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Palm Desert, CA 92211
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Project
Sheriff's Crime Lab - Remodel

Section
Structural Calculations

Job Ref.
J2300064

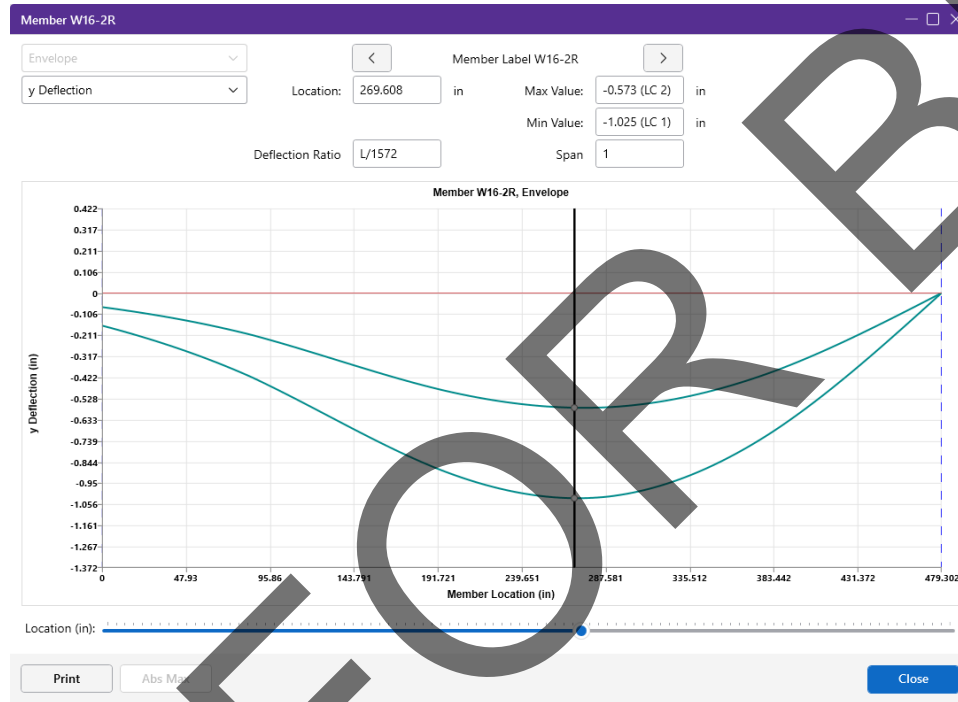
Calc. by
CJS

Sheet no./rev.
76

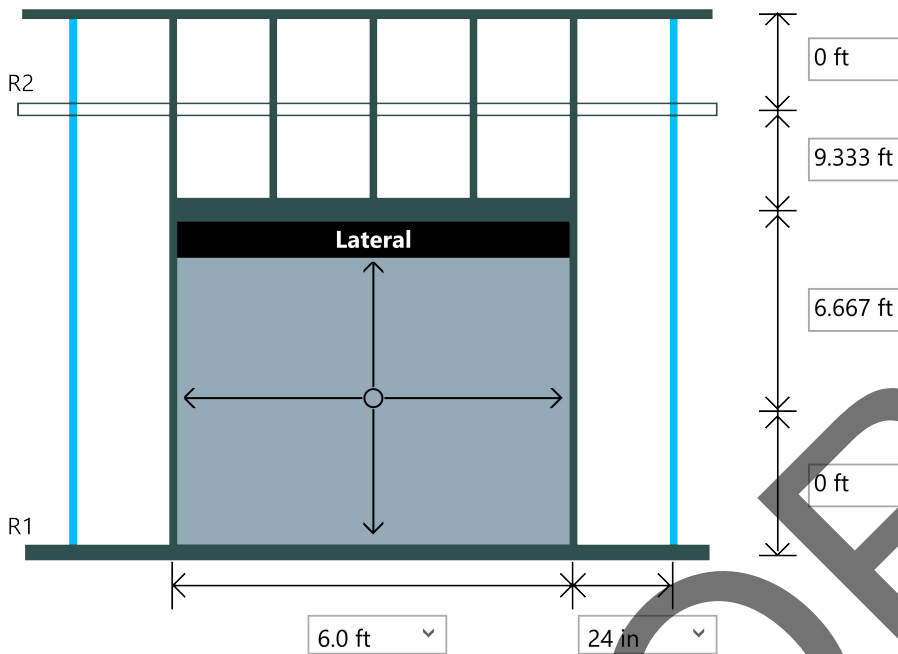
Date
06/14/2024

DEFLECTION CHECK

From RISA-3D analysis, the maximum deflection is in member W16-2R (existing W16X31):



Max. deflection 1.025 in
Span length (excluding cantilever) 32'-9"
Deflection ratio L/383 > L/240 OK



Design Loads

Wall Lateral Pressure :	5 psf
Parapet Lateral Pressure :	
RO Lateral Pressure :	4-Ways
Lateral element force multiplier	1.0
Strength :	0.7
Deflection :	
Header :	Single Member
Gravity Load at Header:	5 psf

Lateral Pressure to: 4-Ways

Brace Settings

Component(s)	Members(s)	Flexural Bracing	Axial KyLy	Axial KtLt	Distortional K-Phi(lb-in/in)	Distortional Lm	Interconnection Spacing
Wall Studs	400S300-54(50), Single@24 in o/c	None	None	None	0	None	N/A
Jamb Studs	400S350-54(50), Single	None	None	None	0	None	N/A
Vertical Header	400S162-54(50), Y-Y Axis	Full	N/A	N/A	0	None	N/A
Lateral Header	400S162-54(50), Single	Full	N/A	N/A	0	None	N/A

Analysis Results

Component(s)	Members(s)	Axial Load (lb)	Max KL/r	Max. Moment (ft-lb)	Max. Shear (lb)	Bottom Reaction (lb)	Top or End Reaction (lb)
Wall Studs	400S300-54(50), Single@24 in o/c	160.0	172	320.0	80.0	80.0	80.0
Jamb Studs	400S350-54(50), Single	253.3	143	622.2	137.5	160.0	90.0
Vertical Header	400S162-54(50), Y-Y Axis	N/A	N/A	210.0	140.0	N/A	140.0
Lateral Header	400S162-54(50), Single	N/A	N/A	150.0	92.5	N/A	92.5

Design Results

Component(s)	Members(s)	Span	Deflection Parapet	A + M Interaction	V + M Interaction	Web Stiffeners	Design OK
Wall Studs	400S300-54(50), Single@24 in o/c	L/898	L/0	0.608	0.02	NA	Yes
Jamb Studs	400S350-54(50), Single	L/610	L/0	0.69	0.34	NA	Yes
Vertical Header	400S162-54(50), Y-Y Axis	L/203	NA	0.65	0.05	No	Yes
Lateral Header	400S162-54(50), Single	L/3468	NA	0.12	0.03	No	Yes
Combined Header				0.77	0		

Simpson Strong-Tie® Connectors @ Studs

Support	Rx(lb)	Ry(lb)	Simpson Strong-Tie® Connector	Connector Interaction	Anchor Interaction
R2	80.00	0.00	400SLT250-33 (33) & (1) .157" SST PDPA/PDPAT-62KP to	53.33 %	48.88 %

steel (3/16" to 1/2" thickness)

R1	80.00	160.00	400T125-33 (33) & (1) 1/4" x 1-3/4" embed Titen2 to 2500 min concrete	11.63 %	30.02 %
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* Reference catalog for connector and anchor requirement notes as well as screw placements requirement

Simpson Strong-Tie® Connectors @ Jambs

Support	Rx(lb)	Ry(lb)	Simpson Strong-Tie® Connector	Connector Interaction	Anchor Interaction
R2	90.00	0.00	400SLT250-33 (33) & (2) #10 screw to CFS (18ga 33ksi)	60.00 %	37.82 %
R1	160.00	253.33	400T150-33 (33) & (1) 1/4" x 1-3/4" embed Titen2 to 2500 min concrete	46.51 %	60.04 %

* Reference catalog for connector and anchor requirement notes as well as screw placements requirement

Simpson Strong-Tie® Wall Stud Bridging Connectors @ Studs

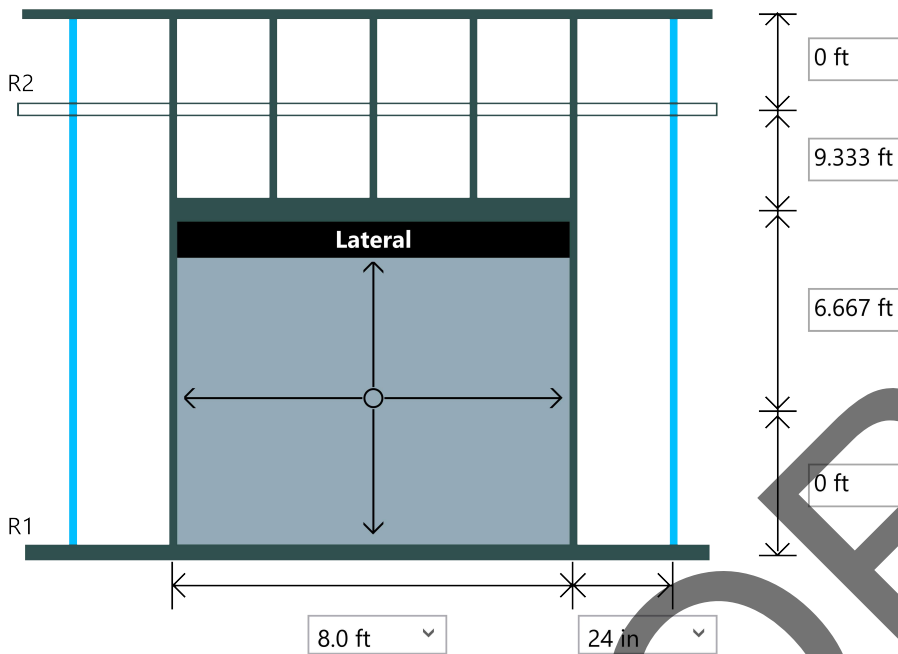
Span/Parapet	Bracing Length(in.)	Design Number of Braces	Pn(lb.)	LSUBH (Min) ¹	LSUBH (Max) ¹	SUBH (Min) ¹	SUBH (Max) ¹	MSUBH (Min) ¹	MSUBH (Max) ¹
Span	Span	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

Simpson Strong-Tie® Wall Stud Bridging Connectors @ Jambs

Span/Parapet	Bracing Length(in.)	Design Number of Braces	Pn(lb.)	LSUBH (Min) ¹	LSUBH (Max) ¹	SUBH (Min) ¹	SUBH (Max) ¹	MSUBH (Min) ¹	MSUBH (Max) ¹
Span	Span	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

Notes:

- 1) Values in parentheses are stress ratios.
- 2) Bridging connectors are not designed for back-back, box, or built-up sections.
- 3) Reference www.strongtie.com for latest load data, important information, and general notes.
- 4) CFS Designer will not select bridging connectors unless all flexural and axial bracing settings are the same.
- 5) If the bracing length is larger than the span length, bridging connectors are not designed.



