

SECTION I

BASIS OF DESIGN TECHNICAL MEMORANDUM

LAKEVIEW LATION RENOVATION PROJECT

FOR

CC MUNITY SERVICES AREA 64 SPRING VALLEY LAKE, CA.

PROJECT NO.: 30.30.0023PART 1

Lakeview Sewer Lift Station Rehabilitation - Phase I

Basis of Design Technical Mem randum

CSA 64 - Spring Valley, CA

Prepared for:

San Bernardir J County
Department of Printing Work
Special Districts

Prepar d by:

Kimley >>> Horn

April 12, 2022

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ACRO IYMS

APN Accesso, arc Number CSA County Serve Area

DIP Γ⊿ctile Iron Piμ FPS eet Per Second

G_ເ 'າns GAL

Gallo. Per Day GPD. GP′، Gallons Per Minute

IN nches

KH (imley-Horn N' .. Minutes

MG. Million Gallons Per Day

Plans, Specifications, and Estimates P5L

RTU Remote Telemetry Unit

JCADA Supervisory Control and Data Acquisition

VCP Vitrified Clay Pipe

VVWRA Victor Valley Water Reclamation Authority

BASIS OF DESIGN TECHNICAL MEMO

To: San Bernardino County

Department of Public Works - Special Districts

Sarp Sekeroglu, P.E.

From: Renee Chuang, P.E.

Sam Wuellner, E.I.T.

Kimley-Horn and Associates, Inc.

Date: April 12, 2022

Subject: Lakeview Sewer Lift Station Rehabilitation

I. INTRODUCTION

Background

This Preliminary Design Report outlines the preliminary design for the rehabilitation of the Lakeview Sewer Lift Station in C^c , 64 'Project). Kimley-Horn has been retained by San Bernardino County Department c Public Wo. Special Districts (DPW-SD) to complete the design report and final design PS. for the rehabilitated lift station.

The Lakeview Lift Static is located on 2 0.12-acre parcel, APN 0480-241-01, between Spring Valley Lake and Lake in vibrory Drive. A vicinity map is presented in Exhibit A. The sewer lift station was constructed in 1971 and consists of two existing concrete wet well structures with hatches, screw pumps, elastical equip, and a SCADA system. The sewage is intercepted in Lakeview Drive and flows by gravity into the wet well. After it is pumped towards grade, the sewage flows by gravity lack into Lateview Drive where it continues flowing northwest, connecting to the downst name SA 64 system and eventually discharging to the Victor Valley Water Reclamation Authority VVV. And ystem for ultimate treatment. The two manholes in Lakeview Drive are connected by a sewer bypass to accommodate sewage flow during construction or maintenance to a lift status.

The Cou. ty has received several complaints of odors coming from the sewer lift station. During several site visits, it was observed that the screw pumps create high turbulence flows that may contribute to the malodorous conditions. Odors were not observed when the pumps were not running. Further, the sewer lift station screw pumps have a history of failure and surrounding equipment show signs of rust and corrosion. Kimley-Horn had previously prepared an Odor and Corrosion Control Study (Study) in 2018, for the Lakeview Lift Station in response to several

complaints from residences of odor. The Study also observed that odors were only detectable during pump operation.

The sewer lift station will be rehabilitated in two phases. Phase I will involve replacing the existing screw pumps and submersible pump with new submersible pumps and additional electrical connections. Photos of the lift station from Kimley-Horn's initial site visit are included in **Appendix A**. Phase II will include installation of a dedicated odor control system. The lift station odor and any new complaints will be monitored after the installation of submersible pumps ' deten. 'ne if an additional odor control system, or Phase II, is necessary.

Existing Conditions

The existing sewer lift station currently serves the residents are no Spring Valley Lake (approximately 4,215 residential lots). From the lift station, the wage so a northwest on Lakeview Drive to Tahoe Lane where it continues beyond the sewer of and connects to an existing sewer system. The existing sewershed is presented in **Exhibit**. As suilts provided by the DPW-SD depicting the existing lift station and surround sewer infrastructure are included in **Appendix B**.

Future Conditions

Future connections to the existing sewer lift station within CSA 1 include: Victory Valley College, Victor Valley College Stadium, Lakeview N Idle School, Endeavour School of Exploration, Excelsior Charter School, 39 additional side ial lots, and a gas station. The sewer feasibility studies for the future connections ar included in Leading C. These additional flows will be included in the lift station rehabilitation design, discussed further in Section II of this report.

II. RECOMMENDATIONS AND CONCLUSIONS

Existing Flow Ar - SCAD. Data

Kimley-Horn ob sined and reviewed SCADA data provided by DPW-SD. The SCADA data provided contains the number of starts and runtime per pump from 20R17-2021. By determining pump runtin, a number of aily starts for the pumps, and volume of the pump operating range within the wet will, the average sewer flow was calculated. The findings are summarized in **Table 1** and a pendix **D**.

Table 1 – SCADA Data Sewer Flow

Average Existing Sewer Flow	0.19	mgd
Operating Range Volume	307.60	gal
Average Runtime/Start	2.29	min

Future Flow Analysis

Projected sewer flow for the future connections to the sewer lift station were previously calculated by Webb Associates are included in the sewer feasibility studies in **Appendix C**. Additionally, the future flows are displayed in **Table 2**.

Table 2 - Future Sewer Flow

Victor Valley College (without stadium)	147000	gr
Lakeview Middle School	8820	gpa
Endeavour School of Exploration	8330	६ व
Excelsior Charter School	1470	gpu
Residential Lots (APN 0482-031-01,-02,-07,-08)	80 کا	24
Gas Station (APN 0482-043-08)	4৬. Դ	gp
Victor Valley College Stadium	24824	dر
Total Future Sewer Flow	0.22	mgd

The projected peak flow was calculated by summing up the xisting and future flows and applying a peaking factor. The DPW-SD Standards for fanital, Sew (November 13, 2012) specifies a peaking factor of 2.8 for average flows rangin between 3 and 0.5 mgd. The peak future sewer flow is displayed in **Table 3**.

Table - Fu' re Peak Sewer Flow

Pe Ki NW	1.05 mgd
Peakir _J Factor	2.8 unitless
Total Average Flov.	0.4 mgd

Lift Station Up rades

Kimley-Horn i entified the rehabilitation alternatives taking into consideration cost, constructal "ity, and mainty lance. All alternatives include:

- Replacing the satisfied screw pumps with two submersible pumps. See **Appendix F** for timinary pump design calculations and pump curve.
- eveling out the wet well base to accommodate the pumps. This will allow a larger operating volume in the existing wet well.
- Discharging via ductile iron pipe (DIP) force main with restrained joints into the existing 21-inch vitrified clay pipe (VCP) that connects the discharge structure to the downstream manhole in Lakeview Drive. The expected velocities in the force main at design conditions are approximately 3.78 fps.

The three alternatives are detailed below.

<u>Alternative 1 – Pump Replacement, Low Operating Range (KH Recommended)</u>

Alternative 1 consists of locating the submersible pumps under the existing 48" X 48" access hatch. An operating range of 1.5' will result in a cycle frequency to 16.8 cycles per hour. Despite the pump cycle frequency being above the typical design target of 6 cycles per hour, we confirmed with the pump manufacturer that the proposed pumps can accommodate up to 30 cycles per hour without jeopardizing the pump warranty. The pumps will alternate starts, alleviating the circuse of a single pump. A backup pump shall be held at the nearest maintenance yar. A breakdown of the costs for Alternative 1 is presented in **Appendix E**. Alternative 1 is deported in **Txhibit** and the costs for Alternative 1 is presented in **Appendix E**.

Alternative 2 – Pump Replacement, High Operating Range

Alternative 2 proposes the same pump layout as Alternative 1, bu with a supering range of 3' and a cycle frequency of 8.3 cycles per hour. This operating range will sult in surcharge of the upstream pipes. This alternative minimizes structural rework and optimize the frequency of pump starts and stops. In case of pump failure, a backup pump and the held at the nearest maintenance yard, similar to Alternative 1. The layout and operating ange of Alternative 2 is depicted in **Exhibit**C. Based on discussions with DPW-SD staff, the high log and get of Alternative 2 was not preferred, therefore a cost estimate was not performed.

Alternative 3 – Full Lift Station Rebuild

Alternative 3 consists of fully reconstructing the existing wet well to increase the base depth, preventing surcharging up the upstrum pipes. A minually, reconstructing the wet well allows for a more typical submersible pump we well ayout. This alternative, however, faces constructability concerns such as extensive shoring and dewatering resulting in substantially higher construction costs compared to Alternatives 1 & 2.

Piping Upgrades

Kimley-Horn utilization to the discharge manhole in Lakeview Drive. The results are displayed in **T** ble 4.

Table 4 - Bentley Flowmaster Results

Normal Depth	6.1	in
Velocity	3.55	fps
Channel Slope	0.5	%
Pipe Diameter	21	in
Pipe Material	VCP	
Discharge	925	gpm

In accordance with the DPW-SD Standards for Sanitary Sewer (November 13, 2012), the sewer velocity is above 2 FPS when flowing at less than 50% full. Therefore, no piping upgrades are required to accommodate the increase in sewer discharge.

Odor Control Upgrades

Phase I of the Lakeview Lift Station Rehabilitation Project does not include any odor control measures. Kimley-Horn anticipates elimination of odor complaints by replacing the source impossion for submersible pumps. Coupons will be installed to monitor for any corrosion is use within the lift station wet wells. Corrosion coupons, typically consisting of stainless steel or singer metal, the lift station with the wet well lift station. The composition is use within the wet well lift station. The composition is used to operators that there is a buildup of corrosive gases in the lift station with well and may need to be mitigated. While it is intended that Phase II – Odor Control Upgrades will not one required after the installation of submersible pumps, they will be accounted for in Phase II. The design to minimize future efforts if odor issues still arise. These design considerations will include uture odor control systems by reviewing power requirements, providing and in the odor control system.

