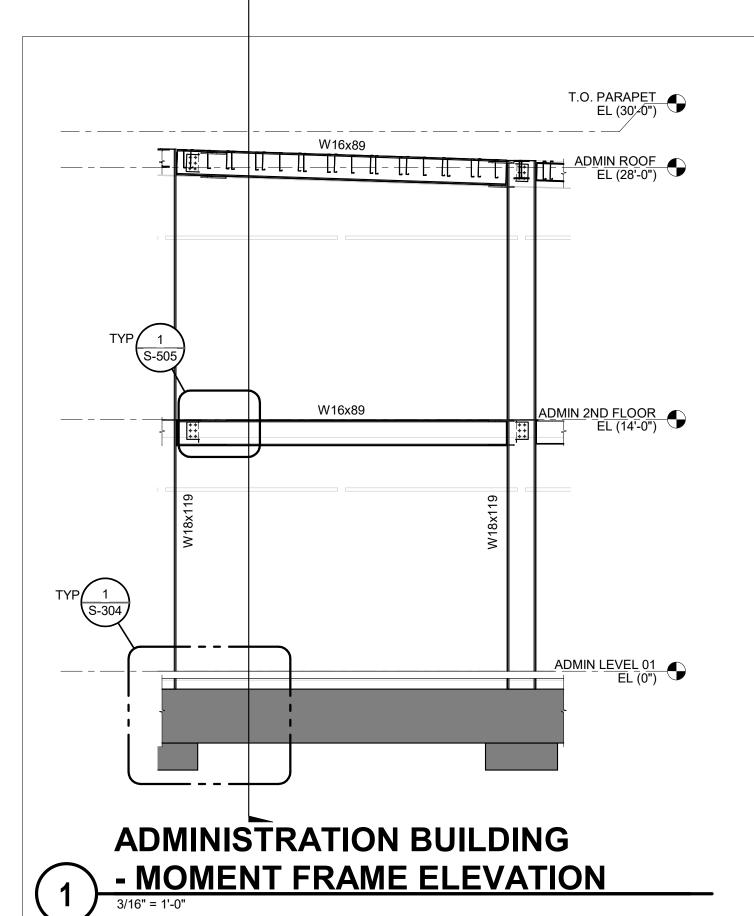
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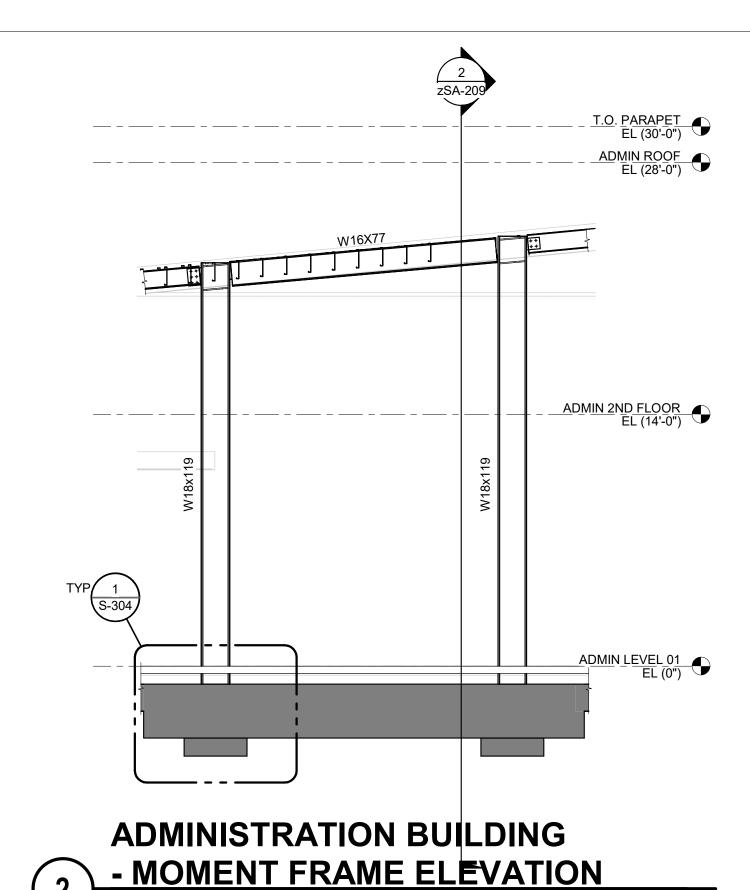
SHEET NUMBER

SA-207

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ADMIN LEVEL 0 

ADMINISTRATION BUILDING - MOMENT FRAME ELEVATION



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initials	date	phase

**REVISIONS/ADDENDA** 

1 06/12/24 PLAN CHECK RESUBMITTAL

PROJECT INFORMATION

Project Number: Drawn By:

CENTER

CARE

Checked By: 06/12/2024 Issue Date:

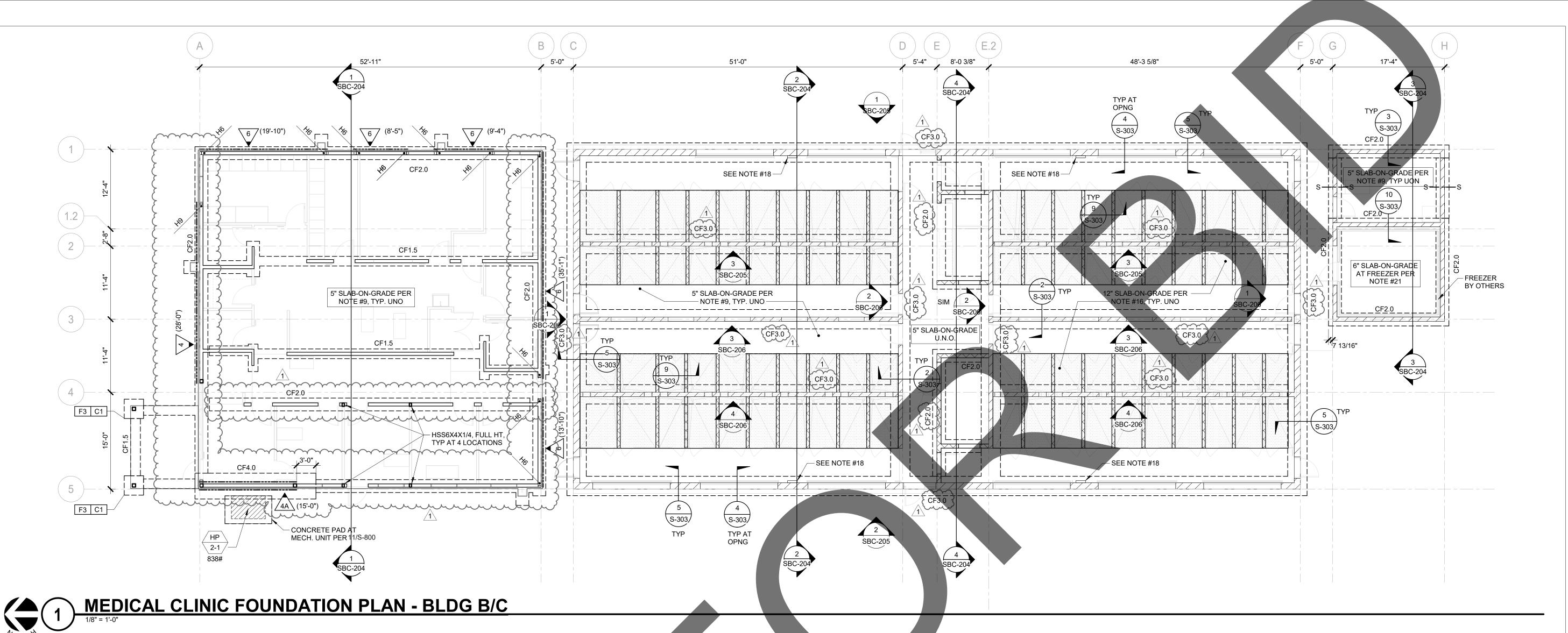
SHEET NAME

ADMINISTRATION BUILDING MOMENT FRAME **ELEVATIONS** 

SHEET NUMBER

SA-208

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#### **FOUNDATION PLAN NOTES**

- 1. SEE SHEET S-100 SERIES FOR STRUCTURAL NOTES. SEE SHEET S-300 SERIES FOR TYPICAL CONCRETE DETAILS. SEE SHEET S-400 SERIES FOR TYPICAL CMU DETAILS SEE SHEET S-500 SERIES FOR TYPICAL STEEL DETAILS. SEE SHEET S-700 SERIES FOR TYPICAL COLD FORM STEEL DETAILS.
- 2. TOP OF SLAB ON GRADE = 0'-0" UNO

WITH ARCHITECTURAL DRAWINGS.