Design Loads

Wall Lateral Pressure :	5 psf
Parapet Lateral Pressure :	
RO Lateral Pressure :	4-Ways
Lateral element force multiplier	1.0
Strength :	0.7
Deflection :	
Header :	Single Member
Gravity Load at Header:	5 psf

Lateral Pressure to: 4-Ways

Brace Settings

Component(s)	Members(s)	Flexural Bracing	Axial KyLy	Axial KtLt	Distortional K-Phi(lb-in/in)	Distortional Lm	Interconnection Spacing
Wall Studs	600S162-33(33), Single@24 in o/c	Mid-Pt	Mid-Pt	Mid-Pt	0	None	N/A
Jamb Studs	600S162-54(50), Single	Mid-Pt	Mid-Pt	Mid-Pt	0	None	N/A
Vertical Header	600S200-54(50), Y-Y Axis	Full	N/A	N/A	0	None	N/A
Lateral Header	600S200-54(50), Single	Full	N/A	N/A	0	None	N/A

Analysis Results

Component(s)	Members(s)	Axial Load (lb)	Max KL/r	Max. Moment (ft-lb)	Max. Shear (lb)	Bottom Reaction (lb)	Top or End Reaction (lb)
Wall Studs	600S162-33(33), Single@24 in o/c	160.0	165	320.0	80.0	80.0	80.0
Jamb Studs	600S162-54(50), Single	300.0	169	777.8	161.1	200.0	106.7
Vertical Header	600S200-54(50), Y-Y Axis	N/A	N/A	373.3	186.7	N/A	186.7
Lateral Header	600S200-54(50), Single	N/A	N/A	289.1	132.2	N/A	132.2

Design Results

Component(s)	Members(s)	Span	Deflection Parapet	A + M Interaction	V + M Interaction	Web Stiffeners	Design OK
Wall Studs	600S162-33(33), Single@24 in o/c	L/984	L/0	0.638	0.13	NA	Yes
Jamb Studs	600S162-54(50), Single	L/750	L/0	0.84	0.31	NA	Yes
Vertical Header	600S200-54(50), Y-Y Axis	L/160	NA	0.75	0.05	No	Yes
Lateral Header	600S200-54(50), Single	L/4083	NA	0.13	0.05	No	Yes
Combined Header				0.87	0		

Simpson Strong-Tie® Connectors @ Studs

Support	Rx(lb)	Ry(lb)	Simpson Strong-Tie® Connector	Connector Interaction	Anchor Interaction
R2	80.00	0.00	600SLT250-33 (33) & (2) #10 screw to CFS (18ga 33ksi)	53.33 %	33.62 %

R1	80.00	160.00	600T150-33 (33) & (1) 1/4" x 1 5/8" embed Titen HD to 2500 psi min concrete	30.34 %	23.88 %
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* Reference catalog for connector and anchor requirement notes as well as screw placements requirement

Simpson Strong-Tie® Connectors @ Jambs

Support	Rx(lb)	Ry(lb)	Simpson Strong-Tie® Connector	Connector Interaction	Anchor Interaction
R2	106.67	0.00	600SLT250-33 (33) & (2) #10 screw to CFS (18ga 33ksi)	71.11 %	44.82 %
R1	200.00	300.00	600T125-33 (33) & (1) 1/4" x 1-3/4" embed Titen2 to 2500 min concrete	58.14 %	75.05 %

* Reference catalog for connector and anchor requirement notes as well as screw placements requirement

Simpson Strong-Tie® Wall Stud Bridging Connectors @ Studs

Span/Parapet	Bracing Length(in.)	Design Number of Braces	Pn(lb.)	LSUBH (Min) ¹	LSUBH (Max) ¹	SUBH (Min) ¹	SUBH (Max) ¹	MSUBH (Min) ¹	MSUBH (Max) ¹
Span	96	1	2036.2	OK (0.37)	OK (0.26)	OK (0.31)	OK (0.23)	No Soln	No Soln

Simpson Strong-Tie® Wall Stud Bridging Connectors @ Jambs

Span/Parapet	Bracing Length(in.)	Design Number of Braces	Pn(lb.)	LSUBH (Min) ¹	LSUBH (Max) ¹	SUBH (Min) ¹	SUBH (Max) ¹	MSUBH (Min) ¹	MSUBH (Max) ¹
Span	96	1	3714.3	OK (0.49)	OK (0.35)	OK (0.39)	OK (0.25)	OK (0.24)	OK (0.16)

Notes:

- 1) Values in parentheses are stress ratios.
- 2) Bridging connectors are not designed for back-back, box, or built-up sections.
- 3) Reference www.strongtie.com for latest load data, important information, and general notes.
- 4) CFS Designer will not select bridging connectors unless all flexural and axial bracing settings are the same.
- 5) If the bracing length is larger than the span length, bridging connectors are not designed.



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J2300064

Calc. by
CJS

Sheet no./rev.
81

Date
06/14/2024

CHECK HEADER CONNECTION

Connector

Simpson Rigid Connector Angle RCA223/68

RCA Rigid Connector Angles Allowable Loads (lb.)

Model	No. of #10 Screws ^{5,6}	Screw Pattern	Stud Framing Thickness ¹¹								
			33 mil (20 ga.)			43 mil (18 ga.)			54 mil (16 ga.)		
			F ₂	F ₃	F ₄	F ₂	F ₃	F ₄	F ₂	F ₃	F ₄
RCA223/54	3	3A	205	495	200	205	590	310	205	590	620
	4	3B	205	580	390	205	580	605	205	580	1,095
	6	3C	205	865	480	205	865	740	205	865	1,095
RCA223/68	3	3A	310	495	200	310	765	310	310	815	620
	4	3B	310	660	390	310	805	605	310	805	1,210
	6	3C	310	990	480	310	1,205	740	310	1,205	1,350

Simpson RCA223/68 shear capacity (F₂)

310 lbs

Shear demand:

Height of CFS wall above header

9'-4"

Max. width of header

8'-0"

Weight of CFS wall + finishes

5 psf

Max. reaction at header support

$= 5 \times 8 \times 9.33 / 2 = 187 \text{ lbs}$ < 310 lbs

PASS – Shear demand is less than shear capacity.

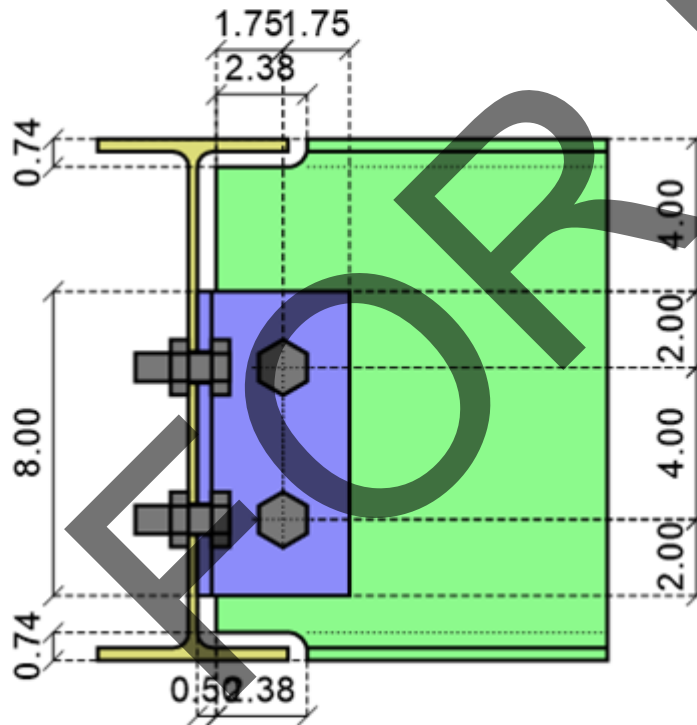
DESIGN OF CONNECTIONS

NOT FOR BID

J32 I - J18: 2D Views Report

Side view

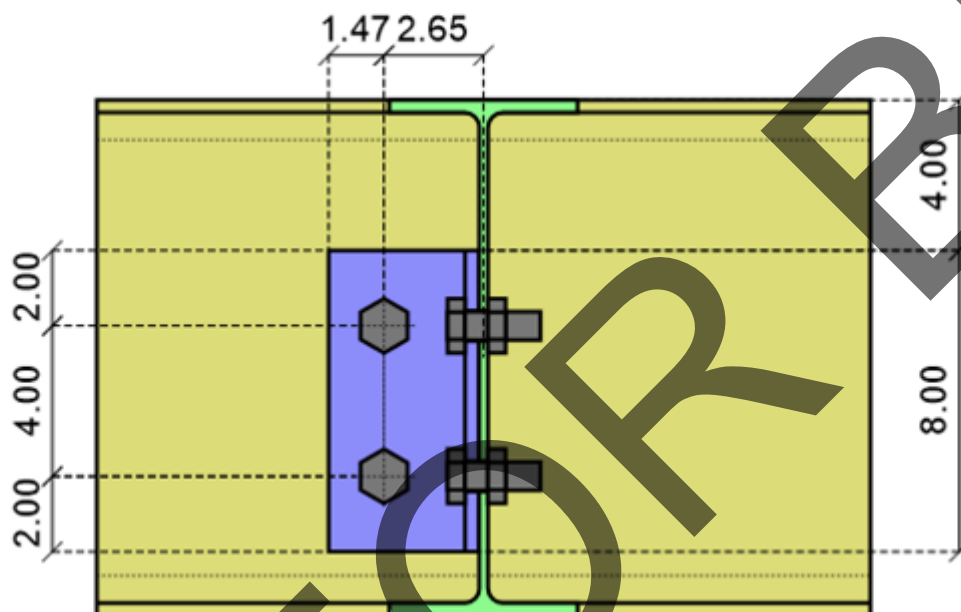
Girder/Beam Single Angle (One Side) Shear Connection