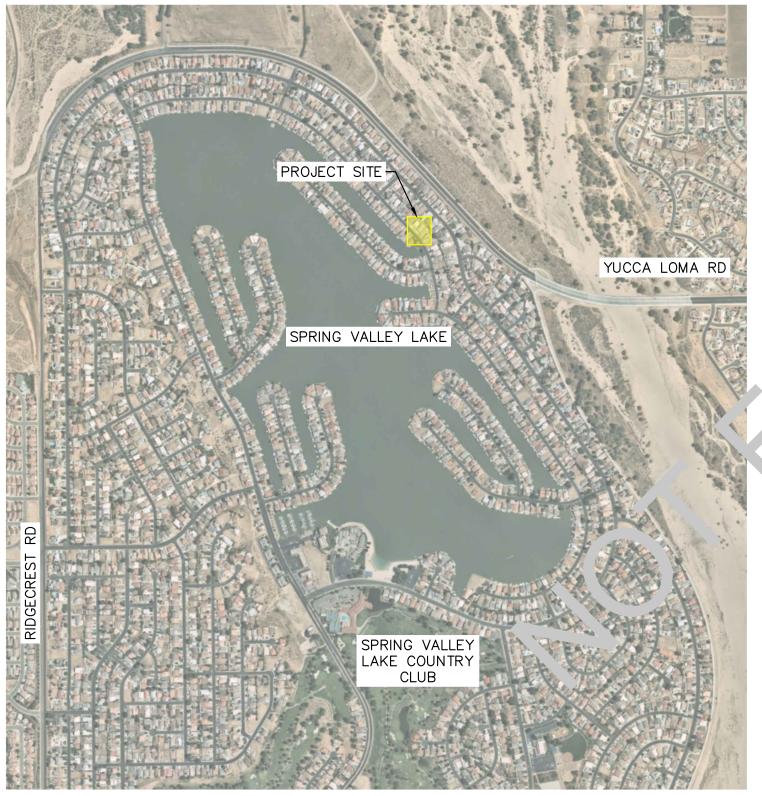
Electrical Upgrades

Kimley-Horn recommends installing a new e ctrical cor fol panel, a new RTU panel, and a disconnect to provide power for the pump, le el transducers, backup floats, and the upgraded SCADA system, which will be integrated by Coulty operators. A backup electrical generator is not required for this upgrade. The dov. strear sewer bypass in Lakeview Drive will accommodate sewer flow during construction or maintenance.

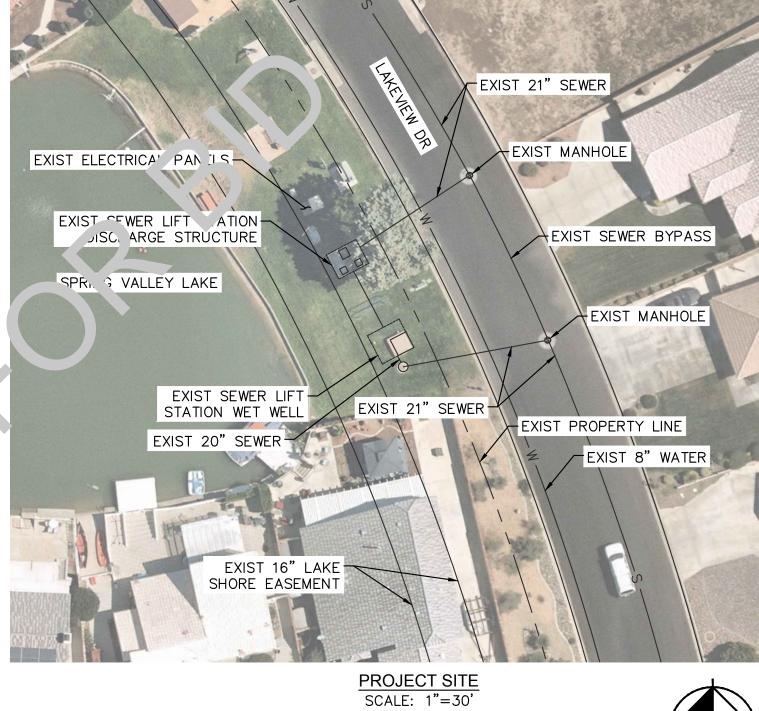


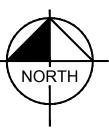
Sarp Sekeroglu, P.E. C 71474 Exp. 12/31/23





VICINITY MAP SCALE: 1"=1000"

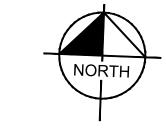




Kimley **Horn

EXHIBIT A - PROJECT VICINITY MAP LAKEVIEW LIFT STATION REHABILITATION

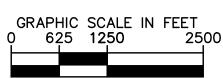


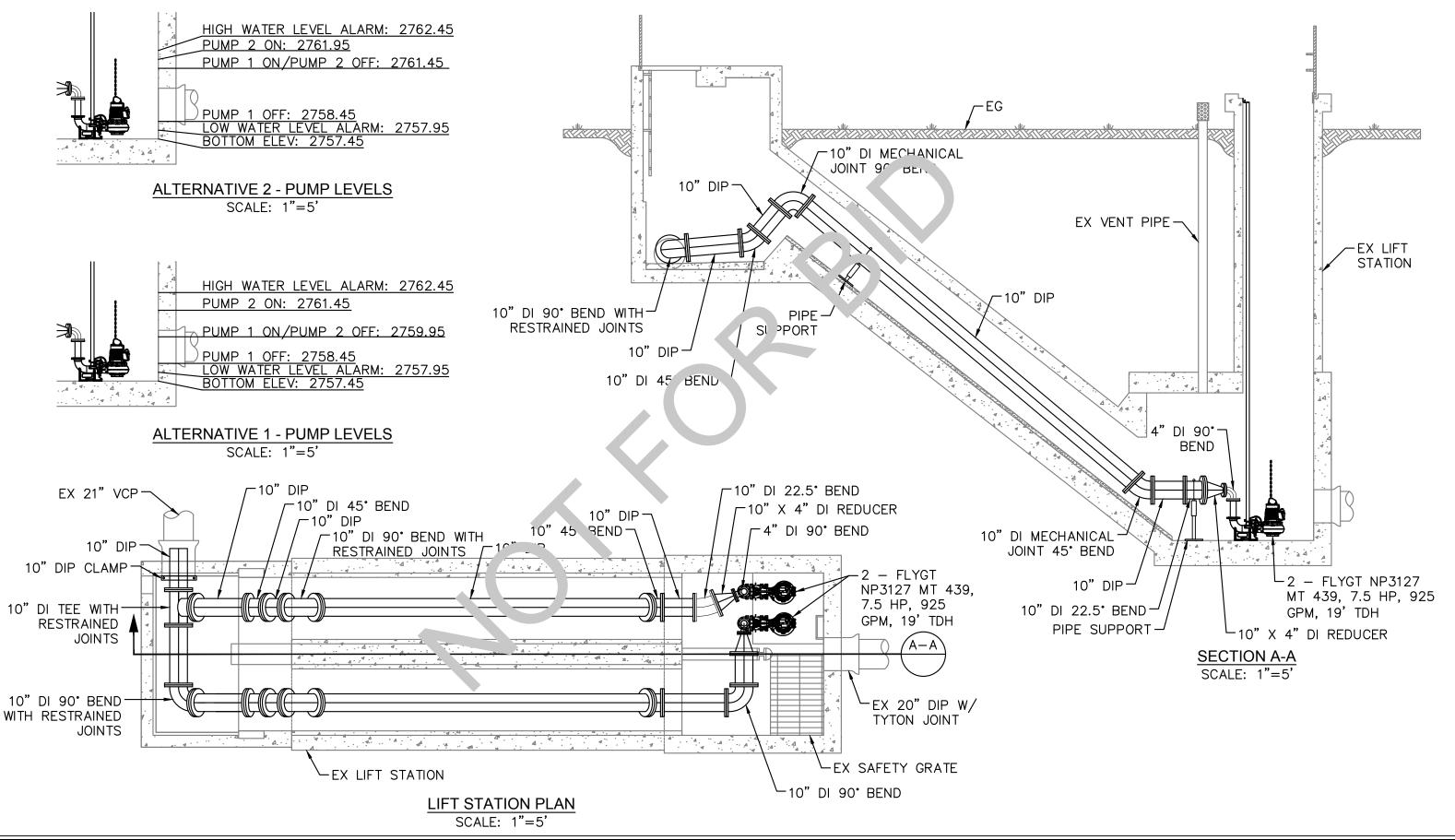


LEGEND

GRAVITY MAIN

FORCE MAIN



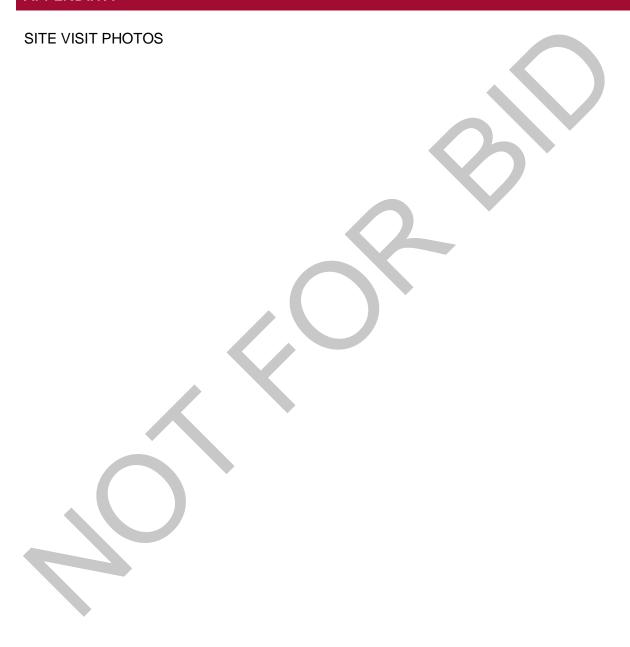


Kimley **Horn

EXHIBIT C - ALTERNATIVES 1 & 2
LAKEVIEW LIFT STATION REHABILITATION



APPENDIX A





scharge e Cover



Wet Well Hatch and Riser

Kimley » Horn

401 B Street, Suite 600 San Diego, CA 92101 Phone: (619) 234-9411

Site Visit Photos

Project:

Lakeview Sewer Lift Station Rehabilitation

Client: San Bernardino County Special Districts Department Job No. 195068122

Not to Scale

Page 1 of 3



Discharge Structure and Screw Pump

Kimley»Horn

401 B Street, Suite 600 San Diego, CA 92101 Phone: (619) 234-9411

Site Visit Photos

Project:

Lakeview Sewer Lift Station Rehabilitation

Client: San Bernardino County Special Districts Department Job No. 195068122

Not to Scale

Page 2 of 3



Discharge Structure and Screw Pumps

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401 B Street, Suite 600 San Diego, CA 92101 Phone: (619) 234-9411

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Job No. 195068122

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Page 3 of 3

LIFT STATION AS-BUILTS



IMPROVEMENT PLANS

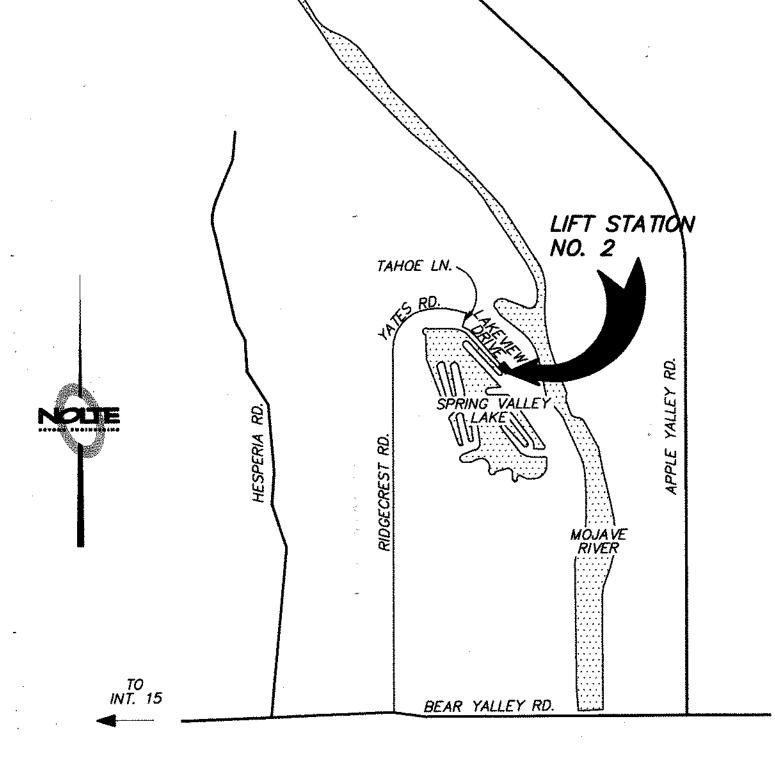
FOR

SAN BERNARDINO COUNTY SPECIAL DISTRICTS COUNTY SERVICE AREA 64

LAKE VIEW LIFT STATION

EXIST. 21" SANITARY SEWER

BOARD OF SUPERV



LOCATION MAP

The engineer preparing these plans will not be responsible for, or liable for, unauthorized changes to or uses of these

plans. All changes to the plans must be in writing and must be approved by the preparer of these plans.

INDEX OF DRAWINGS:

TITLE

SHEET NUMBER

TITLE SHEET & SITE PLAN 1
STRUCTURAL DETAILS 2ELECTRICAL PLANS & DIAGRAMS 3

REFERENCE DRAWINGS:

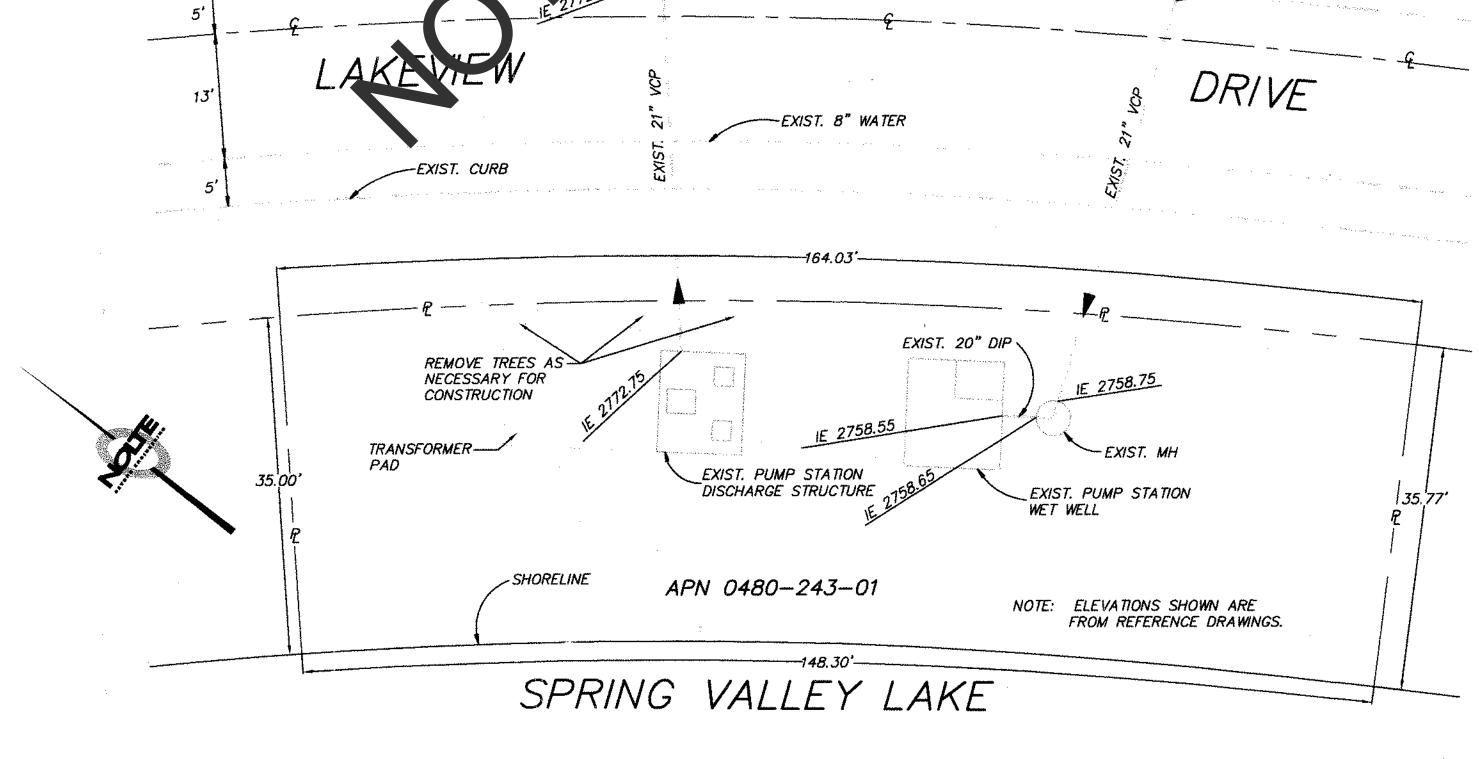
TITLE

SHEET NUMBER

LIFT STATION 2, SITE PLAN, VICINITY MAP & LOCATION MAP LIFT STATION 2, PLAN SECTIONS AND DETAILS

LIFT STATION 2, PLAN SECTIONS AND DETA

LIFT STATION 2, STRUCTURAL DETAILS





NOTE: PROPERTY LINE INFORMATION SHOWN ON SHEET 1 OF ATTACHED AS—BUILT PLANS.

GENERAL NOTES:

- THE ACCURACY OF THE LOCATION OF OR THE EXISTANCE OR NONEXISTANCE OF ANY UTILITY PIPE OR STRUCTURE WITHIN THE LIMITS OF THIS PROJECT DOES NOT CONSTITUTE RESPONSIBILITY BY THE ENGINEER.
- 2. ALL WORK SHALL CONFORM TO SPECIFICATIONS AND RULES & REGULATIONS OF SAN BERNARDINO COUNTY SERVICE AREA NO. 64, THESE PLANS AND SPECIFICATIONS
- 3. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING HIS OPERATIONS WITH OTHER CONTRACTORS AND UTILITY COMPANIES IN THE CONSTRUCTION AREA.
- 4. THE CONTRACTOR SHALL NOTIFY THE COUNTY SERVICE AREA ENGINEER, AND INSPECTOR ONE WEEK IN ADVANCE OF WHEN HE PLANS TO START CONSTRUCTION. AT THAT TIME OR ANY SUCH TIME PRIOR TO THAT AS MAY BE SPECIFIED BY THE ENGINEER, THE CONTRACTOR SHALL SUBMIT SCHEDULE OF HIS WORK SHOWING PRINCIPAL OPERATIONS AND THEIR ESTIMATED STARTING DATES. WHEN INSPECTIONS OR ENGINEERING JUDGEMENTS BECOME NECESSARY AS SET FORTH IN THE SPECIFICATIONS THE CONTRACTOR SHALL GIVE AT LEAST TWENTY FOUR (24) HOURS NOTICE TO THE COUNTY SERVICE AREA ENGINEER.
- NO REVISIONS SHALL BE MADE IN THESE PLANS WITHOUT THE APPROVAL OF THE ENGINEER.