WFX

- 3. TOP OF FOOTING SHALL BE 1'-0" BELOW TOP OF SLAB OR FINISH GRADE, TYP, UNO. TOP OF FOOTING SHALL BE 1'-4" BELOW TOP OF SLAB OR FINISH GRADE, TYP. AT EXTERIOR CMU WALLS, UNO.
- 4. S.A.D. FOR DIMENSIONS, ELEVATIONS, SLOPES, CURBS, STEPS, AND PADS NOTED ON PLAN.
- 5. COORDINATE LOCATION OF SLAB STEPS AND DEPRESSIONS

DISCREPANCIES PRIOR TO CONSTRUCTION.

- 6. CONTRACTOR TO VERIFY ALL DIMENSIONS AND NOTIFY ARCHITECT OF ANY
- 7. ALL FOUNDATION EXCAVATIONS MUST BE INSPECTED AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF REINFORCING STEEL.
- 8. PRIOR TO THE CONTRACTOR REQUESTING A BUILDING DEPARTMENT INSPECTION, THE SOILS ENGINEER SHALL ADVISE THE BUILDING OFFICIAL IN WRITING THAT: A. THE BUILDING PAD WAS PREPARED IN ACCORDANCE
- WITH THE SOILS REPORT, B. THE UTILITY TRENCHES HAVE BEEN PROPERLY
- BACKFILLED AND COMPACTED, AND C. THE FOUNDATION EXCAVATIONS COMPLY WITH THE
- INTENT OF THE SOILS REPORT
- DENOTES CONTINUOUS FOOTING. SEE SCHEDULE 1/S-303 FOR FOOTING SIZE AND REINFORCEMENT

9. TYPICAL SLAB ON GRADE: 5" THICK W/ #4 AT 18" O.C. EA WAY FOR UNDERLAYMENT

- 11. S—--S DENOTES STEPPED FOOTING. SEE DETAIL 5/S-302
- 12. CONTRACTOR SHALL COORDINATE AND LOCATE ALL DUCT, PIPE, CONDUIT, E PENETRATIONS THRU WALLS AND FOOTINGS AND PROVIDE THE ASSOCIATED

FRAMING AND FOUNDATION CONDITIONS PER THE TYPICAL DETAILS.

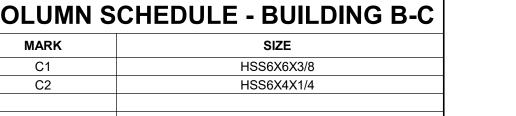
- DENOTES LOAD BEARING METAL STUD FRAMED WALL PER 1/S-700 AND 1/S-702
- DENOTED FULL HEIGHT CMU WALL
- DENOTES METAL STUD SHAR WALL PER 2/S-706
- SLAB ON GRADE: 12" THICK W/ #5 AT 12" OC EA WAY TOP AND BOTTOM - SEE X/S-XXX

#### **FOUNDATION PLAN NOTES (CONT)**

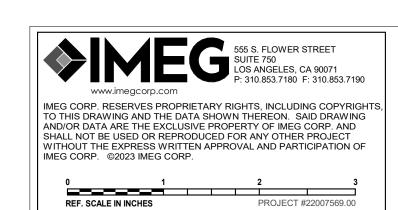
- DENOTES NON-LOAD BEARING METAL STUD WALL PER 1/S-702
- 18. FACE MOUNTED CONTROL PANEL AND PIPING SEE PLUMBING DRAWINGS.
- 19. DENOTES HOLDOWN PER 5/S-706
- 20. DENOTES PARTIAL HEIGHT CMU WALL
- 21. SLAB ON GRADE: 6" THICK W/ #4 @ 12" OC EA. WAY FOR UNDERLAYMENT SEE/S-301

COLUMN SCHEDULE - BUILDING B-C		
MARK	SIZE	
C1	HSS6X6X3/8	
C2	HSS6X4X1/4	

- 1. REFER TO DETAILS 5/S-301 FOR ANCHOR BOLT AND BASE PLATE
- INFORMATION, UNO.



2. REFER TO DETAILS 2/S-304 AND 3/S-304 FOR FOOTING INFORMATION AT STEEL COLUMNS, UNO.





22007569.00 Project Number: Drawn By: Checked By:

RNARDIN

06/12/2024

architecture

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Redlands, CA 92374

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Fax: 909-335-7299 info@miller-aip.com

interiors

planning

owner approval

**REVISIONS/ADDENDA** 

1 06/12/24 PLAN CHECK RESUBMITTAL

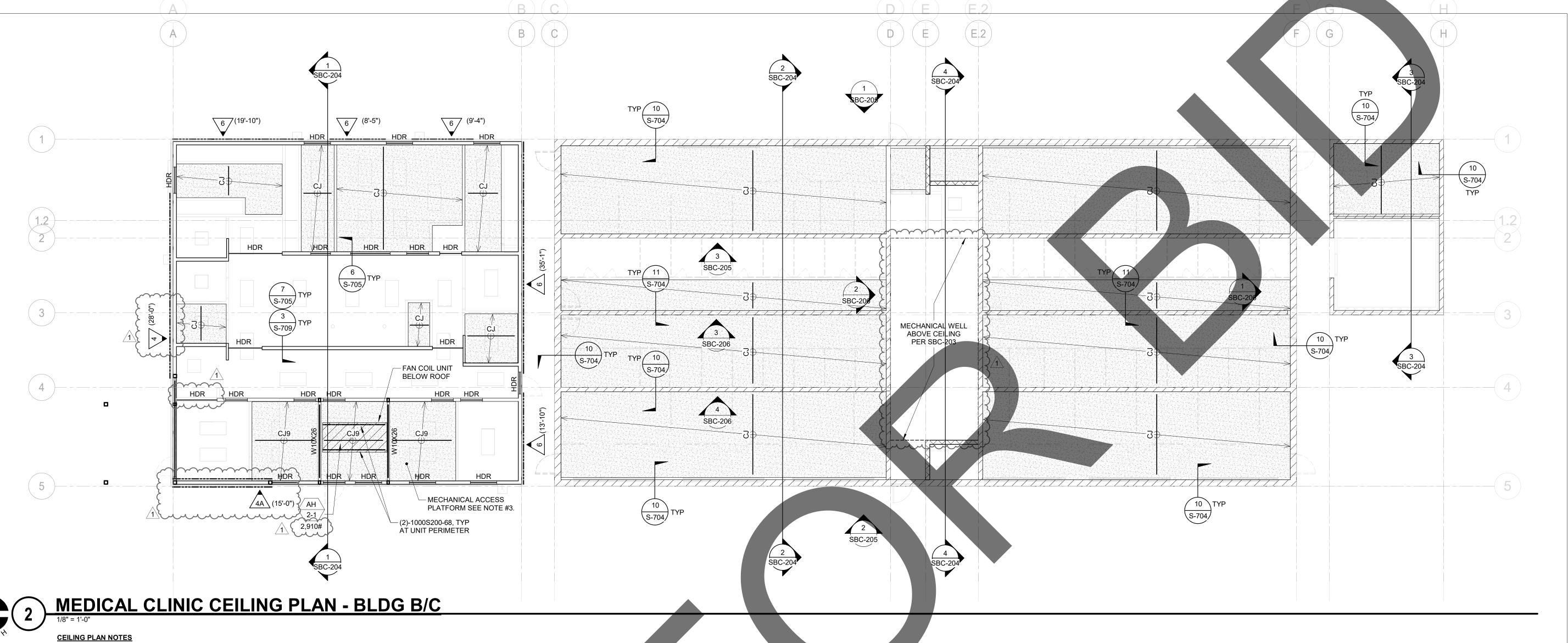
SHEET NAME

Issue Date:

ARE

MEDICAL CLINIC FOUNDATION PLAN

SHEET NUMBER



1. SEE SHEET S-100 SERIES FOR STRUCTURAL NOTES. SEE SHEET S-704

SERIES FOR TYPICAL CEILING DETAILS.

SHOWN REFER TO THE ARCHITECTURAL DRAWINGS.