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J32 I - J18: 2D Views Report (continued):

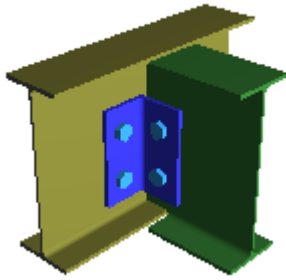
Front view



J32 I - J18: ASD Results Report

ASD

Girder/Beam Single Angle (One Side) Shear Connection



Material Properties:				
Girder	W14X22	A992	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi
Beam	W14X22	A992	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi
Angle	L4X4X6	A572 Gr.50	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi

Input Data:		
Shear Load	-900.99 lbs	User Input Shear Load
Axial Load	-608.05 lbs	User Input Axial Force (tension)

Governing LC: 3D - 12 - LC 12: IBC 21/ASCE Strength 7 (a)

Note: Unless specified, all code references are from AISC 360-10

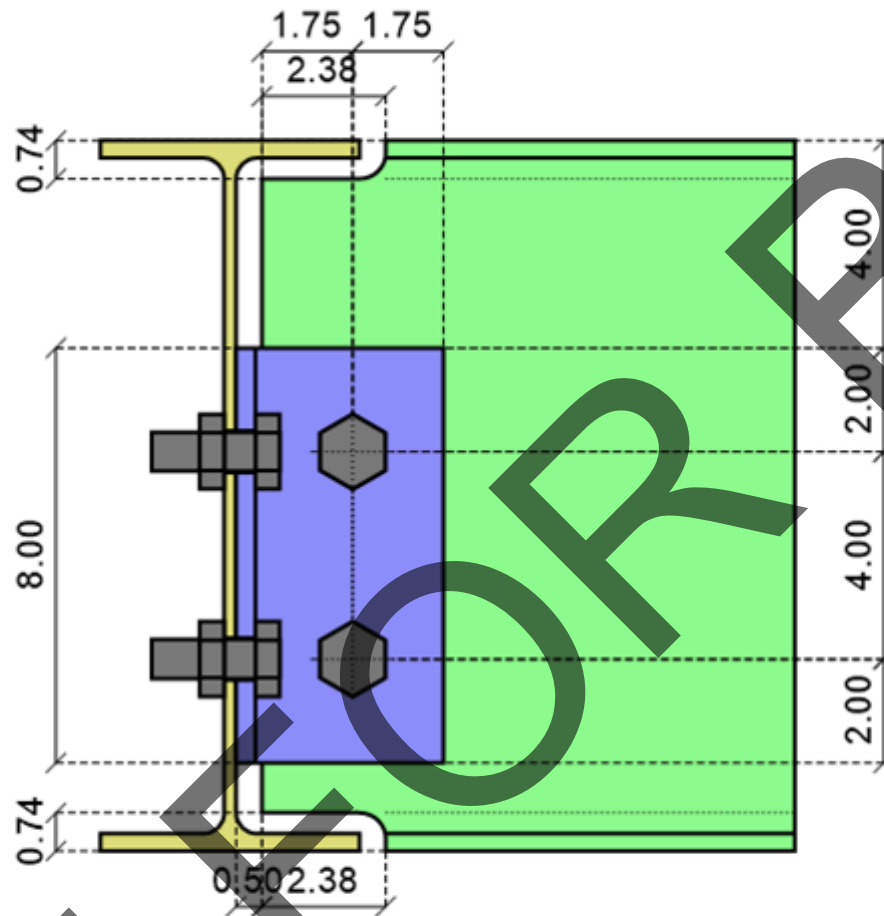
Limit State	Required	Available	Unity Check	Result
Geometry Restrictions at Beam				PASS
Geometry Restrictions at Girder				PASS
Erection Stability				PASS
Beam Shear Yield	900.99 lbs	56258.00 lbs	0.02	PASS
Clip Angle Shear Yield	900.99 lbs	60000.00 lbs	0.02	PASS
Beam Shear Rupture	900.99 lbs	47002.80 lbs	0.02	PASS
Clip Angle Shear Rupture at Beam	900.99 lbs	45703.12 lbs	0.02	PASS
Clip Angle Shear Rupture at Girder	900.99 lbs	45703.12 lbs	0.02	PASS
Beam Axial Yield	608.05 lbs	84218.56 lbs	0.01	PASS
Clip Angle Axial Yield	608.05 lbs	89820.36 lbs	0.01	PASS
Beam Tension Rupture	608.05 lbs	78338.00 lbs	0.01	PASS
Clip Angle Tension Rupture at Beam	608.05 lbs	76171.88 lbs	0.01	PASS
Clip Angle Block Shear at Girder	900.99 lbs	46303.12 lbs	0.02	PASS
Beam Block Shear	900.99 lbs	33840.19 lbs	0.03	PASS
Clip Angle Block Shear at Beam	900.99 lbs	49746.09 lbs	0.02	PASS
Beam Tearout	608.05 lbs	35132.50 lbs	0.02	PASS
Clip Angle Tearout	608.05 lbs	57281.25 lbs	0.01	PASS
Coped Beam Flexural Rupture	900.99 lbs	64590.26 lbs	0.01	PASS
Coped Beam Lateral Torsional Buckling	900.99 lbs	59502.77 lbs	0.02	PASS
Clip Angle Flexural Yield			0.00	PASS
Clip Angle Flexural Rupture			0.00	PASS
Bolt Bearing on Girder	900.99 lbs	23856.47 lbs	0.04	PASS
Bolt Bearing on Clip Angle at Girder	900.99 lbs	23856.47 lbs	0.04	PASS
Bolt Bearing on Beam	1086.97 lbs	23856.47 lbs	0.05	PASS
Bolt Bearing on Clip Angle at Beam	1086.97 lbs	23856.47 lbs	0.05	PASS
Bolt Shear at Girder	900.99 lbs	16465.74 lbs	0.05	PASS
Bolt Group Eccentricity at Girder		0.69		

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J32 I - J18: ASD Results Report (continued):

Limit State	Required	Available	Unity Check	Result
Bolt Shear at Beam	1086.97 lbs	18201.29 lbs	0.06	PASS
Bolt Group Eccentricity at Beam		0.76		
Angle Leg Bending	0.13 kips-ft	0.91 kips-ft	0.15	PASS
Bolt Prying				No Prying
Bolt Tension at Girder				N/A

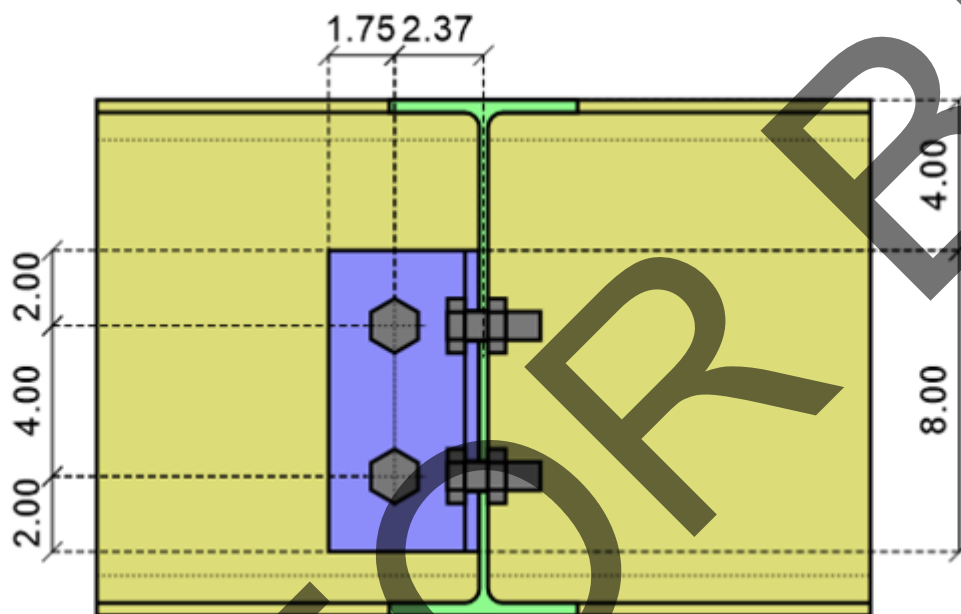
Side view



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J32 J - J19: 2D Views Report (continued):