PROJECT NOTES:

- 1. THE CONTRACTOR SHALL ENFORCE ALL SAFETY MEASURES. THE CONTRACTOR SHALL DESIGN, CONSTRUCT, AND MAINTAIN ALL SAFETY DEVICES, INCLUDING SHORING, AND SHALL BE SOLELY RESPONSIBLE FOR CONFORMING TO ALL LOCAL, STATE, AND FEDERAL SAFETY AND HEALTH STANDARDS, LAWS, AND REGULATIONS.
- 2. CONTRACTOR SHALL ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY. THIS REQUIREMENT SHALL OCCUR CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS.
- 3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY MONUMENTATION AND/OR BENCHMARKS WHICH MAY BE DISTURBED OR DESTROYED BY CONSTRUCTION. SUCH POINTS SHALL BE REFERENCED AND REPLACED WITH APPROPRIATE MONUMENTATION BY A LICENSED LAND SURVEYOR OR A REGISTERED CIVIL ENGINEER AUTHORIZED TO PRACTICE LAND SURVEYING. A CORNER RECORD OR RECORD OF SURVEY, AS APPROPRIATE, SHALL BE FILED BY THE LICENSED LAND SURVEYOR OR REGISTERED CIVIL ENGINEER AS REQUIRED BY THE LAND SURVEYORS ACT.
- 4. CONTRACTOR SHALL PREVENT DEBRIS FROM ENTERING SPRING VALLEY LAKE DURING CONSTRUCTION.
- 5. CONTRACTOR SHALL REPLACE OR REPAIR IN KIND ALL HARDSCAPE, LANDSCAPE, IRRIGATION, PAVEMENT, STRIPING, HEADWALLS, ETC.
- 6. THE CONTRACTOR SHALL ENSURE THAT CONTINUOUS SEWER AND WATER SERVICES ARE PROVIDED DURING CONSTRUCTION OF THESE IMPROVEMENTS TO ALL FACILITIES SERVED BY THE WATER AND SEWER LINES.
- 7. ALL MATERIAL WHICH IS NOT SUITABLE FOR USE ON THIS PROJECT SHALL BECOME THE PROPERTY OF THE CONTRACTOR AND SHALL BE REMOVED FROM THE SITE AND DISPOSED OF IN A MANNER, AND AT A LOCATION ACCEPTABLE TO ALL COGNIZANT AGENCIES, AT CONTRACTOR'S OWN COST.
- 8. THE CONTRACTOR SHALL FAMILIARIZE HIMSELF WITH THE SITE AND SHALL BE SOLELY RESPONSIBLE FOR ANY DAMAGE TO EXISTING FACILITIES RESULTING DIRECTLY OR INDIRECTLY FROM HIS OPERATIONS, WHETHER OR NOT SUCH FACILITIES ARE SHOWN ON THESE PLANS.
- 9. SHOULD THE PROJECT REQUIRE ANY ROAD WORK/REPAIR DURING CONSTRUCTION FROM EQUIPMENT, THE CONTRACTOR SHALL REPAIR THE ROAD PER COUNTY TRANSPORTATION DEPARTMENT (SEE SPECIFCATIONS).

CALL BEFORE YOU DIG -AT LEAST 2

CALL BEFORE YOU DIG -AT LEAST 2
DAYS IN ADVANCE 1-800-227-2600

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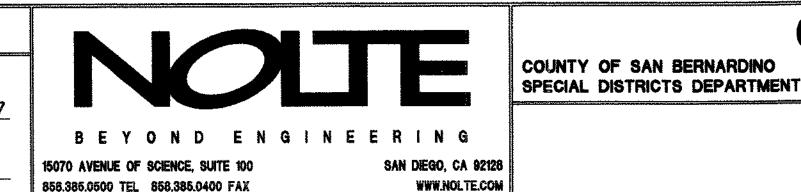
NO. BY DATE REVISIONS:

COUNTY OF SAN BERNARDINO - SPECIAL DISTRICTS DEPARTMENT COUNTY SERVICE AREA 64

APPROVED BY:

JM ORAVETS, ENGINEERING MANAGER

DATE



COUNTY SERVICE AREA 64

157 W. 5TH ST. 2ND FLOOR SAN BERNARDINO, CA 92415

SCALE
VERTICAL: 1"= N/A
HORIZONTAL: 1"= N/A

LAKE VIEW LIFT STATION
TITLE SHEET & NOTES

JOB NUMBER SDB025602

SHEET NUMBER

GENERAL NOTES:

- 1. NOTIFY THE OWNER OF ANY DISCREPANCIES FOUND BEFORE PROCEEDING WITH THE WORK
- 2. THE CONTRACTOR SHALL NOTIFY THE OWNER OF ANY SITE CONDITIONS NOT REFLECTED ON THE DRAWINGS; OF DISCREPANCIES IN MIN. DIMENSIONS INDICATED, SUCH AS GREATER RETAINED EARTH HEIGHTS, CONFLICT IN GRADES, EXTENTS OF BAD SOIL, HEIGHT OF GROUND WATER, DEPTHS OF FOUNDATIONS, ETC., AND ESPECIALLY OF UNCOVERED AND UNEXPECTED UTILITY LINES
- DIMENSIONS, LOCATIONS AND SIZES OF OPENINGS IN FLOORS AND WALLS SHALL BE COORDINATED BY THE CONTRACTOR WITH ALL APPLICABLE DRAWINGS. ALL PENETRATIONS MAY NOT BE INDICATED ON STRUCTURAL DRAWINGS
- ALL WORK NOT DETAILED OR NOTED SHALL BE CONSTRUCTED IN ACCORDANCE WITH OTHER SIMILAR WORK SHOWN ON THE DRAWINGS AND ON TYPICAL DETAILS.
- NO PIPES OR DUCTS SHALL BE PLACED IN SLABS OR WALLS UNLESS SPECIFICALLY DETAILED OR APPROVED BY THE OWNER
- DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MEANS AND METHODS OF CONSTRUCTION INCLUDING DEWATERING, SHORING, FORMING, AND TEMPORARY BRACING, HE SHALL UNDERTAKE ALL NECESSARY MEASURES TO INSURE SAFETY OF ALL PERSONS AND STRUCTURES AT THE SITE. OBSERVATION VISITS TO THE SITE BY THE OWNER SHALL NOT INCLUDE REVIEW OF THESE MEASURES.
- UNLESS OTHERWISE SHOWN, LOCATION OF ALL CONSTRUCTION JOINTS SHALL HAVE THE APPROVAL OF THE ENGINEER. IN FLUID RETAINING STRUCTURES, PVC WATERSTOPS SHALL BE PROVIDED IN ALL CONSTRUCTION JOINTS.

REINFORCING STEEL

- 1. DETAILING, FABRICATION AND PLACEMENT OF REINFORCING BARS (UNLESS OTHERWISE NOTED) MUST FOLLOW THE A.C.I. MANUAL OF STANDARD PRACTICE FOR DETAILING REINFÓRCED CONCRETE STRUCTURES, A.C.I. 315 LATEST EDITION.
- 2. . ALL REINFORCING BARS SHALL CONFORM TO THE STANDARD SPECIFICATION FOR DEFORMED BILLET-STEEL BARS FOR CONCRETE REINFORCEMENT, ASTM DESIGNATION A615-85: GRADE 60 EXCEPT #3 RE-BARS SHALL BE GRADE 40.
- 3. LAP ALL SPLICES IN CONCRETE AS "CLASS B" SPLICES MINIMUM, UNLESS OTHERWISE
- REINFORCING STEEL SHALL HAVE A MINIMUM CONCRETE COVER AS TABULATED BELOW UNLESS OTHERWISE NOTED. A. FOOTINGS AND SLABS CAST AGAINST EARTH 3--IN.
 - WALLS EXPOSED TO SOIL
- 2-IN.
- WHERE CONTINUOUS BARS ARE CALLED OUT, PROVIDE CONTACT SPLICES (AS REQUIRED) IN ACCORDANCE WITH REINFORCING STEEL NOTE NO. 3.
- WELDING OF MILD STEEL SHALL BE PROHIBITED.
- BOTTOM STEEL OF SLABS, FOOTINGS AND GRADE BEAMS SHALL BE SUPPORTED OFF OF THE EARTH OR FORMS BY PRECAST CONCRETE BLOCKS WIRE TIED TO THE

CONCRETE

- CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 19 OF THE UBC, 1997 EDITION.
- 2. ALL REINFORCING BARS, ANCHOR BOLTS AND INSERTS SHALL BE WELL SECURED PRIOR TO POURING CONCRETE.
- STRUCTURAL CONCRETE COMPRESSIVE DESIGN STRENGTH AT 28 (f'c) DAYS SHALL BE AS FOLLOWS: ALL - 2500 PSI (SPECIAL INSPECTION NOT REQUIRED)
- 4. ALL EXPOSED CONCRETE EDGES SHALL HAVE 3/4" CHAMFER.

SOIL & FOUNDATIONS

- 1. CHARACTER OF SOIL: COMPRESSIBLE ALLUVIAL SOILS
- 2. ALLOWABLE SOIL BEARING PRESSURE: N/A
- NO CONCRETE OR REBAR SHALL BE PLACED IN ANY FOUNDATION UNTIL THE EXCAVATION HAS BEEN INSPECTED BY THE OWNER.