ALL DIMENSIONAL INFORMATION SHOWN IS BASED ON THE ARCHITECTURAL DRAWINGS. FOR ANY DIMENSIONAL INFORMATION NOT

3. MECHANICAL ACCESS W/3/4" PLYWOOD OVER CHEILING JOISTS - SEE ARCHITECTURAL DRAWINGS FOR EXTENT

owner approval

architecture

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planning

**REVISIONS/ADDENDA** 

1 06/12/24 PLAN CHECK RESUBMITTAL

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06/12/2024

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PROJECT INFORMATION

Project Number:

Drawn By: Checked By:

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SHEET NAME

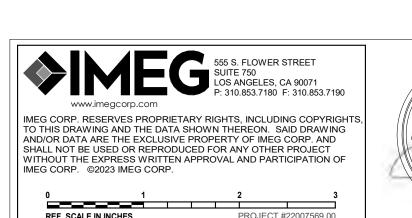
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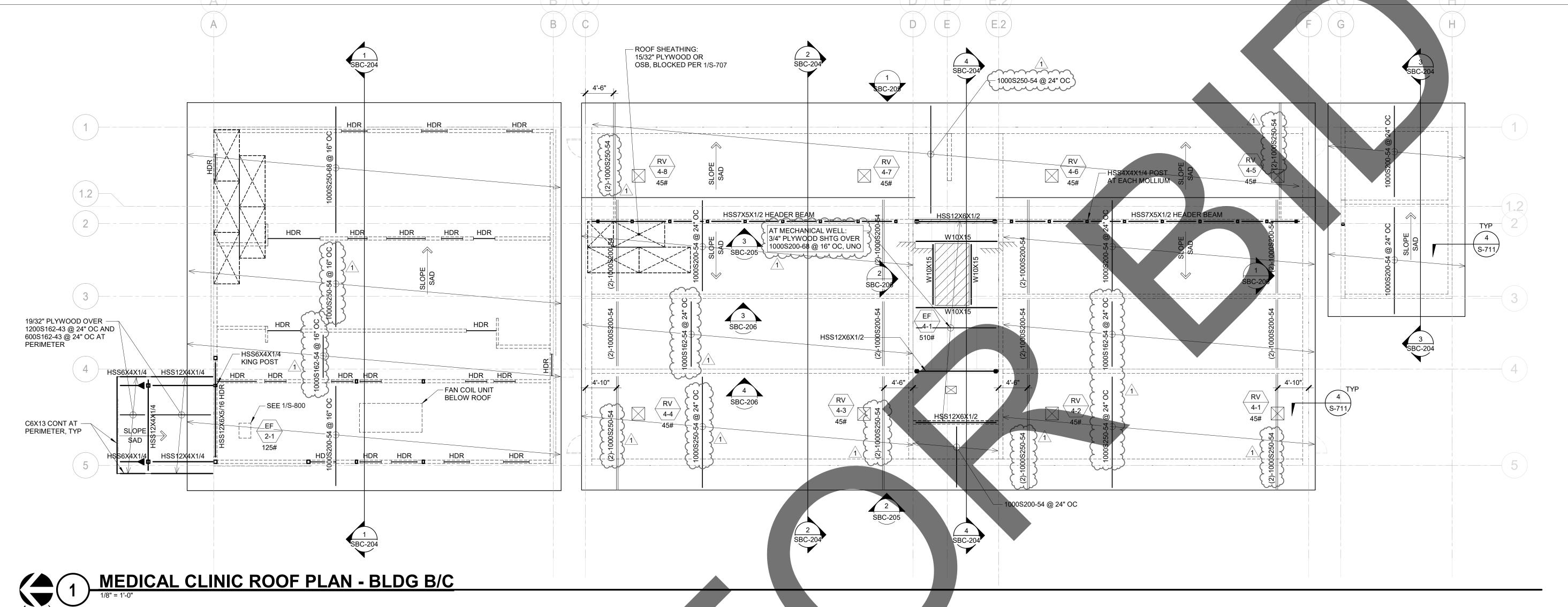
MEDICAL CLINIC **CEILING PLAN** 

SHEET NUMBER

SBC-202

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#### **ROOF PLAN NOTES**

- 1. SEE SHEET **S-100** SERIES FOR STRUCTURAL NOTES. SEE SHEET **S-700** SERIES FOR TYPICAL DETAILS.
- ALL DIMENSIONAL INFORMATION SHOWN IS BASED ON THE ARCHITECTURAL DRAWINGS. FOR ANY DIMENSIONAL INFORMATION NOT SHOWN REFER TO THE ARCHITECTURAL DRAWINGS.
- XX DENOTES MECHANICAL UNIT PER MEP DWGS DENOTES UNIT MAX OPER WT
   W/ CURB AND ALL ATTACHMENTS INCLUDED
- DENOTES METAL STUD SHEAR WALL BELOW
- 6. ALIGN ROOF JOISTS WITH CF WALL STUDS PER DETAIL 3/S-700





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**REVISIONS/ADDENDA** 

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ANIMAL

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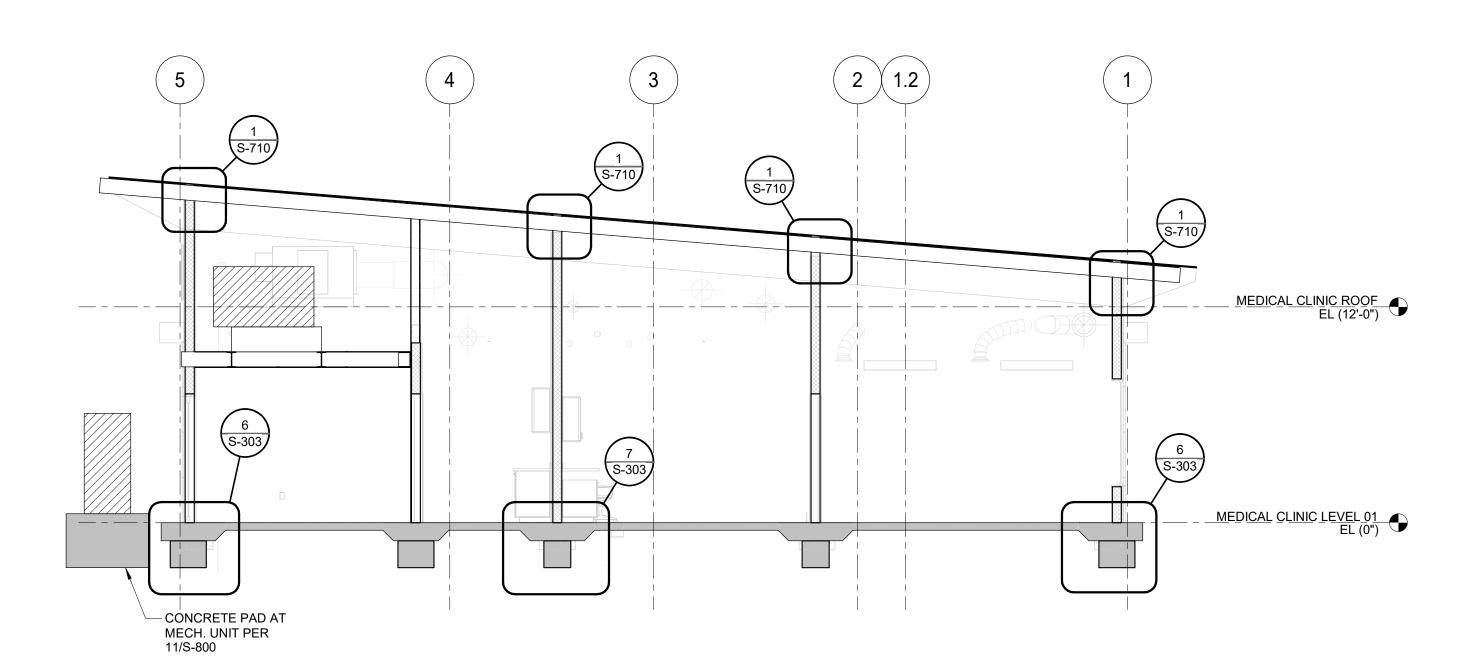
Project Number: 22007569.00 Drawn By: Checked By: Issue Date:

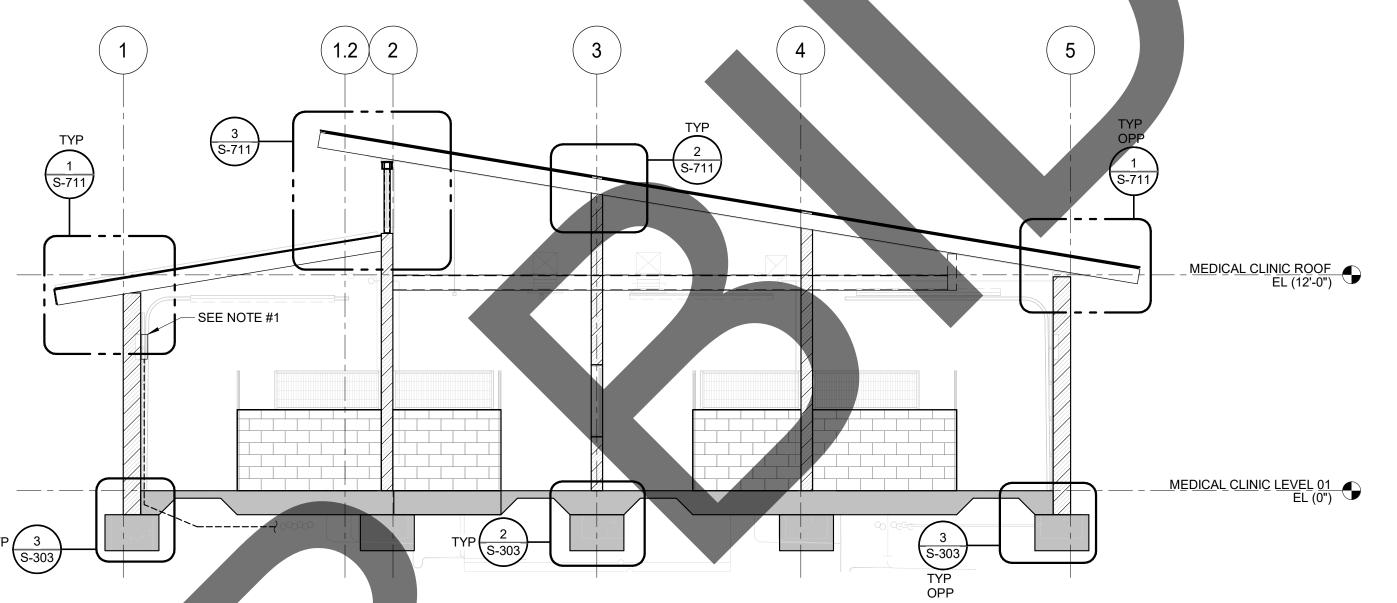
SHEET NAME

MEDICAL CLINIC **ROOF PLAN** 

06/12/2024

SHEET NUMBER



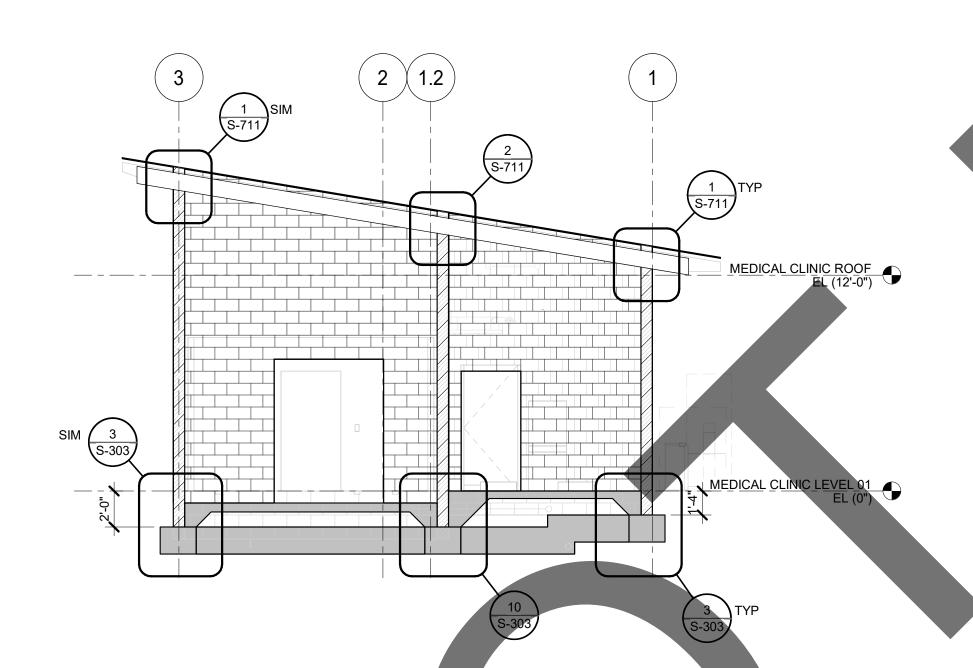


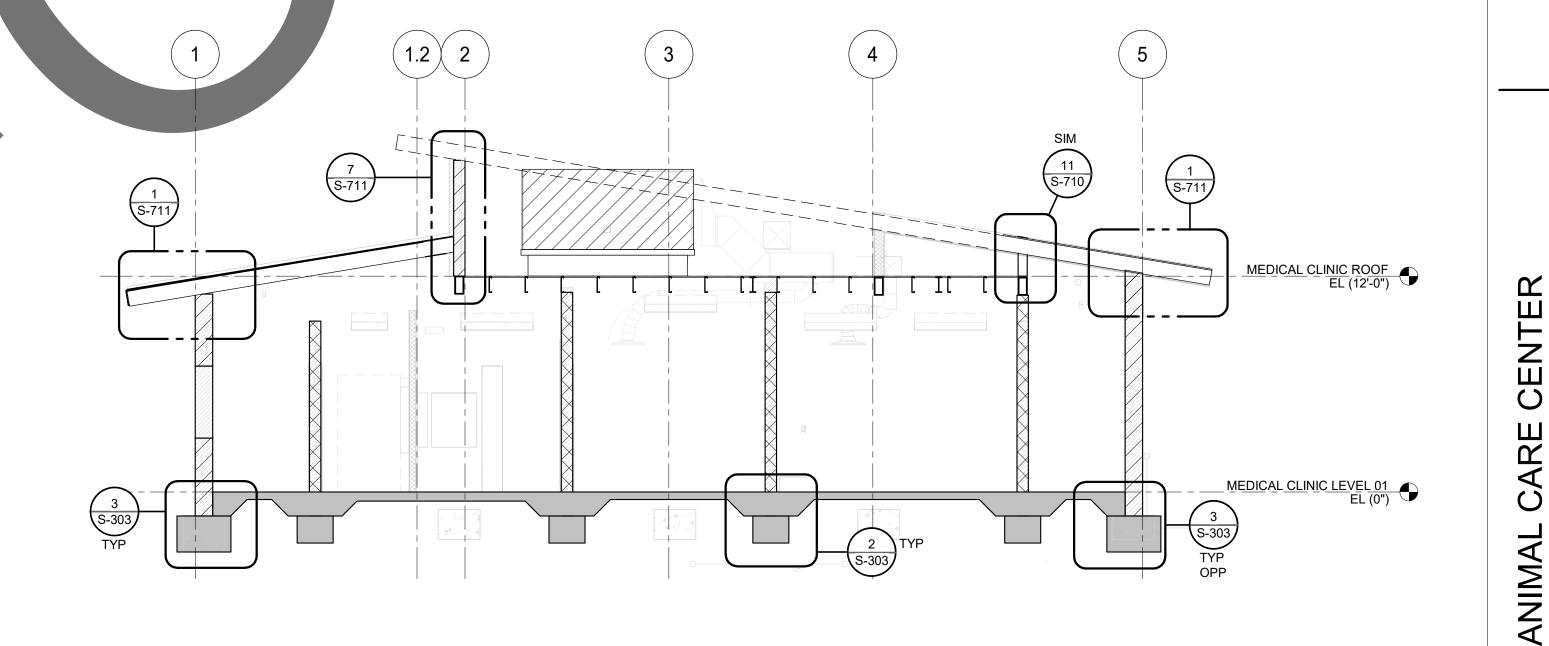
NOTES:

1. FACE MOUNTED CONTROL PANEL AND PIPING PER PLAN - SEE PLUMBING DRAWINGS FOR LOCATION.

## MEDICAL CLINIC BUILDING SECTION 3/16" = 1'-0"





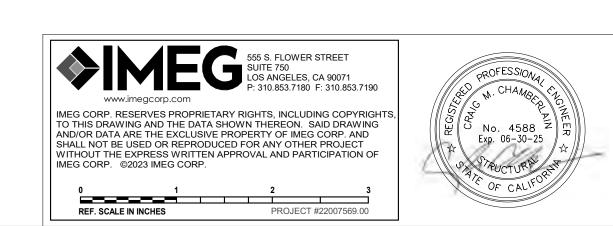


EUTHANASIA/FREEZER BUILDING SECTION

3/16" = 1'-0"

DOG CLINIC BUILDING SECTION

3/16" = 1'-0"





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**REVISIONS/ADDENDA** 

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SAN BERNARDINO COUNT

06/12/2024

PROJECT INFORMATION

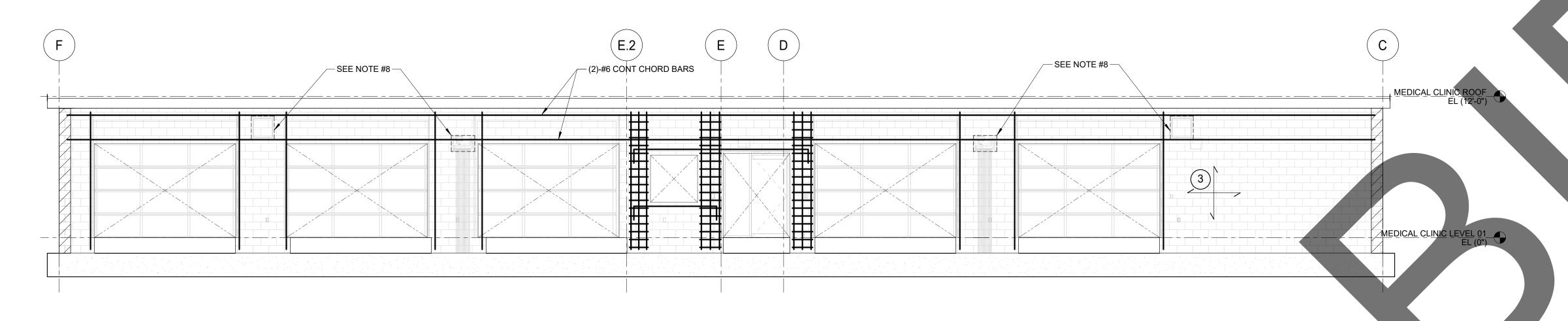
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Checked By: Issue Date:

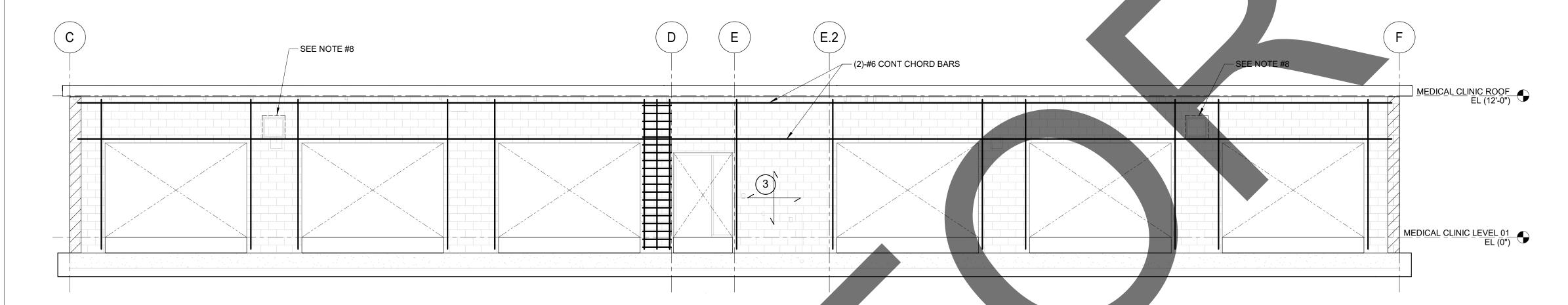
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MEDICAL CLINIC BUILDING SECTIONS

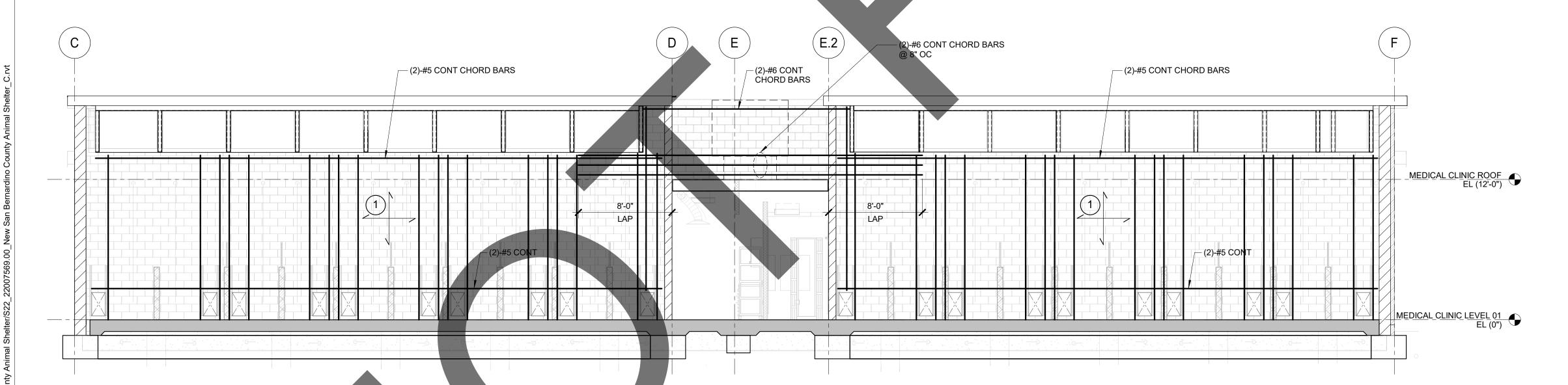
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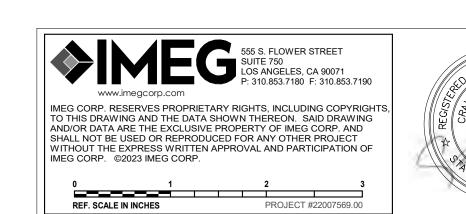
## DOG CLINIC EXTERIOR WALL ELEVATION - GL "1" - 12" THICK CMU

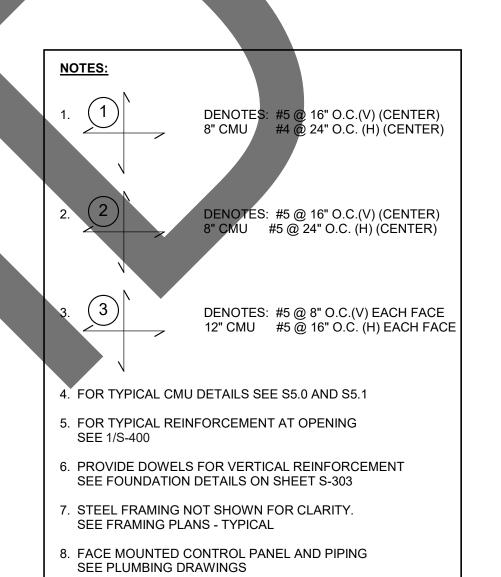


## DOG CLINIC EXTERIOR WALL ELEVATION - GL "5" - 12" THICK CMU



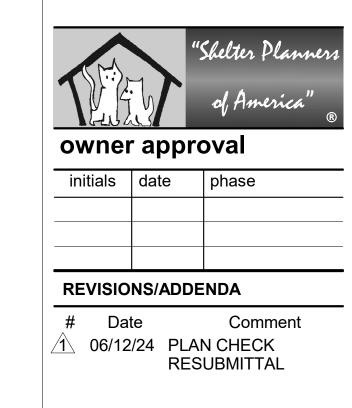
OOG CLINIC EXTERIOR WALL ELEVATION - GL "2" - 8" THICK CMU
3/16" = 1'-0"

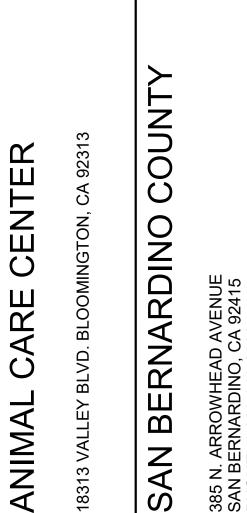






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#### PROJECT INFORMATION

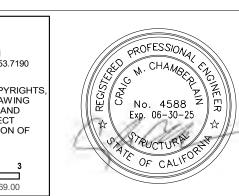
Project Number: Drawn By: Checked By: 06/12/2024 Issue Date:

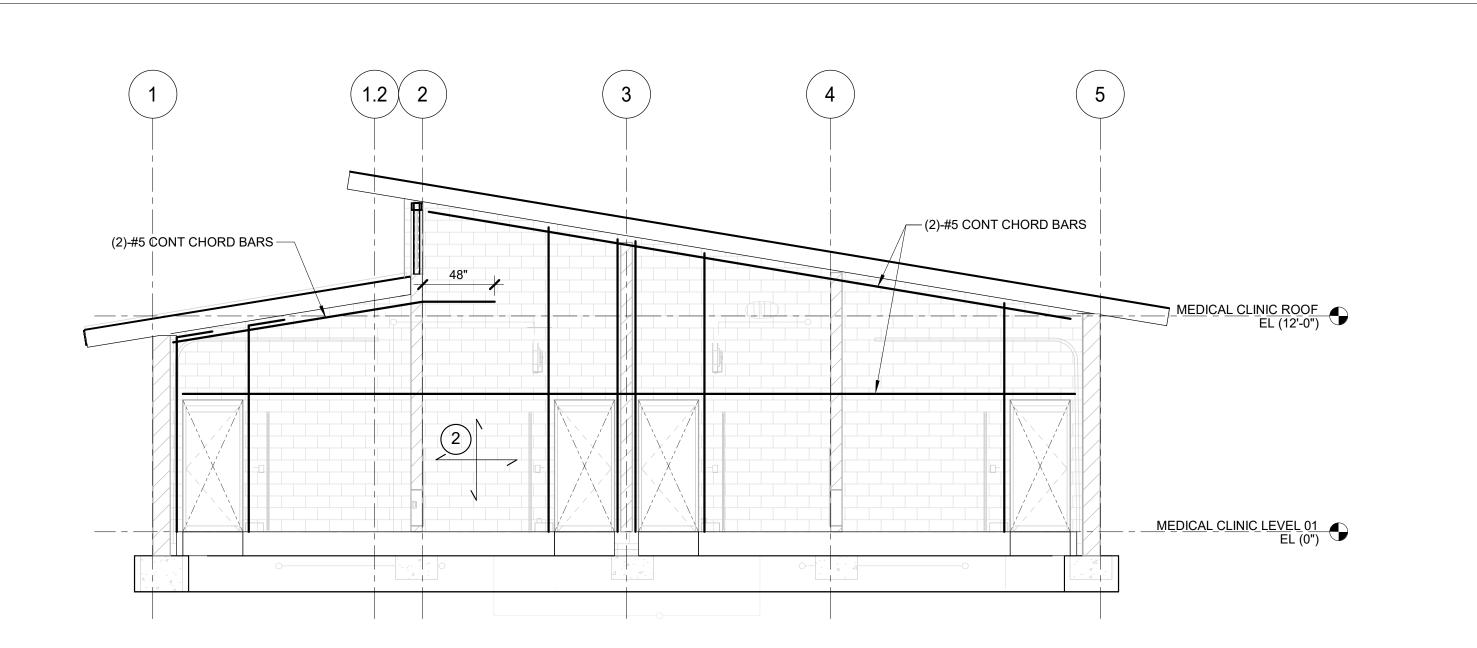
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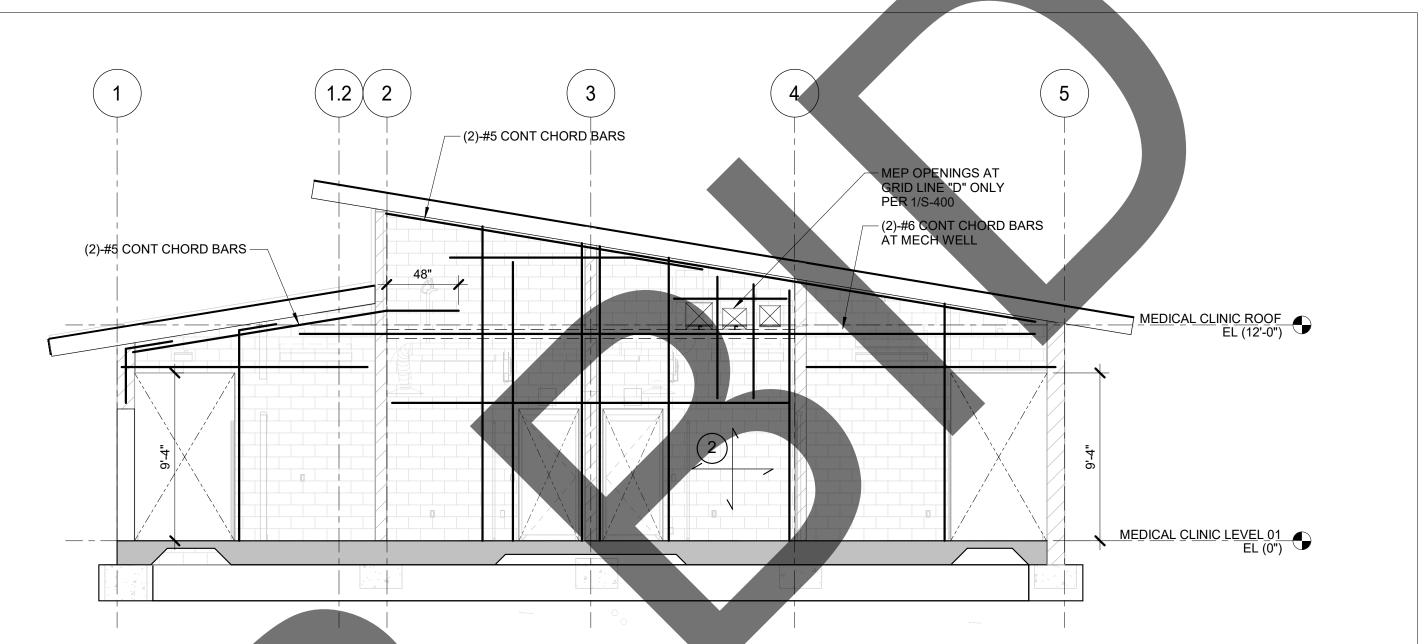
MEDICAL CLINIC WALL

**ELEVATIONS** 

SHEET NUMBER









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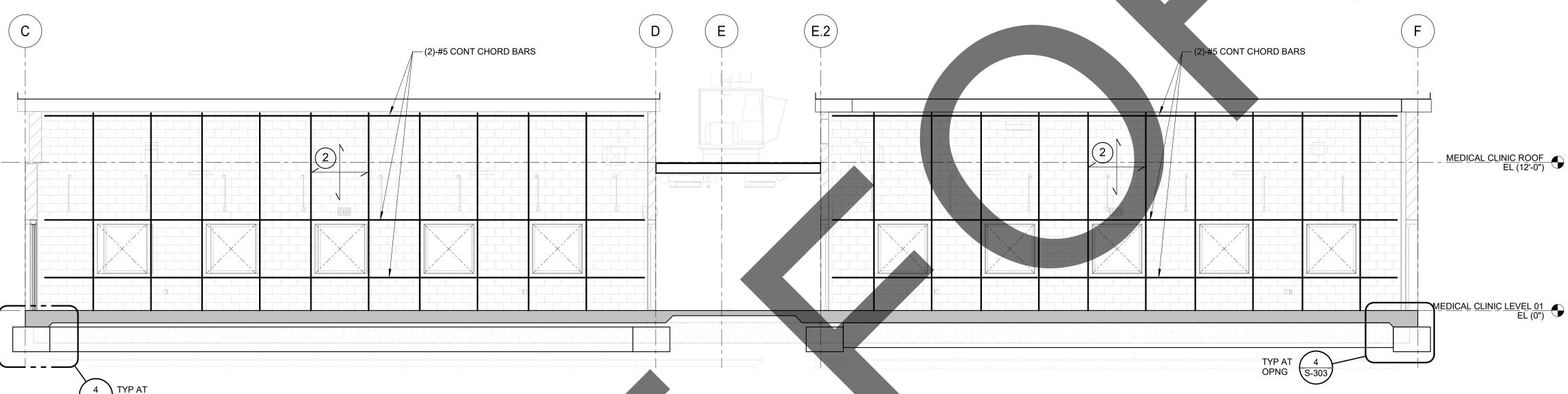
owner approval

**REVISIONS/ADDENDA** 

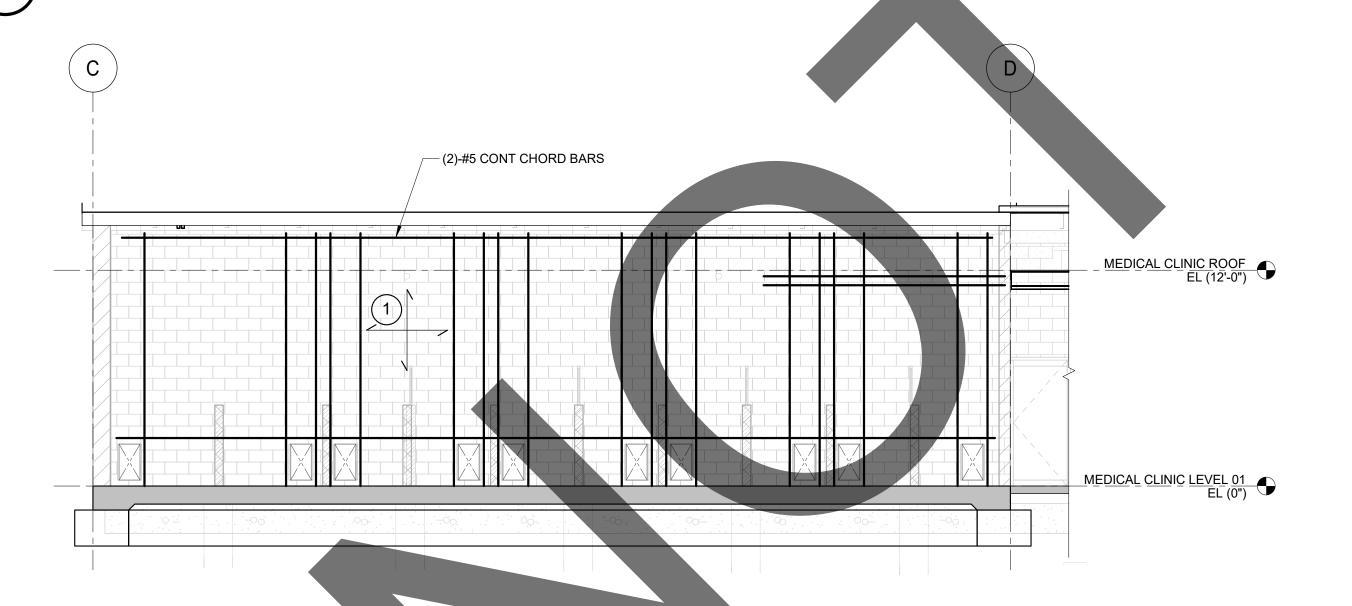
06/12/24 PLAN CHECK RESUBMITTAL

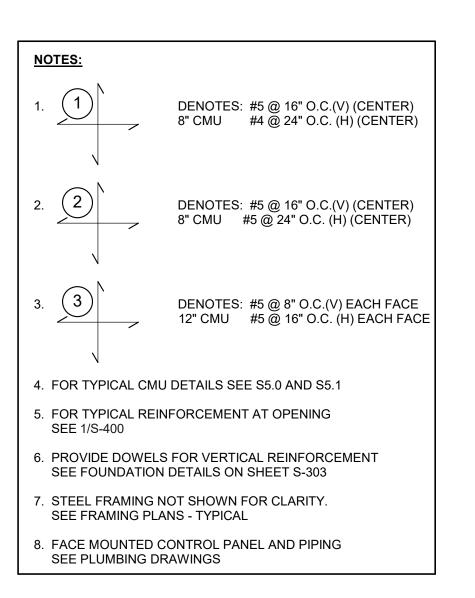
DOG CLINIC EXTERIOR WALL ELEVATION - GL "C" AND "F" - 12" THICK CMU

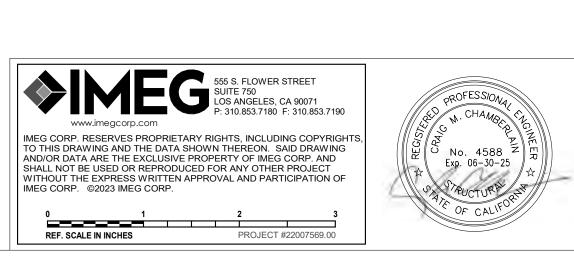
DOG CLINIC EXTERIOR WALL ELEVATION - GL "D" AND "E.2" - 8" THICK CMU

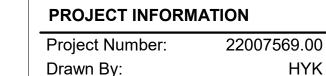


DOG CLINIC INTERIOR WALL ELEVATION - GL "3" - 8" THICK CMU









Drawn By: Checked By: 06/12/2024 Issue Date:

BERNARDINO

SHEET NAME

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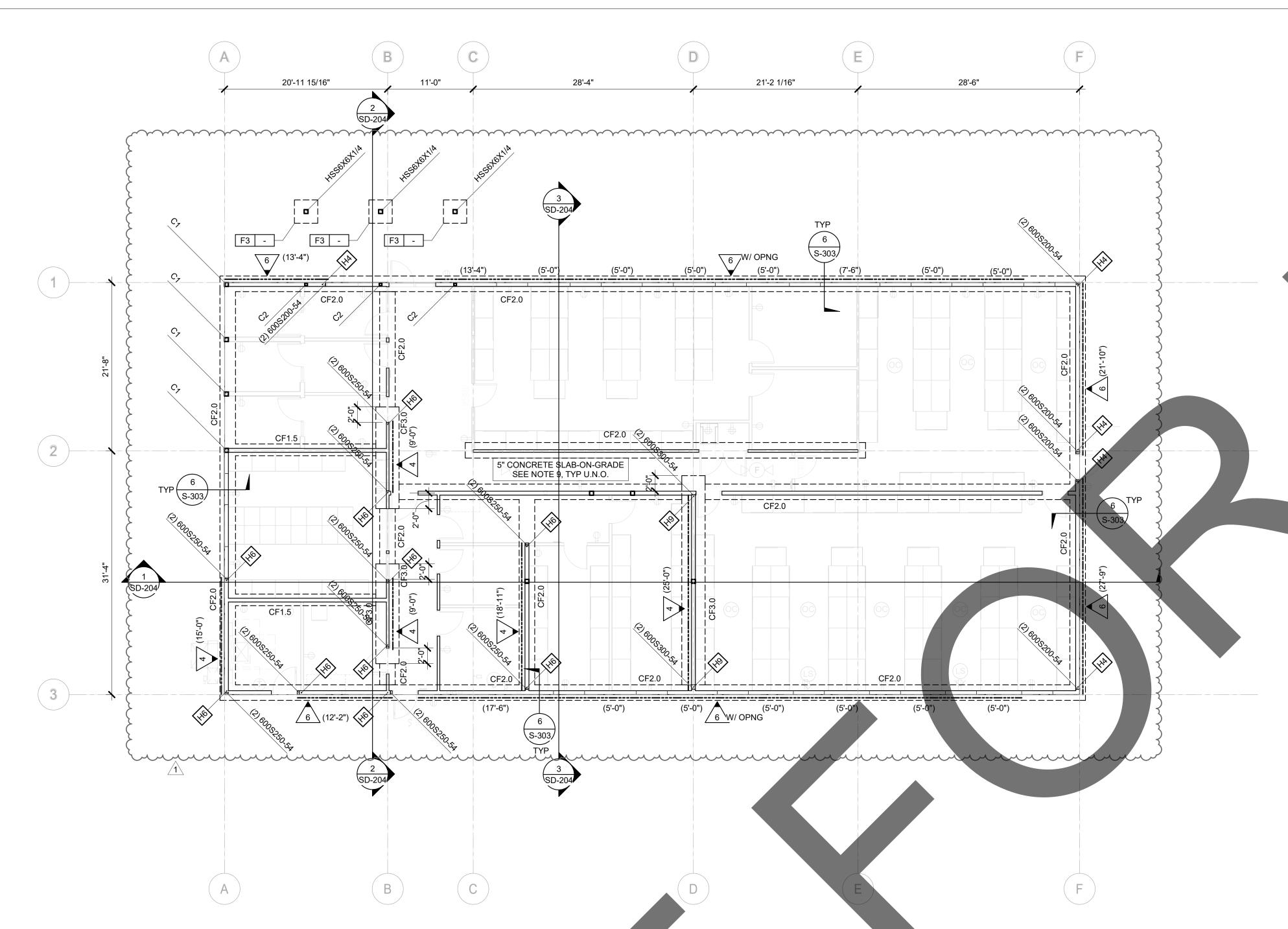
MEDICAL CLINIC WALL **ELEVATIONS** 

SHEET NUMBER

SBC-206

DOG CLINIC INTERIOR WALL ELEVATION - GL "4" - 8" THICK CMU

3/16" = 1'-0"



**CAT BUILDING FOUNDATION PLAN - BLDG D** 

#### **FOUNDATION PLAN NOTES**

- 1. SEE SHEET S-100 SERIES FOR STRUCTURAL NOTES. SEE SHEET \$-300 SERIES FOR TYPICAL CONCRETE DETAIL SEE SHEET S-500 SERIES FOR TYPICAL STEEL DETAILS.
  SEE SHEET S-700 SERIES FOR TYPICAL DOLD FORM STEEL DETAILS
- 2. TOP OF SLAB ON GRADE = 0'-0" UNO
- TOP OF FOOTING SHALL BE 1'-0" BELOW TOP OF SLAB OR FINISH GRADE, UNO.
- 4. S.A.D. FOR DIMENSIONS, ELEVATIONS, SLOPES, CURBS, STEPS, AND PADS NOTED ON PLAN.
- 5. COORDINATE LOCATION OF SLAB STEPS AND DEPRESSIONS TH ARCHITECTURAL DRAWINGS.
- 6. CONTRACTOR TO VERIFY ALL DIMENSIONS AND NOTIFY ARCHITECT OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION.
- 7. ALL FOUNDATION EXCAVATIONS MUST BE INSPECTED AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF REINFORCING STEEL.
- PRIOR TO THE CONTRACTOR REQUESTING A BUILDING RTMENT INSPECTION, THE SOILS ENGINEER SHALL ADVISE BUILDING OFFICIAL IN WRITING THAT: THE BUILDING PAD WAS PREPARED IN ACCORDANCE
- THE SOILS REPORT, UTILITY TRENCHES HAVE BEEN PROPERLY BACKFILLED AND COMPACTED, AND HE FOUNDATION EXCAVATIONS COMPLY WITH THE

NTENT OF THE SOILS REPORT

- TYPICAL SLAB ON GRADE: 5" THICK W/ #4 AT 18" O.C. EA WAY FOR UNDERLAYMENT
- DENOTES CONTINUOUS FOOTING. SEE SCHEDULE 1/S-303 - - - FOR FOOTING SIZE AND REINFORCEMENT
- 11. S—-- S DENOTES STEPPED FOOTING. SEE DETAIL 5/S-302
- 12. CONTRACTOR SHALL COORDINATE AND LOCATE ALL DUCT, PIPE, CONDUIT, ETC PENETRATIONS THRU WALLS AND FOOTINGS AND PROVIDE THE ASSOCIATED FRAMING AND FOUNDATION CONDITIONS PER THE TYPICAL DETAILS.
- DENOTES METAL STUD FRAMED WALL PER 1/S-700 AND 1/S-702

EDULE - BUILDING D
SIZE
HSS6X6X1/4
HSS4X4X1/4
HSS6X4X3/8

- REFER TO DETAILS 5/S-301 FOR ANCHOR BOLT AND BASE PLATE
- INFORMATION, UNO.