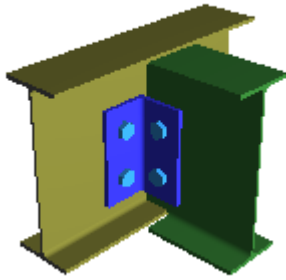
Front view



J32 J - J19: ASD Results Report

ASD

Girder/Beam Single Angle (One Side) Shear Connection



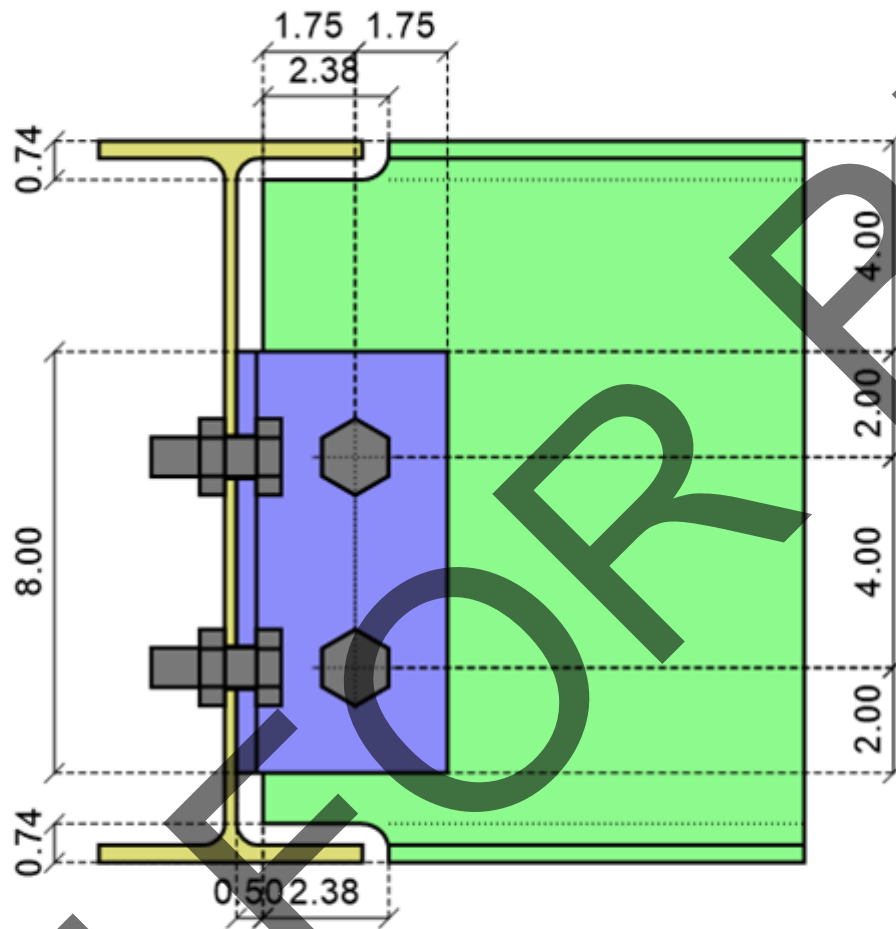
Material Properties:				
Girder	W14X22	A992	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi
Beam	W14X22	A992	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi
Angle	L4X4X6	A572 Gr.50	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi
Input Data:				
Shear Load	12158.16 lbs	User Input Shear Load		
Axial Load	-48.10 lbs	User Input Axial Force (tension)		

Governing LC: 3D - 6 - LC 6: IBC 21/ASCE ASD 3 (a)

Note: Unless specified, all code references are from AISC 360-10

Limit State	Required	Available	Unity Check	Result
Geometry Restrictions at Beam				PASS
Geometry Restrictions at Girder				PASS
Erection Stability				PASS
Beam Shear Yield	12158.16 lbs	56258.00 lbs	0.22	PASS
Clip Angle Shear Yield	12158.16 lbs	60000.00 lbs	0.20	PASS
Beam Shear Rupture	12158.16 lbs	47002.80 lbs	0.26	PASS
Clip Angle Shear Rupture at Beam	12158.16 lbs	45703.12 lbs	0.27	PASS
Clip Angle Shear Rupture at Girder	12158.16 lbs	45703.12 lbs	0.27	PASS
Clip Angle Block Shear at Girder	12158.16 lbs	49746.09 lbs	0.24	PASS
Beam Block Shear	12158.16 lbs	41775.19 lbs	0.29	PASS
Clip Angle Block Shear at Beam	12158.16 lbs	49746.09 lbs	0.24	PASS
Coped Beam Flexural Rupture	12158.16 lbs	64590.26 lbs	0.19	PASS
Coped Beam Lateral Torsional Buckling	12158.16 lbs	59502.77 lbs	0.20	PASS
Bolt Bearing on Girder	12158.16 lbs	23856.47 lbs	0.51	PASS
Bolt Bearing on Clip Angle at Girder	12158.16 lbs	23856.47 lbs	0.51	PASS
Bolt Bearing on Beam	12158.16 lbs	23856.47 lbs	0.51	PASS
Bolt Bearing on Clip Angle at Beam	12158.16 lbs	23856.47 lbs	0.51	PASS
Bolt Shear at Girder	12158.16 lbs	15122.62 lbs	0.80	PASS
Bolt Group Eccentricity at Girder		0.63		
Bolt Shear at Beam	12158.16 lbs	15555.61 lbs	0.78	PASS
Bolt Group Eccentricity at Beam		0.65		
Bolt Prying				Prying
Bolt Tension at Girder				N/A

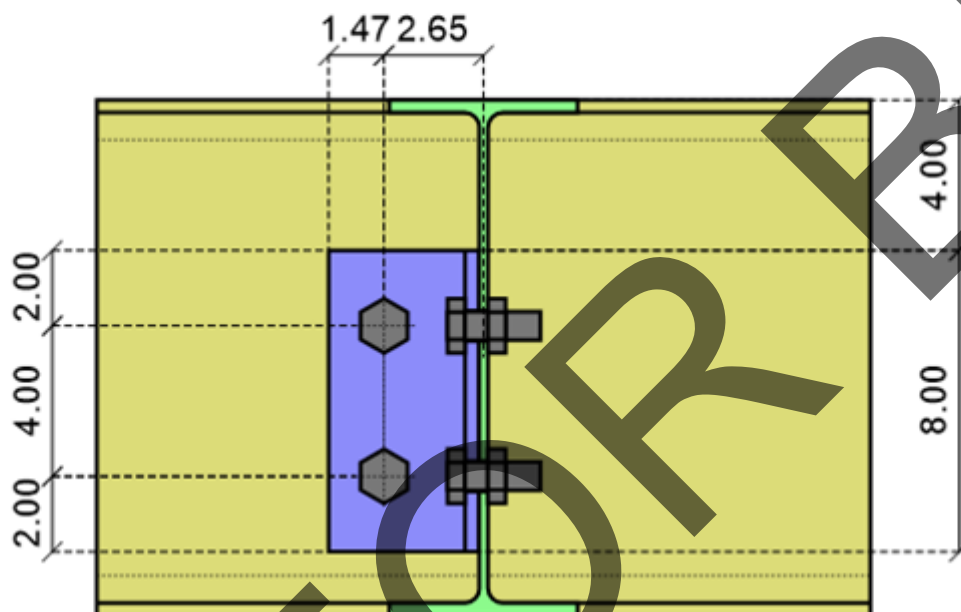
Side view



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J33 I - J14: 2D Views Report (continued):

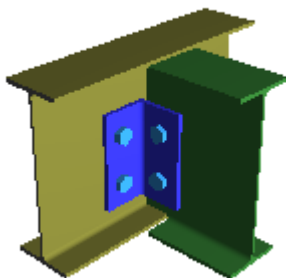
Front view



J33 I - J14: ASD Results Report

ASD

Girder/Beam Single Angle (One Side) Shear Connection



Material Properties:

Girder	W14X22	A992	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi
Beam	W14X22	A992	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi
Angle	L4X4X6	A572 Gr.50	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi

Input Data:

Shear Load	-87.98 lbs	User Input Shear Load
Axial Load	-796.86 lbs	User Input Axial Force (tension)

Governing LC: 3D - 12 - LC 12: IBC 21/ASCE Strength 7 (a)

Note: Unless specified, all code references are from AISC 360-10

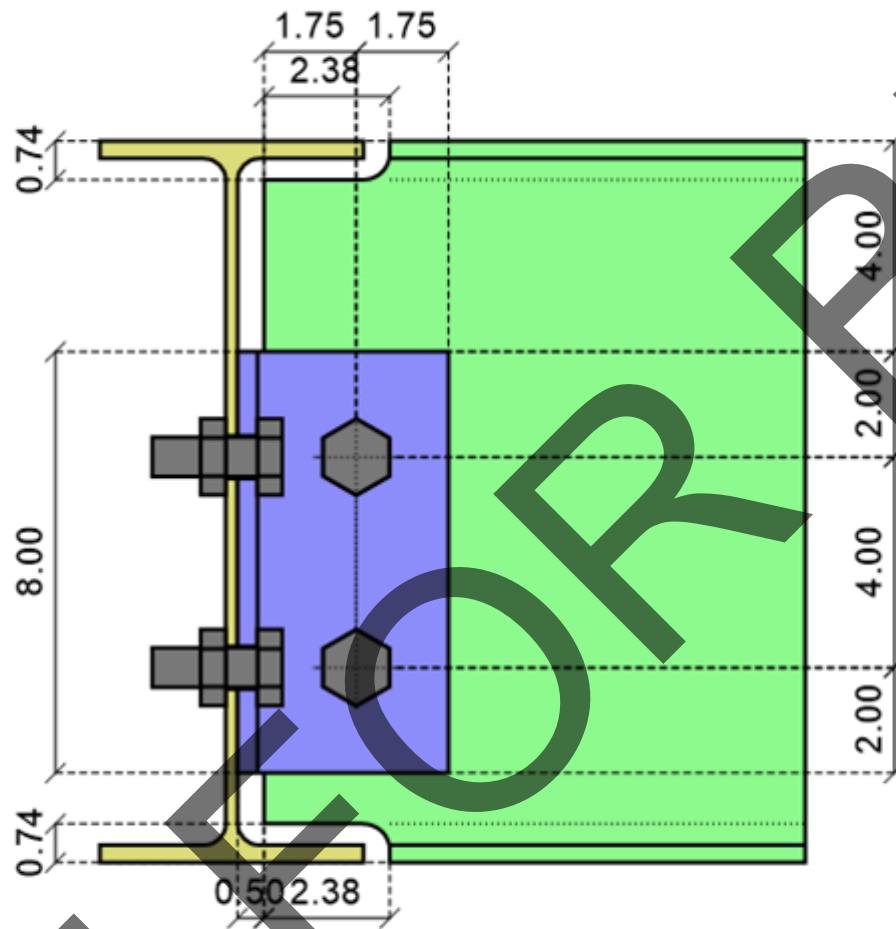
Limit State	Required	Available	Unity Check	Result
Geometry Restrictions at Beam				PASS
Geometry Restrictions at Girder				PASS
Erection Stability				PASS
Beam Shear Yield	87.98 lbs	56258.00 lbs	0.00	PASS
Clip Angle Shear Yield	87.98 lbs	60000.00 lbs	0.00	PASS
Beam Shear Rupture	87.98 lbs	47002.80 lbs	0.00	PASS
Clip Angle Shear Rupture at Beam	87.98 lbs	45703.12 lbs	0.00	PASS
Clip Angle Shear Rupture at Girder	87.98 lbs	45703.12 lbs	0.00	PASS
Beam Axial Yield	796.86 lbs	84218.56 lbs	0.01	PASS
Clip Angle Axial Yield	796.86 lbs	89820.36 lbs	0.01	PASS
Beam Tension Rupture	796.86 lbs	78338.00 lbs	0.01	PASS
Clip Angle Tension Rupture at Beam	796.86 lbs	76171.88 lbs	0.01	PASS
Clip Angle Block Shear at Girder	87.98 lbs	46303.12 lbs	0.00	PASS
Beam Block Shear	87.98 lbs	33840.19 lbs	0.00	PASS
Clip Angle Block Shear at Beam	87.98 lbs	49746.09 lbs	0.00	PASS
Beam Tearout	796.86 lbs	35132.50 lbs	0.02	PASS
Clip Angle Tearout	796.86 lbs	57281.25 lbs	0.01	PASS
Coped Beam Flexural Rupture	87.98 lbs	64590.26 lbs	0.00	PASS
Coped Beam Lateral Torsional Buckling	87.98 lbs	59502.77 lbs	0.00	PASS
Clip Angle Flexural Yield			0.00	PASS
Clip Angle Flexural Rupture			0.00	PASS
Bolt Bearing on Girder	87.98 lbs	23856.47 lbs	0.00	PASS
Bolt Bearing on Clip Angle at Girder	87.98 lbs	23856.47 lbs	0.00	PASS
Bolt Bearing on Beam	801.70 lbs	23856.47 lbs	0.03	PASS
Bolt Bearing on Clip Angle at Beam	801.70 lbs	23856.47 lbs	0.03	PASS
Bolt Shear at Girder	87.98 lbs	16484.82 lbs	0.01	PASS
Bolt Group Eccentricity at Girder		0.69		