REFERENCE DRAWINGS:

FOR REINFORCEMENT DETAILS OF EXISTING WETWELL SEE REFERENCE DRAWING SHEETS

DEMOLITION:

CONTRACTOR SHALL REMOVE AND DISPOSE OF EXISTING REMOVABLE CONCRETE TOP STRUCTURE AS IDENTIFIED ON REFERENCE DRAWING SHEET 3, VERTICAL SECTION-UPPER

ABBREVIATIONS ANCHOR BOLT

CENTERLINE CLEAR CONCRETE CONTINUOUS EACH FACE **EMBEDMENT** EACH WAY **EXISTING**

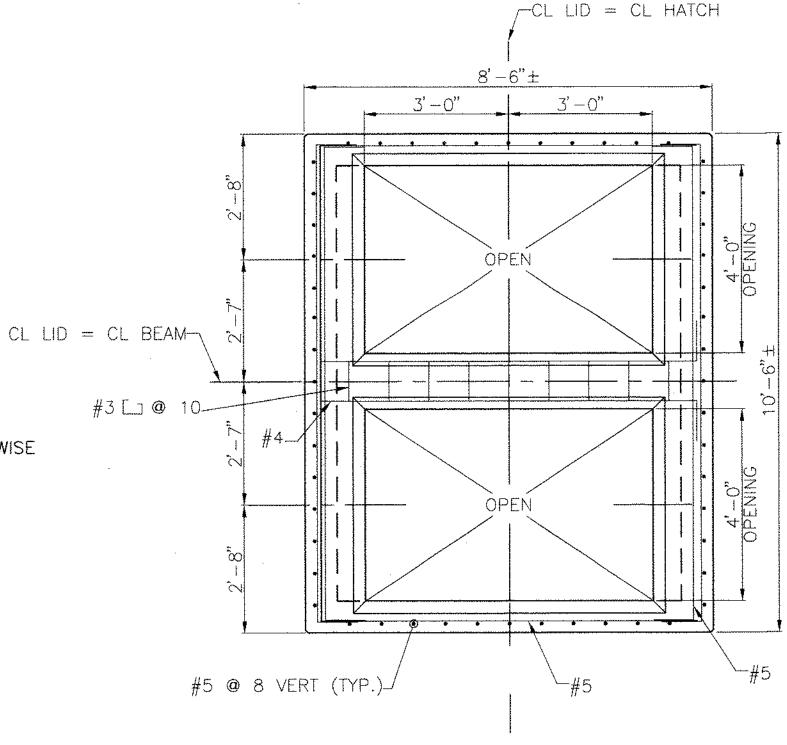
ALUMINUM

FAR SIDE **GALVANIZED** MATERIAL MAXIMUM MINIMUM NOT TO SCALE

NEAR SIDE TOP AND BOTTOM TOTAL ON CENTER **TYPICAL**

UNLESS NOTED OTHERWISE

VERTICAL WATER STOP DIAMETER

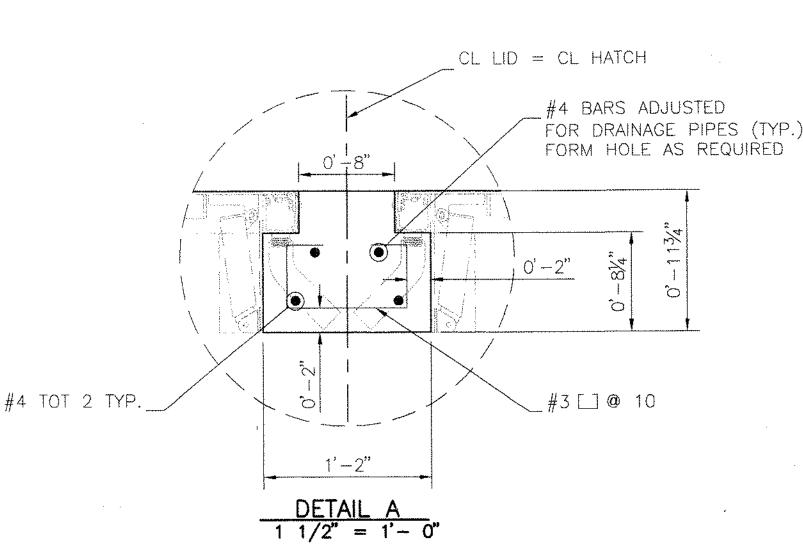


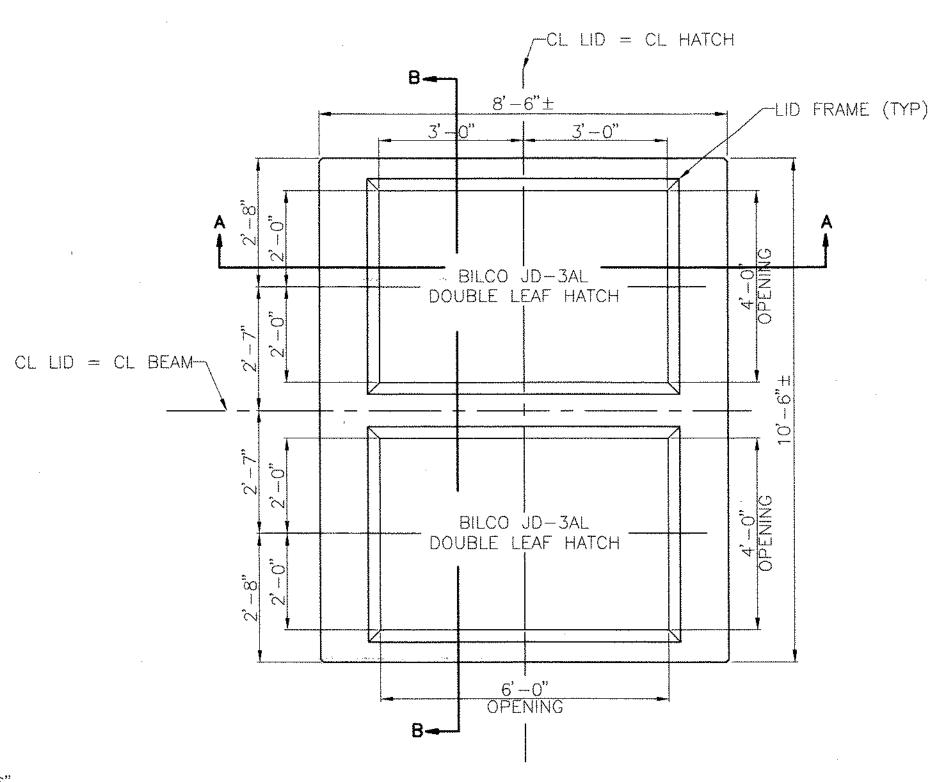
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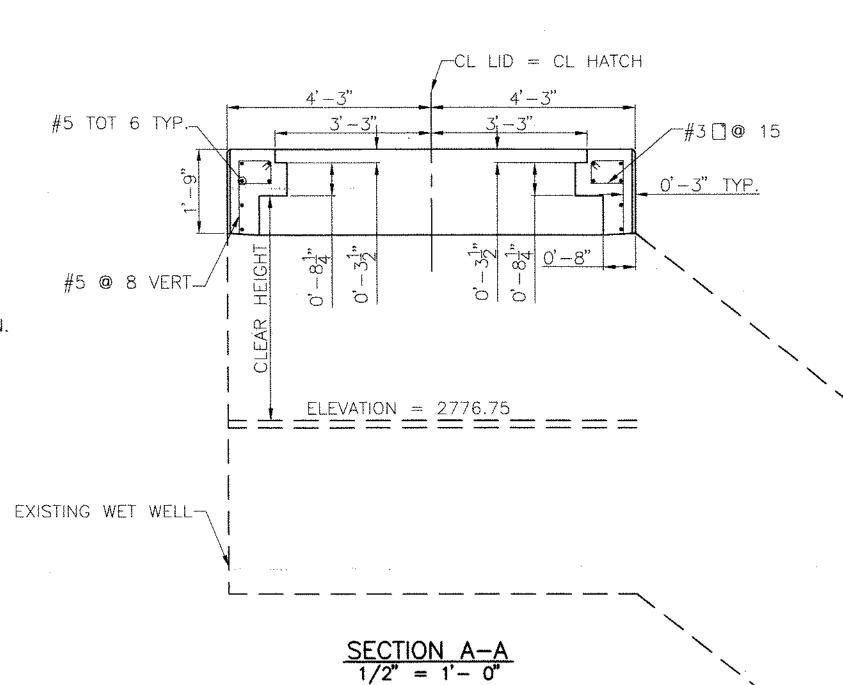
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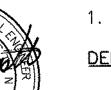
WWW.NOLTE.COM

SEE SPECIFICATION SECTION 08310 FOR HATCH AND LOCKING INFORMATION.









COUNTY SERVICE AREA 64

COUNTY OF SAN BERNARDINO 157 W. 5TH ST. 2ND FLOOR SPECIAL DISTRICTS DEPARTMENT SAN BERNARDINO, CA 92415

CALL BEFORE YOU DIG -AT LEAST 2 DAYS IN ADVANCE 1-800-227-2600

NO. BY DATE REVISIONS: COUNTY OF SAN BERNARDINO - SPECIAL DISTRICTS DEPARTMENT DATE: <u>5/25/07</u> TIME: <u>12: 32: 47 PM</u> **COUNTY SERVICE AREA 64** SERVER: NONE LAYOUT: Layout1 PATH: N:\SDB025602\DELV\20070523\DRAWINGS 6/15/07 DATE DESIGNER: _____CLG ___PROJ. MGR: ___ CAUTION: The engineer preparing these plans will not be responsible for, or liable for, unauthorized changes to or uses of these plans. All changes to the plans must be in writing and must be approved by the preparer of these plans.