  2. REFER TO DETAILS 2/S-304 AND 3/S-304 FOR FOOTING INFORMATION AT STEEL COLUMNS, UNO.



interiors planning

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## initials date

#### **REVISIONS/ADDENDA**

1 06/12/24 PLAN CHECK RESUBMITTAL

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#### PROJECT INFORMATION

Project Number: Drawn By: Checked By: 06/12/2024 Issue Date:

SHEET NAME

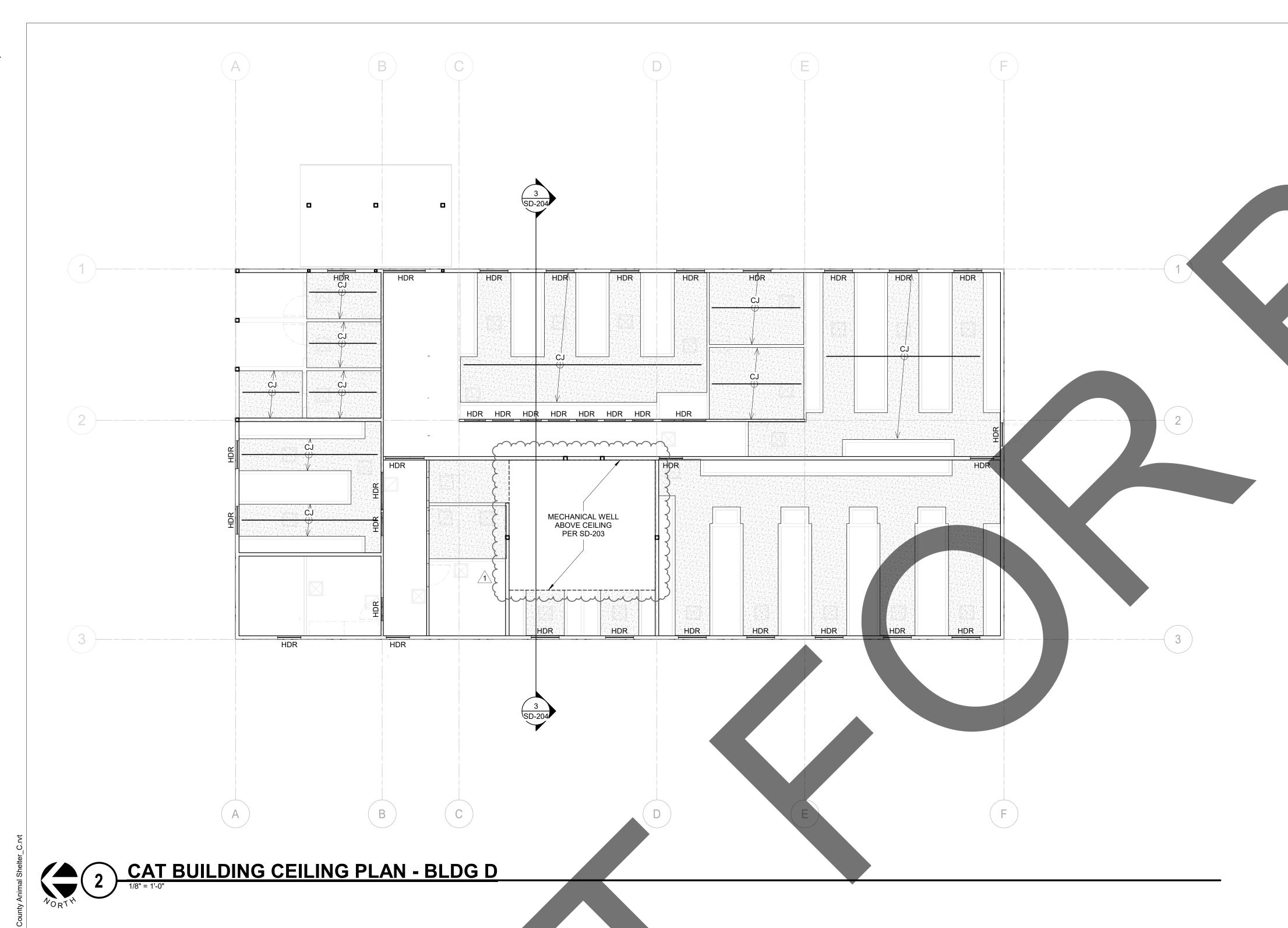
CAT BUILDING FOUNDATION PLAN

SHEET NUMBER

SD-201

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		of America		
owner approval				
initials	date	phase		

**REVISIONS/ADDENDA** 

1 06/12/24 PLAN CHECK RESUBMITTAL

SAN BERNARDINO COUNT

06/12/2024

PROJECT INFORMATION

Project Number:

CARE CENTER

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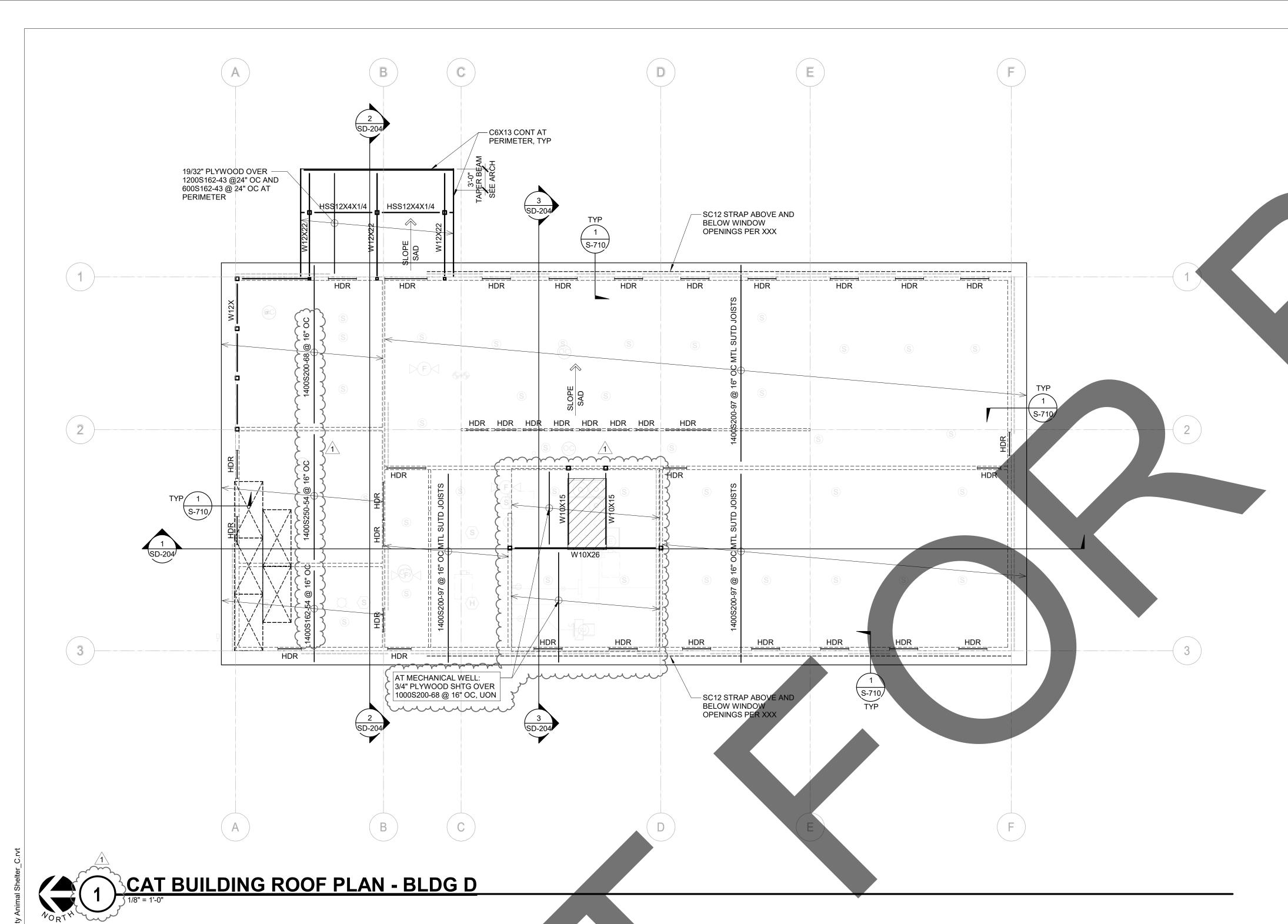
Drawn By: Checked By: Issue Date:

SHEET NAME

CAT BUILDING CEILING PLAN

SHEET NUMBER

SD-202



OOF PLAN NOTES

SEE SHEET **S-100** SERIES FOR STRUCTURAL NOTES.
SEE SHEET **S-700** SERIES FOR TYPICAL COLD FORM DETAILS.

- ALL DIMENSIONAL INFORMATION SHOWN IS BASED ON THE ARCHITECTURAL DRAWINGS. FOR ANY DIMENSIONAL INFORMATION NOT SHOWN REFER TO THE ARCHITECTURAL DRAWINGS.
- BEAM TO BE EQUALLY SPACED BETWEEN SUPPORTS UNO.
- 4. FOR COLUMN SIZES, SEE FOUNDATION PLAN.
- DENOTES MECHANICAL UNIT PER MEP DWGS
- DENOTES UNIT MAX OPER WT
  W/ CURB AND ALL ATTACHMENTS INCLUDED

architecture interiors planning

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owner approval

initials date phase

**REVISIONS/ADDENDA** 

1 06/12/24 PLAN CHECK RESUBMITTAL

06/12/2024

PROJECT INFORMATION

Project Number: Drawn By:

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CARE

ANIMAL

Checked By: Issue Date:

SHEET NAME

CAT BUILDING **ROOF PLAN** 

SHEET NUMBER

SD-203

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