continued on next page...

J33 I - J14: ASD Results Report (continued):

Limit State	Required	Available	Unity Check	Result
Bolt Shear at Beam	801.70 lbs	16230.75 lbs	0.05	PASS
Bolt Group Eccentricity at Beam		0.68		
Angle Leg Bending	0.18 kips-ft	0.91 kips-ft	0.19	PASS
Bolt Prying				No Prying
Bolt Tension at Girder				N/A

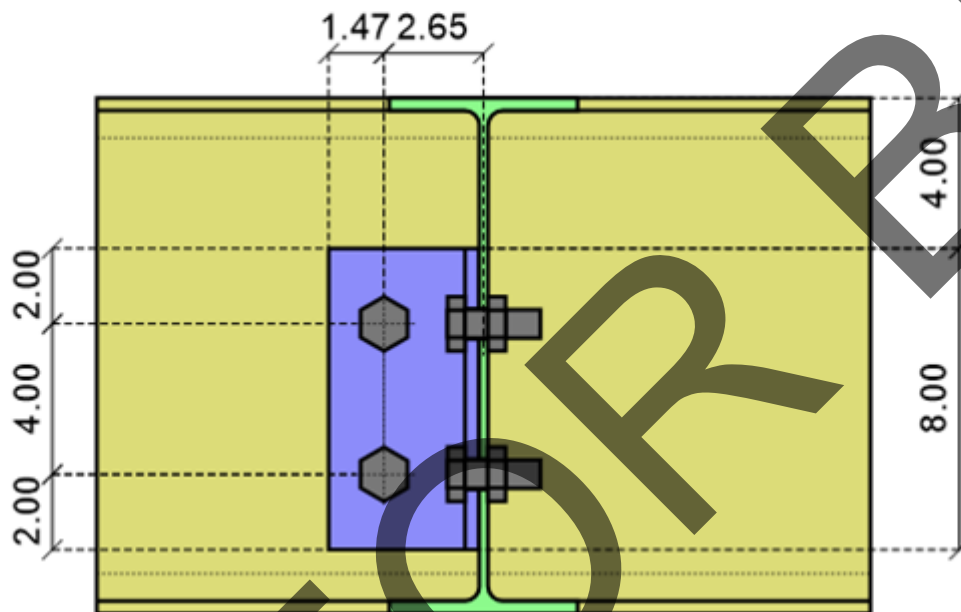
Side view



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J33 J - J15: 2D Views Report (continued):

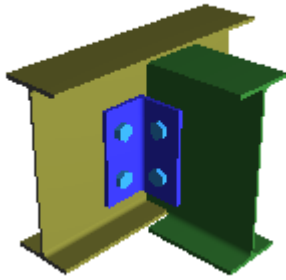
Front view



J33 J - J15: ASD Results Report

ASD

Girder/Beam Single Angle (One Side) Shear Connection



Material Properties:				
Girder	W14X22	A992	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi
Beam	W14X22	A992	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi
Angle	L4X4X6	A572 Gr.50	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi

Input Data:		
Shear Load	83.05 lbs	User Input Shear Load
Axial Load	-796.88 lbs	User Input Axial Force (tension)

Governing LC: 3D - 12 - LC 12: IBC 21/ASCE Strength 7 (a)

Note: Unless specified, all code references are from AISC 360-10

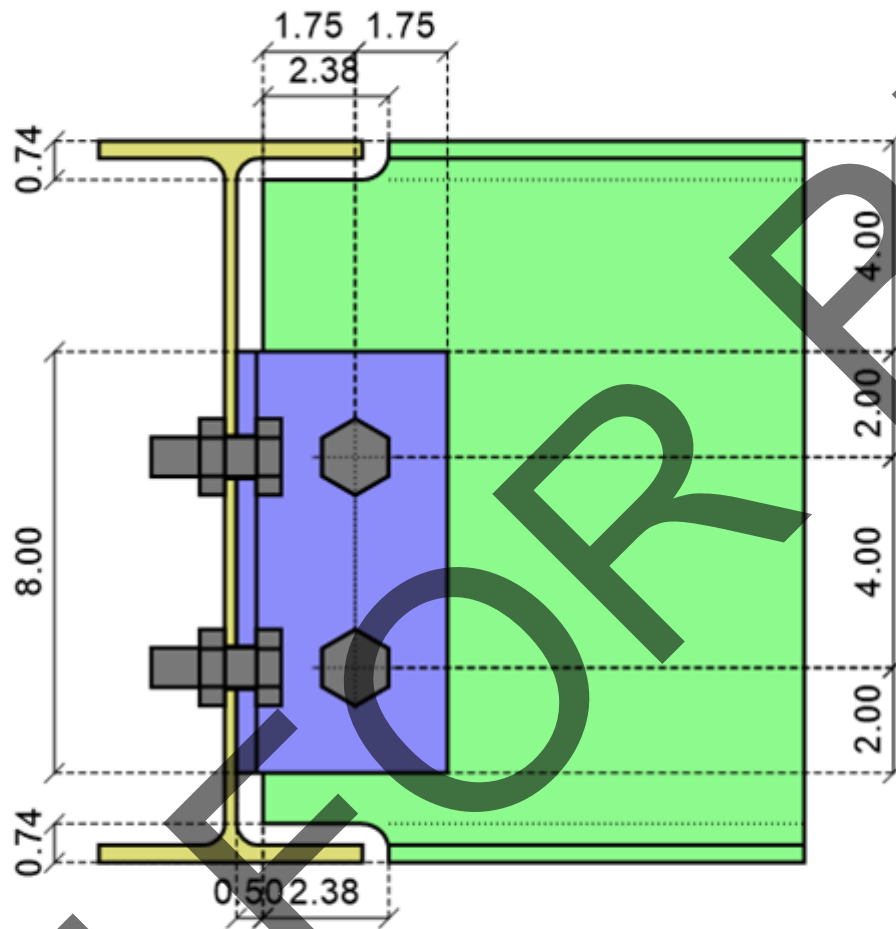
Limit State	Required	Available	Unity Check	Result
Geometry Restrictions at Beam				PASS
Geometry Restrictions at Girder				PASS
Erection Stability				PASS
Beam Shear Yield	83.05 lbs	56258.00 lbs	0.00	PASS
Clip Angle Shear Yield	83.05 lbs	60000.00 lbs	0.00	PASS
Beam Shear Rupture	83.05 lbs	47002.80 lbs	0.00	PASS
Clip Angle Shear Rupture at Beam	83.05 lbs	45703.12 lbs	0.00	PASS
Clip Angle Shear Rupture at Girder	83.05 lbs	45703.12 lbs	0.00	PASS
Beam Axial Yield	796.88 lbs	84218.56 lbs	0.01	PASS
Clip Angle Axial Yield	796.88 lbs	89820.36 lbs	0.01	PASS
Beam Tension Rupture	796.88 lbs	78338.00 lbs	0.01	PASS
Clip Angle Tension Rupture at Beam	796.88 lbs	76171.88 lbs	0.01	PASS
Clip Angle Block Shear at Girder	83.05 lbs	46303.12 lbs	0.00	PASS
Beam Block Shear	83.05 lbs	41775.19 lbs	0.00	PASS
Clip Angle Block Shear at Beam	83.05 lbs	49746.09 lbs	0.00	PASS
Beam Tearout	796.88 lbs	35132.50 lbs	0.02	PASS
Clip Angle Tearout	796.88 lbs	57281.25 lbs	0.01	PASS
Coped Beam Flexural Rupture	83.05 lbs	64590.26 lbs	0.00	PASS
Coped Beam Lateral Torsional Buckling	83.05 lbs	59502.77 lbs	0.00	PASS
Clip Angle Flexural Yield			0.00	PASS
Clip Angle Flexural Rupture			0.00	PASS
Bolt Bearing on Girder	83.05 lbs	23856.47 lbs	0.00	PASS
Bolt Bearing on Clip Angle at Girder	83.05 lbs	23856.47 lbs	0.00	PASS
Bolt Bearing on Beam	801.20 lbs	23856.47 lbs	0.03	PASS
Bolt Bearing on Clip Angle at Beam	801.20 lbs	23856.47 lbs	0.03	PASS
Bolt Shear at Girder	83.05 lbs	16379.85 lbs	0.01	PASS
Bolt Group Eccentricity at Girder		0.69		

continued on next page...