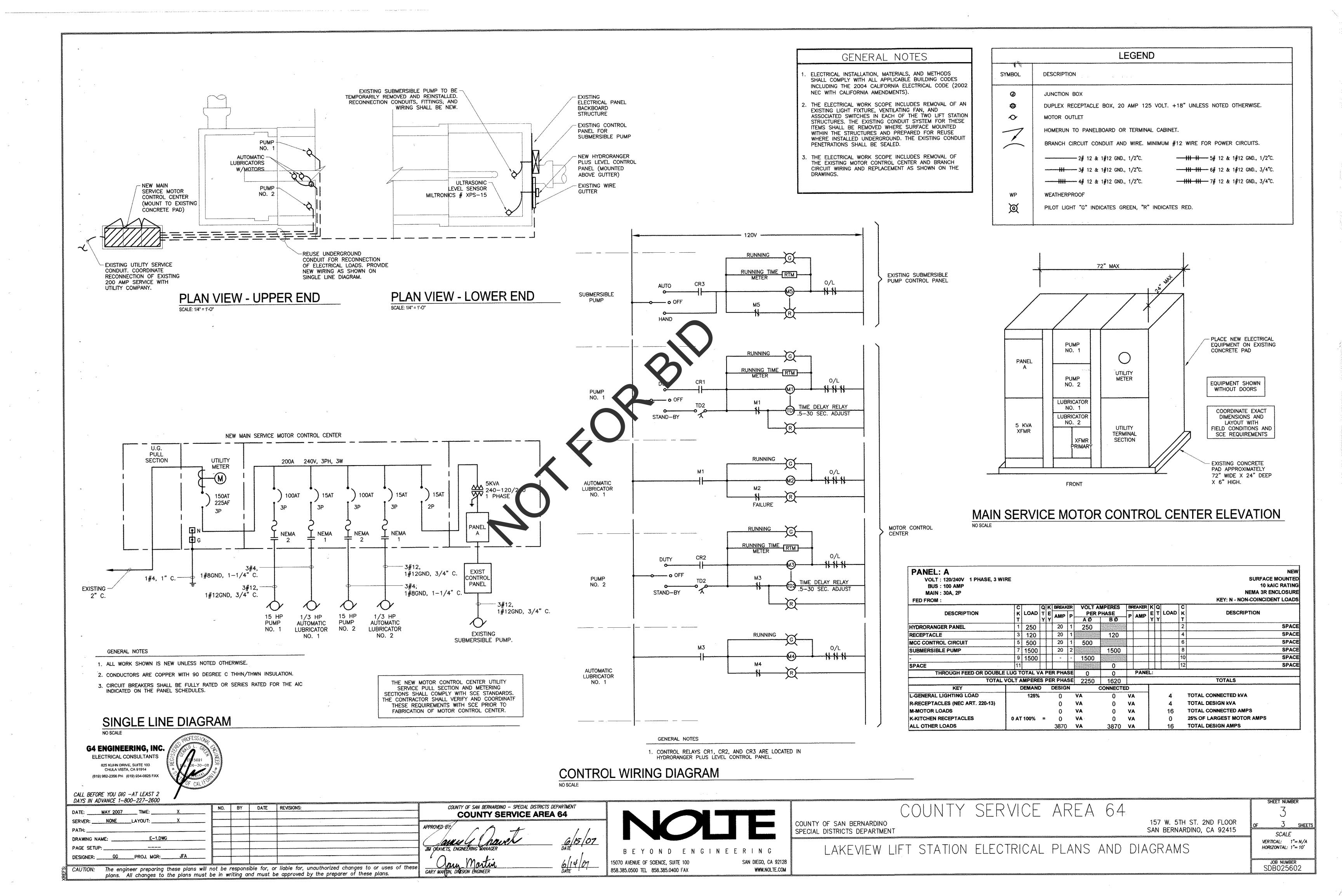
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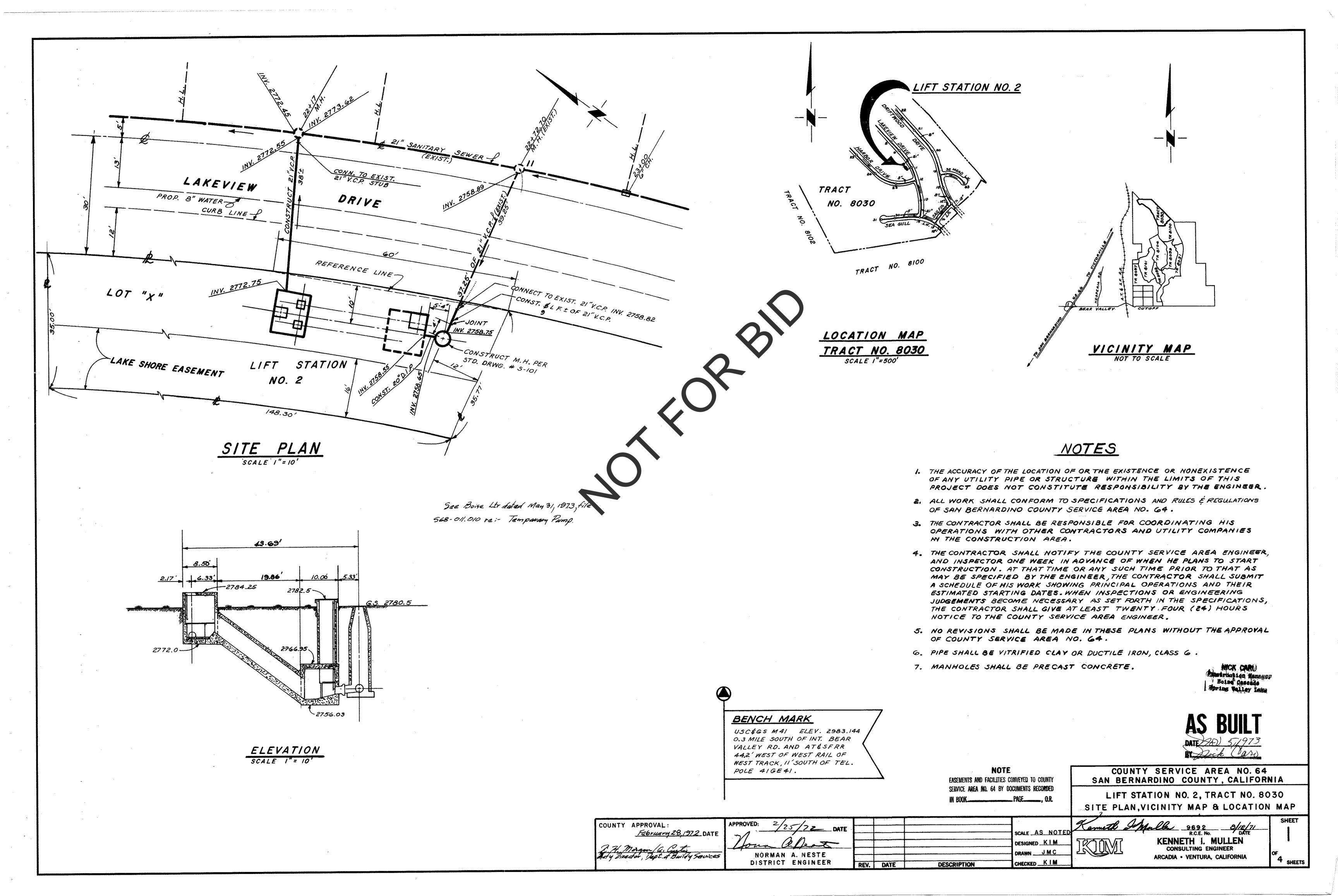
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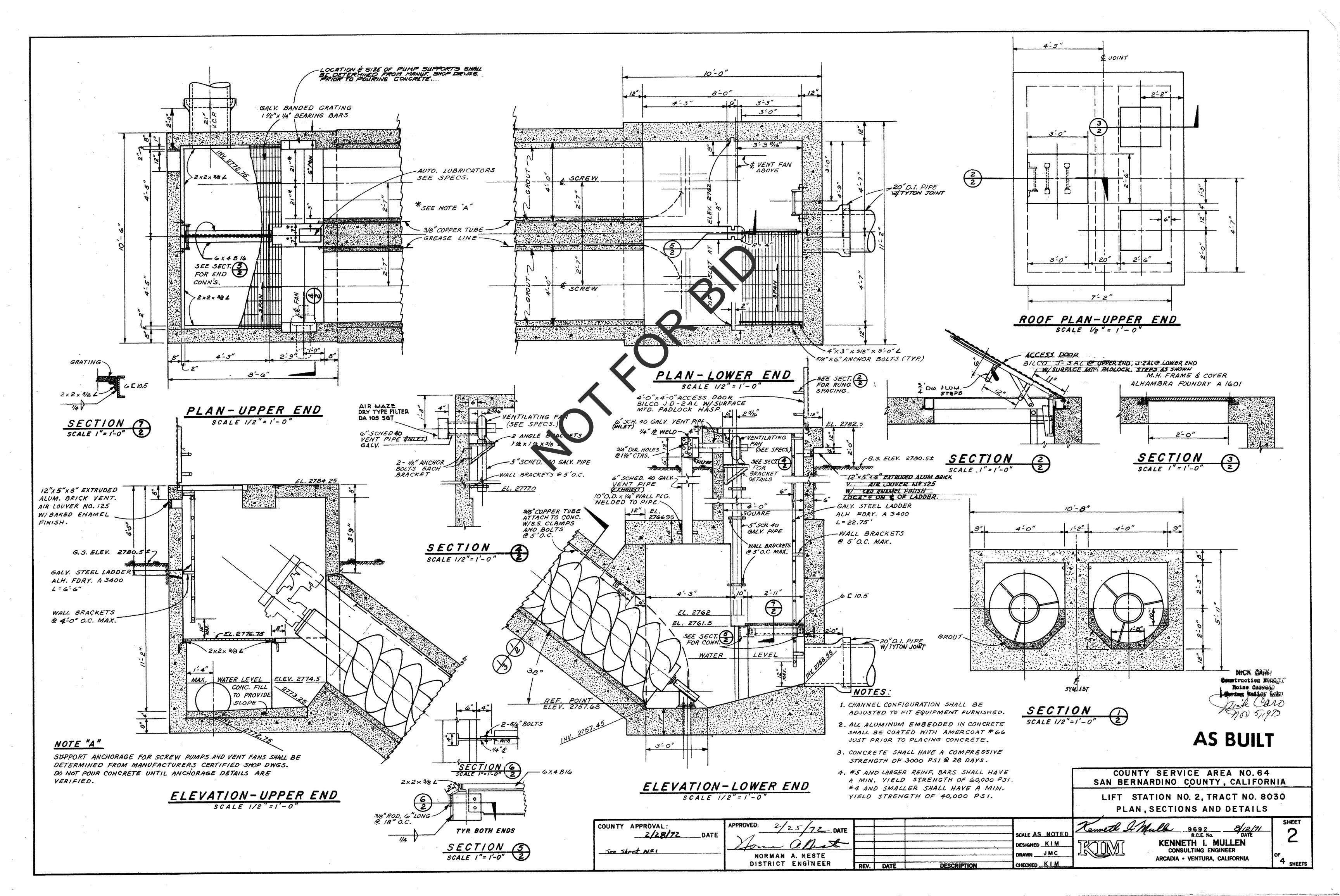
LAKE VIEW LIFT STATION STRUCTURAL DETAILS