J33 J - J15: ASD Results Report (continued):

Limit State	Required	Available	Unity Check	Result
Bolt Shear at Beam	801.20 lbs	16149.64 lbs	0.05	PASS
Bolt Group Eccentricity at Beam		0.68		
Angle Leg Bending	0.18 kips-ft	0.91 kips-ft	0.19	PASS
Bolt Prying				No Prying
Bolt Tension at Girder				N/A

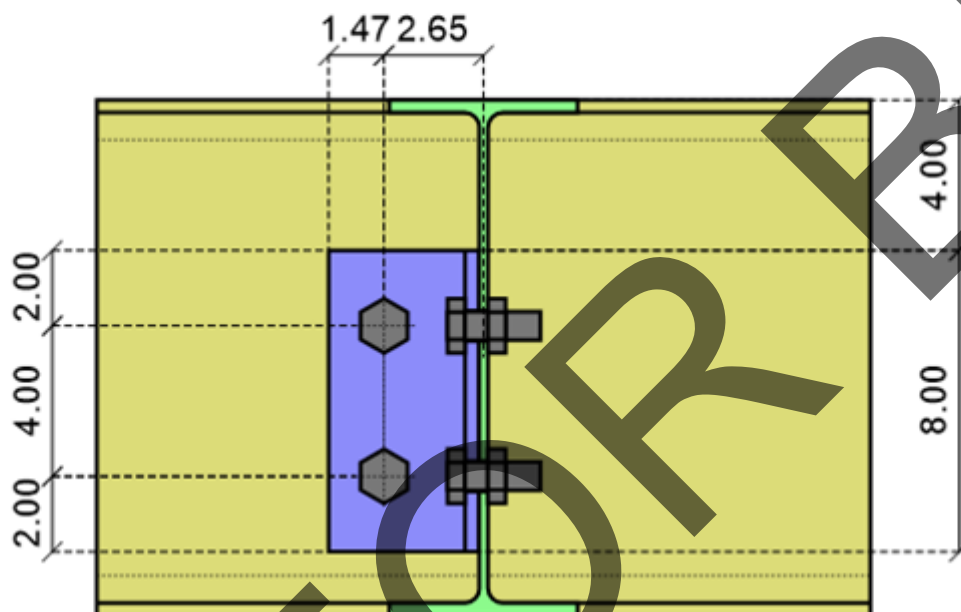
Side view



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J34 I - J13: 2D Views Report (continued):

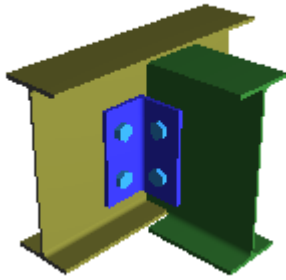
Front view



J34 I - J13: ASD Results Report

ASD

Girder/Beam Single Angle (One Side) Shear Connection



Material Properties:				
Girder	W14X22	A992	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi
Beam	W14X22	A992	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi
Angle	L4X4X6	A572 Gr.50	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi

Input Data:		
Shear Load	-491.70 lbs	User Input Shear Load
Axial Load	-182.67 lbs	User Input Axial Force (tension)

Governing LC: 3D - 12 - LC 12: IBC 21/ASCE Strength 7 (a)

Note: Unless specified, all code references are from AISC 360-10

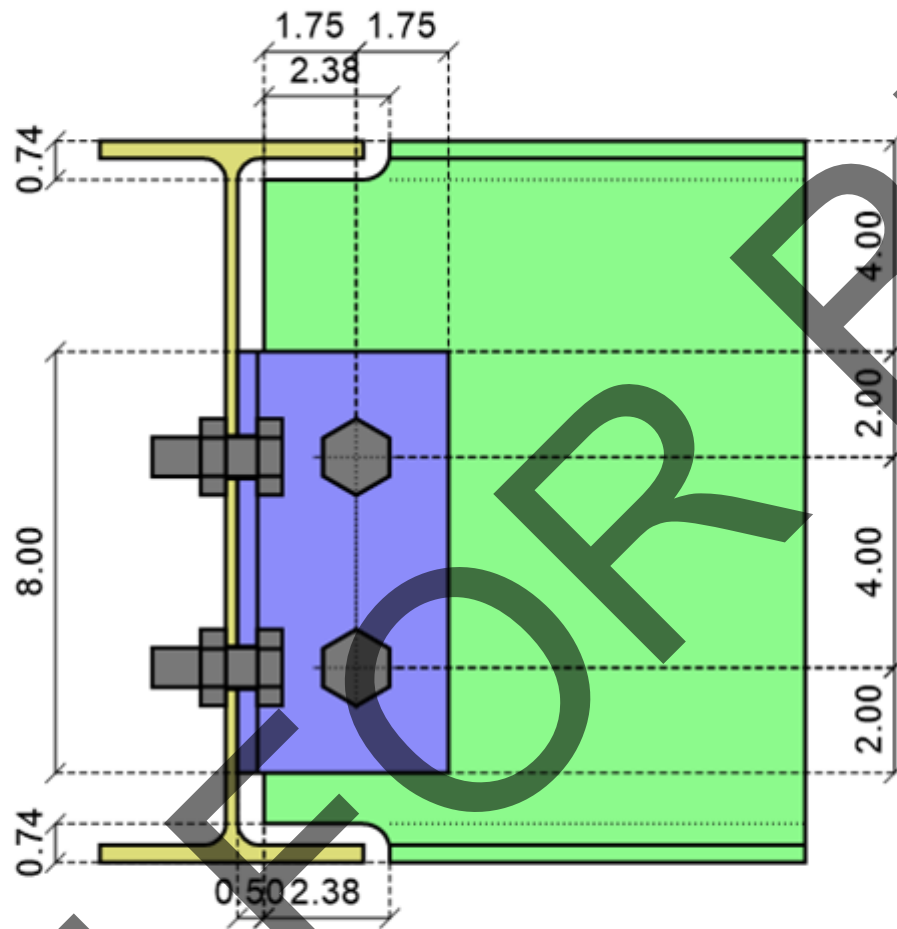
Limit State	Required	Available	Unity Check	Result
Geometry Restrictions at Beam				PASS
Geometry Restrictions at Girder				PASS
Erection Stability				PASS
Beam Shear Yield	491.70 lbs	56258.00 lbs	0.01	PASS
Clip Angle Shear Yield	491.70 lbs	60000.00 lbs	0.01	PASS
Beam Shear Rupture	491.70 lbs	47002.80 lbs	0.01	PASS
Clip Angle Shear Rupture at Beam	491.70 lbs	45703.12 lbs	0.01	PASS
Clip Angle Shear Rupture at Girder	491.70 lbs	45703.12 lbs	0.01	PASS
Beam Axial Yield	182.67 lbs	84218.56 lbs	0.00	PASS
Clip Angle Axial Yield	182.67 lbs	89820.36 lbs	0.00	PASS
Beam Tension Rupture	182.67 lbs	78338.00 lbs	0.00	PASS
Clip Angle Tension Rupture at Beam	182.67 lbs	76171.88 lbs	0.00	PASS
Clip Angle Block Shear at Girder	491.70 lbs	46303.12 lbs	0.01	PASS
Beam Block Shear	491.70 lbs	33840.19 lbs	0.01	PASS
Clip Angle Block Shear at Beam	491.70 lbs	49746.09 lbs	0.01	PASS
Beam Tearout	182.67 lbs	35132.50 lbs	0.01	PASS
Clip Angle Tearout	182.67 lbs	57281.25 lbs	0.00	PASS
Coped Beam Flexural Rupture	491.70 lbs	64590.26 lbs	0.01	PASS
Coped Beam Lateral Torsional Buckling	491.70 lbs	59502.77 lbs	0.01	PASS
Clip Angle Flexural Yield			0.00	PASS
Clip Angle Flexural Rupture			0.00	PASS
Bolt Bearing on Girder	491.70 lbs	23856.47 lbs	0.02	PASS
Bolt Bearing on Clip Angle at Girder	491.70 lbs	23856.47 lbs	0.02	PASS
Bolt Bearing on Beam	524.54 lbs	23856.47 lbs	0.02	PASS
Bolt Bearing on Clip Angle at Beam	524.54 lbs	23856.47 lbs	0.02	PASS
Bolt Shear at Girder	491.70 lbs	15229.97 lbs	0.03	PASS
Bolt Group Eccentricity at Girder		0.64		

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J34 I - J13: ASD Results Report (continued):

Limit State	Required	Available	Unity Check	Result
Bolt Shear at Beam	524.54 lbs	16878.45 lbs	0.03	PASS
Bolt Group Eccentricity at Beam		0.71		
Angle Leg Bending	0.04 kips-ft	0.91 kips-ft	0.04	PASS
Bolt Prying				No Prying
Bolt Tension at Girder				N/A

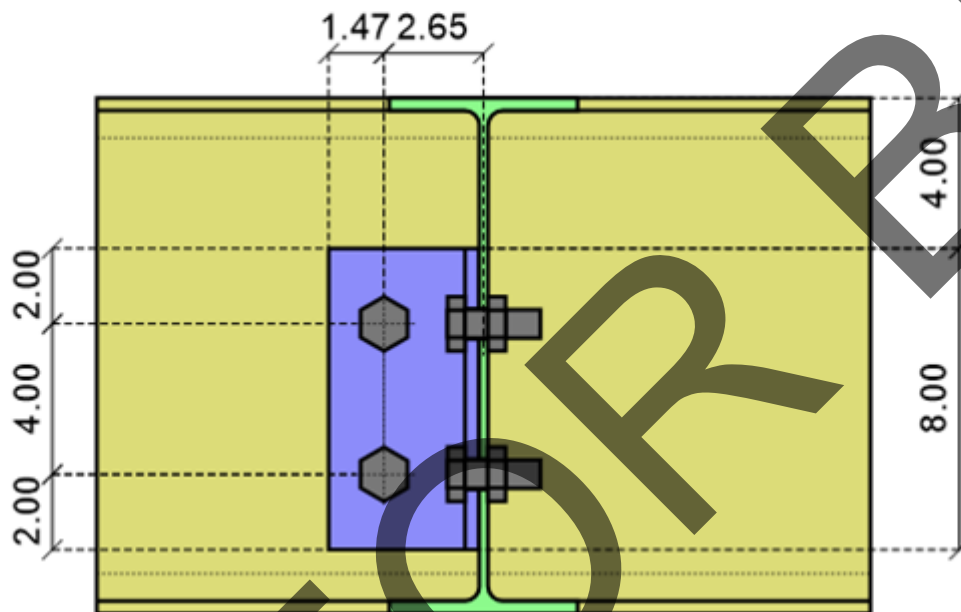
Side view



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J34 J - J14: 2D Views Report (continued):

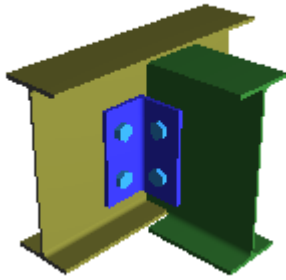
Front view



J34 J - J14: ASD Results Report

ASD

Girder/Beam Single Angle (One Side) Shear Connection



Material Properties:				
Girder	W14X22	A992	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi
Beam	W14X22	A992	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi
Angle	L4X4X6	A572 Gr.50	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi

Input Data:		
Shear Load	491.70 lbs	User Input Shear Load
Axial Load	-555.23 lbs	User Input Axial Force (tension)

Governing LC: 3D - 12 - LC 12: IBC 21/ASCE Strength 7 (a)

Note: Unless specified, all code references are from AISC 360-10

Limit State	Required	Available	Unity Check	Result
Geometry Restrictions at Beam				PASS
Geometry Restrictions at Girder				PASS
Erection Stability				PASS
Beam Shear Yield	491.70 lbs	56258.00 lbs	0.01	PASS
Clip Angle Shear Yield	491.70 lbs	60000.00 lbs	0.01	PASS
Beam Shear Rupture	491.70 lbs	47002.80 lbs	0.01	PASS
Clip Angle Shear Rupture at Beam	491.70 lbs	45703.12 lbs	0.01	PASS
Clip Angle Shear Rupture at Girder	491.70 lbs	45703.12 lbs	0.01	PASS
Beam Axial Yield	555.23 lbs	84218.56 lbs	0.01	PASS
Clip Angle Axial Yield	555.23 lbs	89820.36 lbs	0.01	PASS
Beam Tension Rupture	555.23 lbs	78338.00 lbs	0.01	PASS
Clip Angle Tension Rupture at Beam	555.23 lbs	76171.88 lbs	0.01	PASS
Clip Angle Block Shear at Girder	491.70 lbs	46303.12 lbs	0.01	PASS
Beam Block Shear	491.70 lbs	41775.19 lbs	0.01	PASS
Clip Angle Block Shear at Beam	491.70 lbs	49746.09 lbs	0.01	PASS
Beam Tearout	555.23 lbs	35132.50 lbs	0.02	PASS
Clip Angle Tearout	555.23 lbs	57281.25 lbs	0.01	PASS
Coped Beam Flexural Rupture	491.70 lbs	64590.26 lbs	0.01	PASS
Coped Beam Lateral Torsional Buckling	491.70 lbs	59502.77 lbs	0.01	PASS
Clip Angle Flexural Yield			0.00	PASS
Clip Angle Flexural Rupture			0.00	PASS
Bolt Bearing on Girder	491.70 lbs	23856.47 lbs	0.02	PASS
Bolt Bearing on Clip Angle at Girder	491.70 lbs	23856.47 lbs	0.02	PASS
Bolt Bearing on Beam	741.65 lbs	23856.47 lbs	0.03	PASS
Bolt Bearing on Clip Angle at Beam	741.65 lbs	23856.47 lbs	0.03	PASS
Bolt Shear at Girder	491.70 lbs	18718.98 lbs	0.03	PASS
Bolt Group Eccentricity at Girder		0.78		

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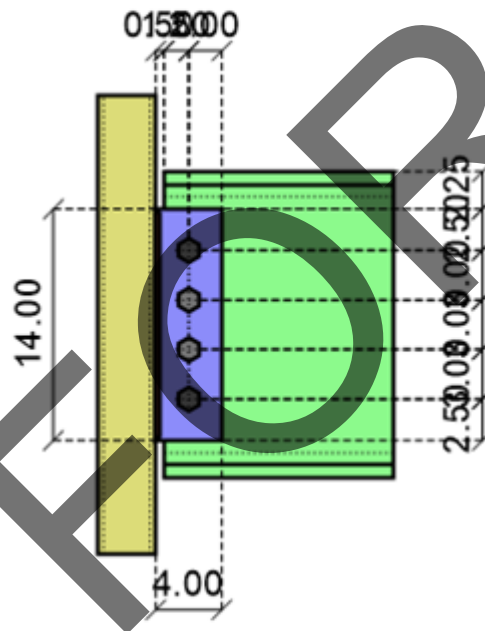
J34 J - J14: ASD Results Report (continued):

Limit State	Required	Available	Unity Check	Result
Bolt Shear at Beam	741.65 lbs	20439.03 lbs	0.04	PASS
Bolt Group Eccentricity at Beam		0.86		
Angle Leg Bending	0.12 kips-ft	0.91 kips-ft	0.13	PASS
Bolt Prying				No Prying
Bolt Tension at Girder				N/A

W18-L I - C_(1.9-B.3): 2D Views Report

Column/Beam Shear Tab Shear Connection

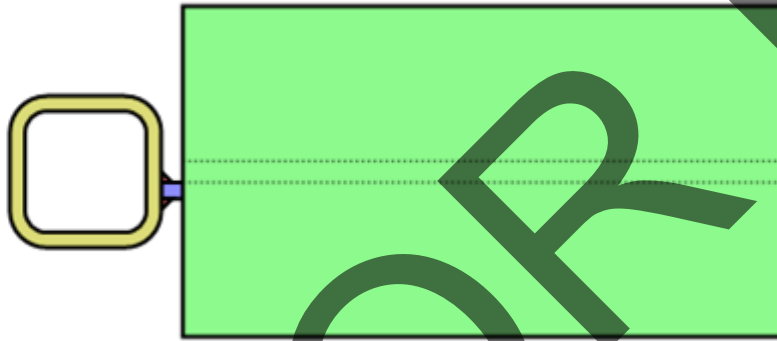
Side view



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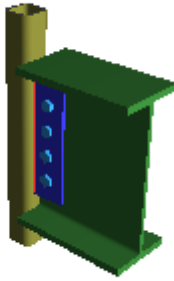
W18-L I - C_(1.9-B.3): 2D Views Report (continued):

Top view



W18-L I - C_(1.9-B.3): ASD Results Report

Column/Beam Shear Tab Shear Connection



Material Properties:

Column	HSS3.5X3.5X6	A500 Gr.C RECT	$F_y = 50.00$ ksi	$F_u = 62.00$ ksi
Beam	W18X71	A992	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi
Plate	P0.38x4.00x14.00	A572 Gr.50	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi

Input Data:

Shear Load	13907.00 lbs	User Input Shear Load
Axial Load	-1.04 lbs	User Input Axial Force (tension)
Column Force	13909.40 lbs	User Input Column Force
Column Moment	0.51 kips-ft	User Input Column Moment

Governing LC: 3D - 6 - LC 6: IBC 21/ASCE ASD 3 (a)

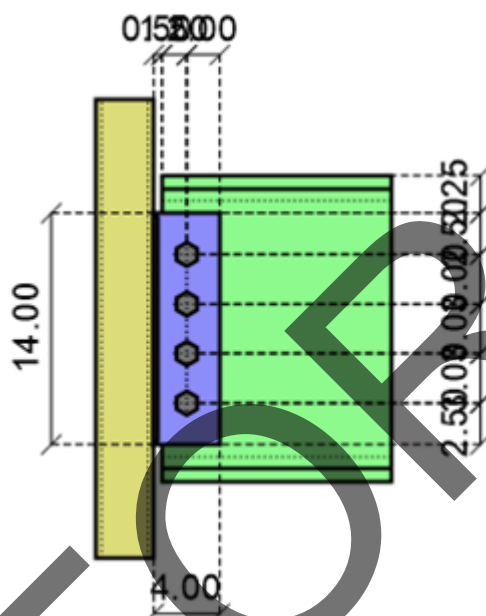
Note: Unless specified, all code references are from AISC 360-10

Limit State	Required	Available	Unity Check	Result
HSS Punching Shear				PASS
Geometry Restrictions at Beam				PASS
Column Weld Limitations				PASS
Rotational Ductility, Erection Stability				PASS
Beam Shear Yield	13907.00 lbs	183150.00 lbs	0.08	PASS
Plate Shear Yield	13907.00 lbs	105000.00 lbs	0.13	PASS
Beam Shear Rupture	13907.00 lbs	144787.50 lbs	0.10	PASS
Plate Shear Rupture at Beam	13907.00 lbs	76781.25 lbs	0.18	PASS
Beam Block Shear	13907.00 lbs	155636.72 lbs	0.09	PASS
Plate Block Shear	13907.00 lbs	80742.19 lbs	0.17	PASS
Lateral Stability / Stabilizer Plates	13907.00 lbs	520818.63 lbs	0.03	PASS
Plate Flexural Yield			0.02	PASS
Plate Flexural Rupture			0.03	PASS
Plate Flexural Buckling	13907.00 lbs	366766.47 lbs	0.04	PASS
Bolt Bearing on Beam	13907.00 lbs	47712.94 lbs	0.29	PASS
Bolt Bearing on Plate at Beam	13907.00 lbs	47712.94 lbs	0.29	PASS
Bolt Shear at Beam	13907.00 lbs	44749.96 lbs	0.31	PASS
Bolt Group Eccentricity		0.94		
Weld at Column	15699.95 lbs/ft	89088.00 lbs/ft	0.18	PASS