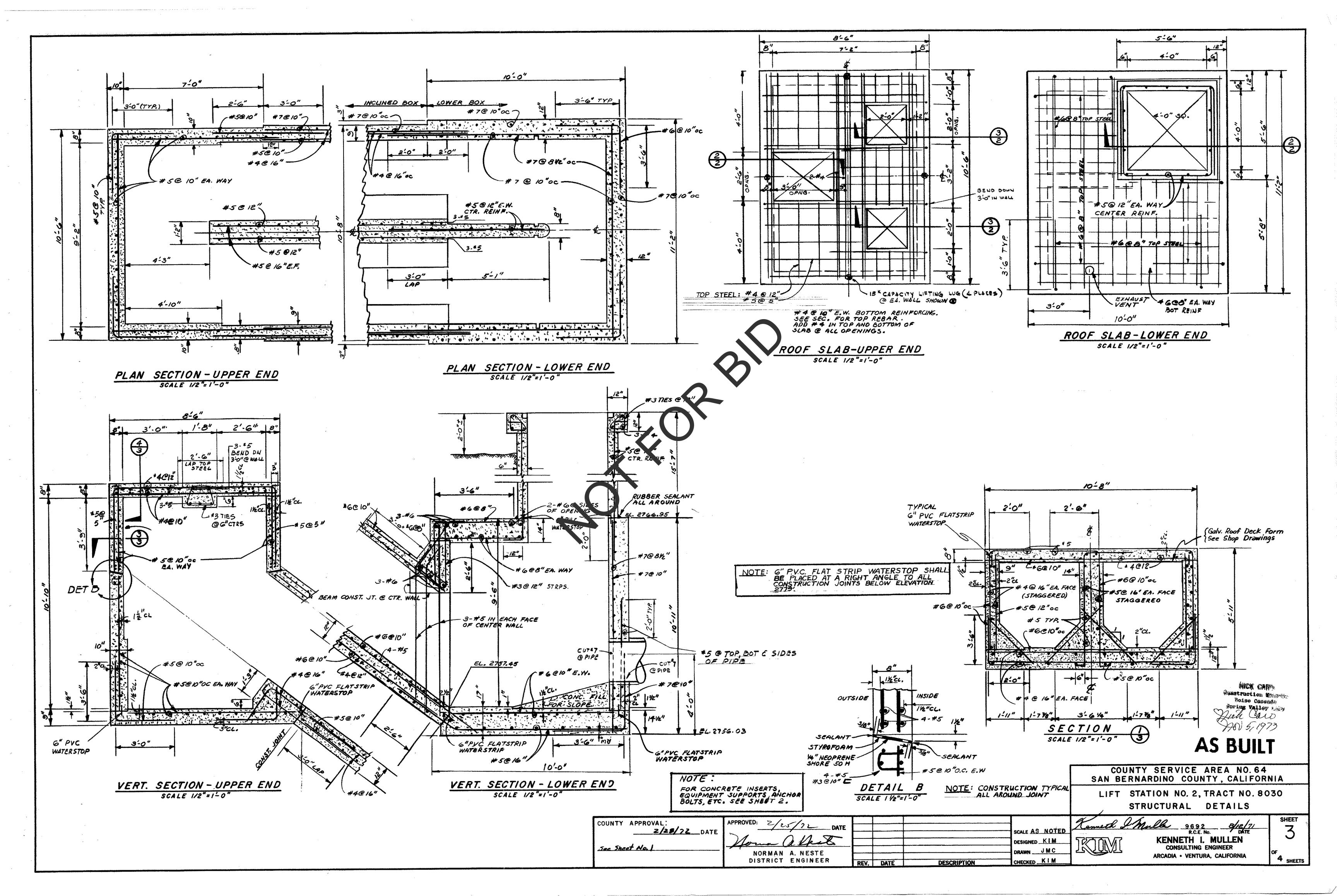
VERTICAL: 1"= N/A HORIZONTAL: 1"= N/A JOB NUMBER SDB035300

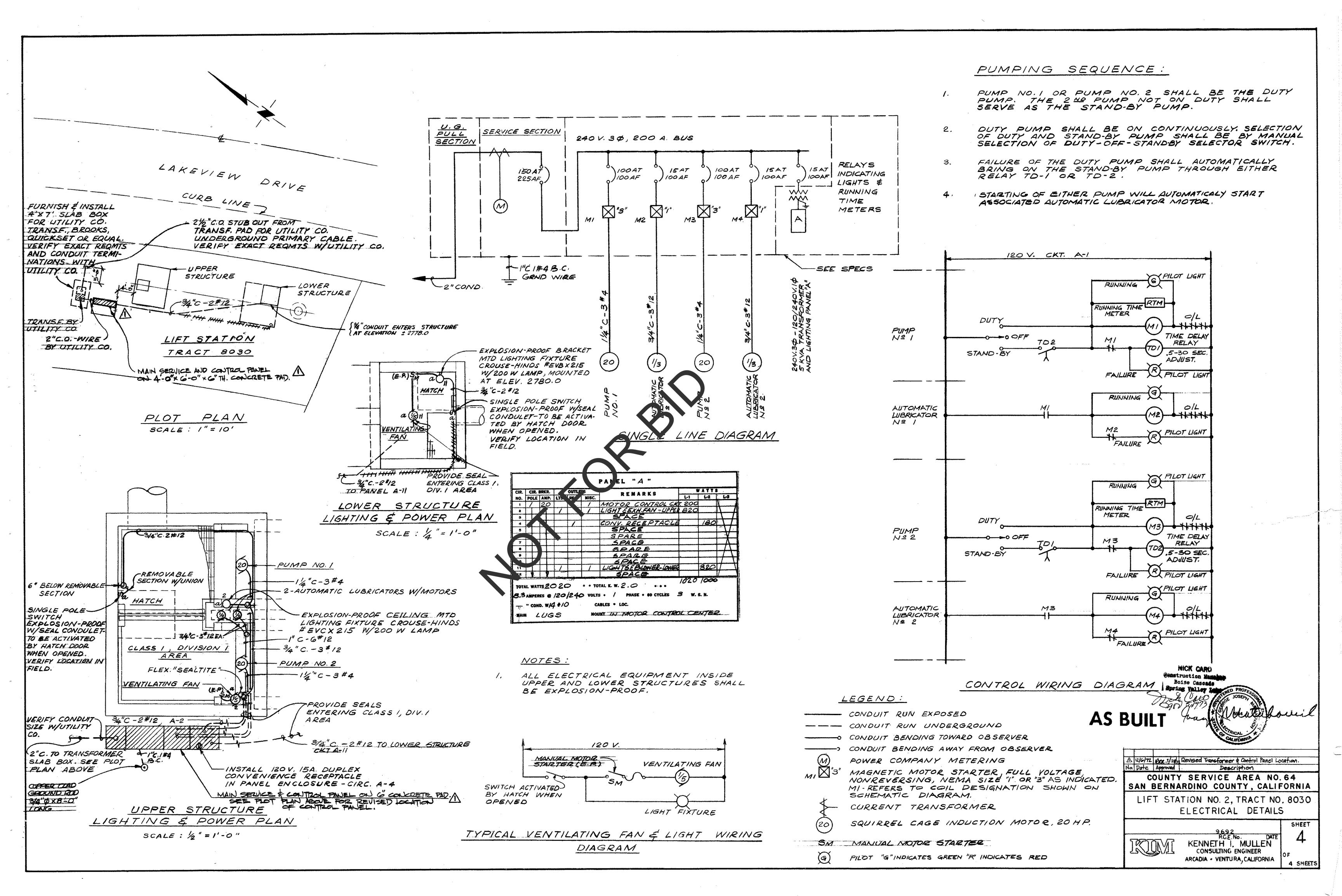
SCALE











WEBB ENGINEERING SEWER FEASIBILITY STUDIES





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Mr. Nelson Sarti, P.E. Project Manager

SAN BERNARDINO COUNTY
DEPARTMENT OF PUBLIC WORKS, SPECIAL DIS JUICTS

PO Box 11969

San Bernardino, CA 92423

RE: Sewer Study Letter for APN 0482-0 1-71,-02 07, and -08, located on the southwest corner of the interaction of Pahute Avenue and Tamarisk Road.

Dear Mr. Sarti:

Pursuant to the District's request, we have perfected as wer study for the above referenced project that includes a review of how the proposed project affects the District's existing facilities and identifies any required system improvements. The proposed project consists of 39 residential lots and is located west of Tamarisk Roll and buth of Pahute Avenue in the City of Victorville, as shown in **Figure 1**. The proposed recility of requesting sewer service from County Service Area 64.

Project Wastewater Generation

The project's wastewater fow generation is calculated using comparable wastewater generation factors. A sewer generation falue of 100 gpd/capita was used in the calculation of the proposed project's sewer generation, a specified in the County of San Bernardino Special Districts Department Standards in Sanitary Sewer (November 13, 2012). According to the U.S. Census Bureau data fro 12015-20 9, there is a density of 3.60 people per household in the city of Victorville. Therefore, approsed Gavelopment's calculated sewer generation is calculated below.

Total: Jouseholds = 39 homes
Populatic Density = 3.60 people/home
Sewer Generation Rate = 100 gpd/person

$$Sewer \, Generation_{Average} = (39 \, homes) \times \left(3.60 \frac{people}{home}\right) \times \left(100 \frac{gpd}{person}\right) = \, 14,040 \, gpd$$

The estimated peak flow for this project is 28,080 gpd or 19.5 gpm based on the peaking factor consistent with the sewer monitoring study.

WEBB W.O.: 2021-0127

Mr. Nelson Sarti, P.E.
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Project Connection Point

In preparation of this sewer study, we have reviewed available data related to the proposed sewer connection point. Available data includes District atlas maps and a hydrollic. Ode' of the existing CSA 64 sewer system created by WEBB to perform previous studion. There are existing 8-in sewers in Tamarisk Road and Pahute Avenue fronting the project boundaries and a likely sewer plan has been developed yet. A likely sewer connection point to the existing SA 6 sewer system would be located either on Tamarisk Road or Pahute Avenue in the public right-of way.

Hydraulic Model Information

A hydraulic model of the CSA 64 Easterly Sewer 'vicem was created by WEBB to perform a previous sewer study dated October 20, 2011. III. 121, 17EBB was asked to perform two additional sewer studies, and in each study, WEBB recommended flow monitoring in the CSA 64 easterly system to verify if current field sewer flow match WilbB's 2017 hydraulic model flows. Field flow monitoring was conducted from Julie to Julie 1021 and the hydraulic model was re-calibrated in August 2021 after the field flow numitoring. The re-calibrated CSA 64 sewer model is used for this analysis. Please see the attached and resist for the re-calibrating the sewer model using the field flow monitoring.

Analysis Load Input

To include the project flows in the system's existing model, an average daily sewer load of 0.0140 MGD, a calculated above, was applied to the 8-inch diameter sewer line in Tamarisk Road In addition, the following model point wastewater loads summarized in **Table 1** were added to account for other development in the CSA 64 system. **Figure 1** show a map of the proposed project location, point load locations, and the system's model rewer to lies.

Table 1

Point Load Source	Sewer Flow (gpd) – Average Flow	Sewer Flow (gpm) – Peak Flow
Victor Valley College (w/o Stadium) (1)	147,000	204.17
Lakeview Middle School ⁽¹⁾	8,820	12.25
Endeavour School of Exploration ⁽¹⁾	8,330	11.57
Excelsior Charter School ⁽¹⁾	14,700	20.42
APN 0482-043-08 ⁽³⁾	4,950	6.88
Victor Valley College Stadium ⁽²⁾	N/A	47.22

¹ From the CSA 64 Easterly Sewer System Modeling/Hesperia Sewer Study (10/20/2017)

² From the CSA 64 Victor Valley College – New Stadium and Education Event Center Sewer Study (04/09/2021)

³ From the CSA 64 APN 0482-043-08 Sewer Study (11/29/2021)

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Analysis Results

The model's sewer pipelines from the proposed development location to the down stream Lakeview Lift Station were analyzed for high depth-over-Diameter (d/D) refios. The down stream pipelines with the highest d/D ratios are listed in **Table 2** below.

Table 2

Pipe ID	Diameter (in)	Total Flow (mgd)	d/D
12-8030-11	21	. 338	0.509
14-8030-13	21	.207	0.492
13-8030-12	21	2.211	0.490
15-8030-14	21	2.17	0.455
1-8099-6	15	.935	0.455
6-8099-5	15	0.935	0.455

The peak flow at the Lakeview Lift Struon is estimated at 2.34 MGD or 3.62 cfs. The model analysis concludes that downstream sewer piper generates from the proposed development experience flow d/D ratios that are below the maximum. O standards, with the model inputs summarized in this letter. The District can consider accepting sewage flows from these proposed projects without upgrading the existing cancel in system pipelines, however the Lakeview Lift Station should be reviewed in detail for its capability for this increased flowrate.

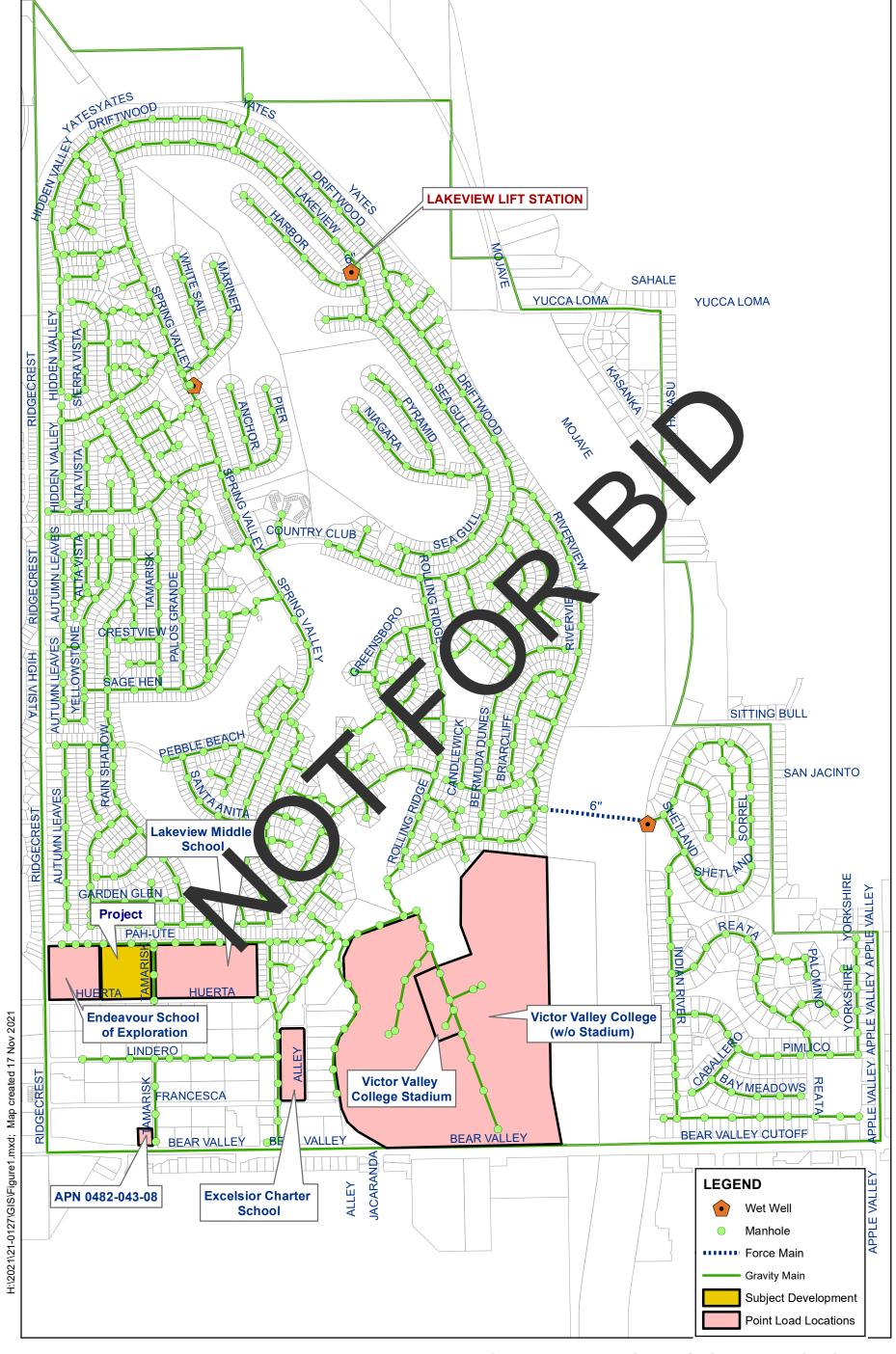
NO. C65862

Should you have ny questions, please call me.

ALBERT A. WL B A COLATES

Bradley Sackett, Webb Associates Senior Engineer

cc: Gustavo Gomez, Webb Associates
Bruce Davis, Webb Associates



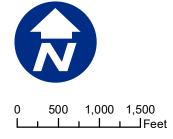


FIGURE 1 - EXISTING SEWER SYSTEM CSA 64