W18-L J - C_(2.4-B.3): 2D Views Report

Column/Beam Shear Tab Shear Connection

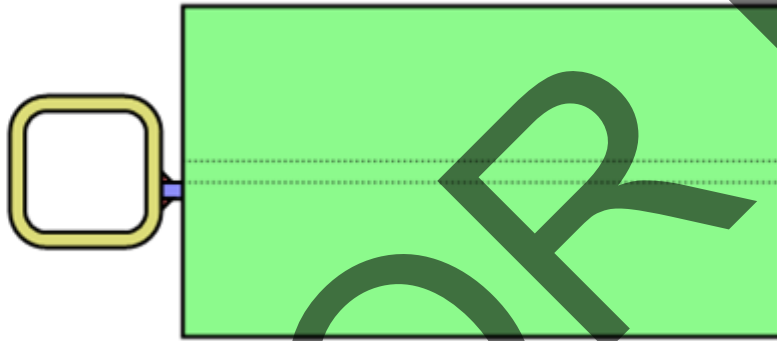
Side view



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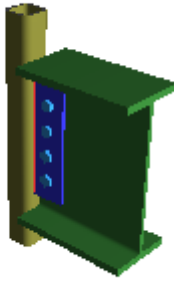
W18-L J - C_(2.4-B.3): 2D Views Report (continued):

Top view



W18-L J - C_(2.4-B.3): ASD Results Report

Column/Beam Shear Tab Shear Connection



Material Properties:

Column	HSS3.5X3.5X6	A500 Gr.C RECT	$F_y = 50.00$ ksi	$F_u = 62.00$ ksi
Beam	W18X71	A992	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi
Plate	P0.38x4.00x14.00	A572 Gr.50	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi

Input Data:

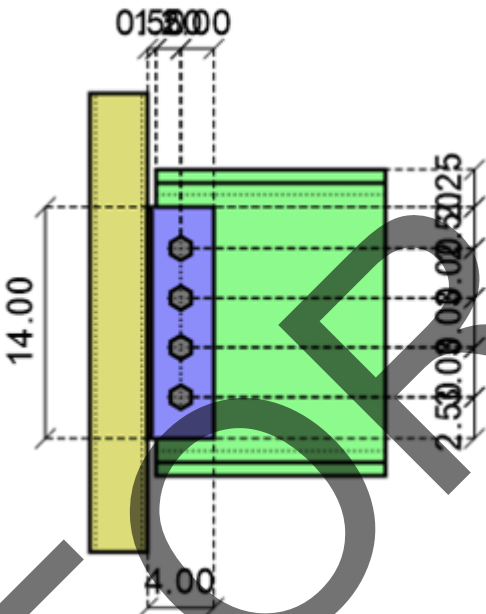
Shear Load	-2658.44 lbs	User Input Shear Load
Axial Load	7823.08 lbs	User Input Axial Force (compression)
Column Force	22273.21 lbs	User Input Column Force
Column Moment	-0.66 kips-ft	User Input Column Moment

Governing LC: 3D - 10 - LC 10: IBC 21/ASCE
Strength 6 (c)

Note: Unless specified, all code references are from AISC 360-10

Limit State	Required	Available	Unity Check	Result
HSS Punching Shear				PASS
Geometry Restrictions at Beam				PASS
Column Weld Limitations				PASS
Rotational Ductility, Erection Stability				PASS
Beam Shear Yield	2658.44 lbs	183150.00 lbs	0.01	PASS
Plate Shear Yield	2658.44 lbs	105000.00 lbs	0.03	PASS
Beam Shear Rupture	2658.44 lbs	144787.50 lbs	0.02	PASS
Plate Shear Rupture at Beam	2658.44 lbs	76781.25 lbs	0.03	PASS
Beam Axial Yield	7823.08 lbs	625748.50 lbs	0.01	PASS
Plate Axial Yield	7823.08 lbs	157185.63 lbs	0.05	PASS
Beam Block Shear	2658.44 lbs	155636.72 lbs	0.02	PASS
Plate Block Shear	2658.44 lbs	80742.19 lbs	0.03	PASS
Compression Buckling of the Plate	7823.08 lbs	157185.63 lbs	0.05	PASS
Lateral Stability / Stabilizer Plates	8262.44 lbs	520818.63 lbs	0.02	PASS
Plate Flexural Yield			0.00	PASS
Plate Flexural Rupture			0.00	PASS
Plate Flexural Buckling			0.06	PASS
Bolt Bearing on Beam	8262.44 lbs	47712.94 lbs	0.17	PASS
Bolt Bearing on Plate at Beam	8262.44 lbs	47712.94 lbs	0.17	PASS
Bolt Shear at Beam	8262.44 lbs	40923.39 lbs	0.20	PASS
Bolt Group Eccentricity		0.86		
Weld at Column	8262.44 lbs	103936.00 lbs	0.08	PASS
HSS Transverse Plastification	7823.08 lbs	53007.23 lbs	0.15	PASS

Side view



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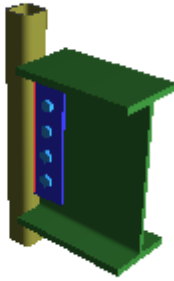
W18-R I - C_(2.4-B.3): 2D Views Report (continued):

Top view



W18-R I - C_(2.4-B.3): ASD Results Report

Column/Beam Shear Tab Shear Connection



Material Properties:

Column	HSS3.5X3.5X6	A500 Gr.C RECT	$F_y = 50.00$ ksi	$F_u = 62.00$ ksi
Beam	W18X71	A992	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi
Plate	P0.38x4.00x14.00	A572 Gr.50	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi

Input Data:

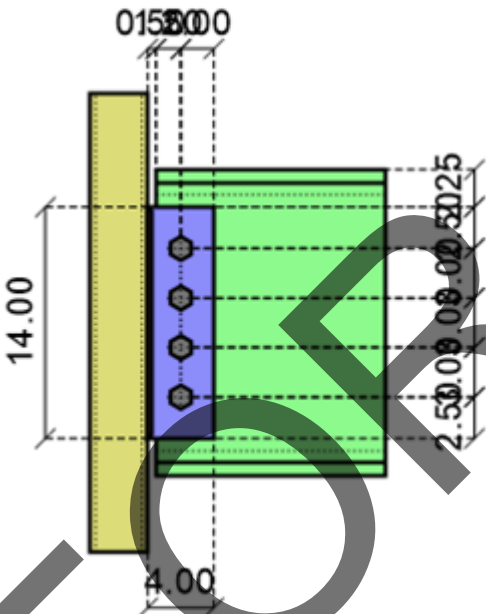
Shear Load	8651.40 lbs	User Input Shear Load
Axial Load	7875.06 lbs	User Input Axial Force (compression)
Column Force	22273.21 lbs	User Input Column Force
Column Moment	-0.66 kips-ft	User Input Column Moment

Governing LC: 3D - 10 - LC 10: IBC 21/ASCE
Strength 6 (c)

Note: Unless specified, all code references are from AISC 360-10

Limit State	Required	Available	Unity Check	Result
HSS Punching Shear				PASS
Geometry Restrictions at Beam				PASS
Column Weld Limitations				PASS
Rotational Ductility, Erection Stability				PASS
Beam Shear Yield	8651.40 lbs	183150.00 lbs	0.05	PASS
Plate Shear Yield	8651.40 lbs	105000.00 lbs	0.08	PASS
Beam Shear Rupture	8651.40 lbs	144787.50 lbs	0.06	PASS
Plate Shear Rupture at Beam	8651.40 lbs	76781.25 lbs	0.11	PASS
Beam Axial Yield	7875.06 lbs	625748.50 lbs	0.01	PASS
Plate Axial Yield	7875.06 lbs	157185.63 lbs	0.05	PASS
Beam Block Shear	8651.40 lbs	155636.72 lbs	0.06	PASS
Plate Block Shear	8651.40 lbs	80742.19 lbs	0.11	PASS
Compression Buckling of the Plate	7875.06 lbs	157185.63 lbs	0.05	PASS
Lateral Stability / Stabilizer Plates	11698.86 lbs	520818.63 lbs	0.02	PASS
Plate Flexural Yield			0.01	PASS
Plate Flexural Rupture			0.01	PASS
Plate Flexural Buckling			0.10	PASS
Bolt Bearing on Beam	11698.86 lbs	47712.94 lbs	0.25	PASS
Bolt Bearing on Plate at Beam	11698.86 lbs	47712.94 lbs	0.25	PASS
Bolt Shear at Beam	11698.86 lbs	38776.31 lbs	0.30	PASS
Bolt Group Eccentricity		0.81		
Weld at Column	11698.86 lbs	103936.00 lbs	0.11	PASS
HSS Transverse Plastification	7875.06 lbs	53007.23 lbs	0.15	PASS

Side view



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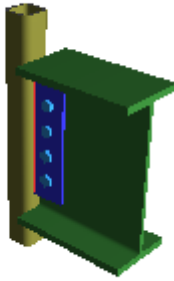
W18-R J - C_(3.9-B.3): 2D Views Report (continued):

Top view



W18-R J - C_(3.9-B.3): ASD Results Report

Column/Beam Shear Tab Shear Connection



Material Properties:

Column	HSS3.5X3.5X6	A500 Gr.C RECT	$F_y = 50.00$ ksi	$F_u = 62.00$ ksi
Beam	W18X71	A992	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi
Plate	P0.38x4.00x14.00	A572 Gr.50	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi

Input Data:

Shear Load	-5739.66 lbs	User Input Shear Load
Axial Load	-436.36 lbs	User Input Axial Force (tension)
Column Force	-3018.97 lbs	User Input Column Force
Column Moment	0.39 kips-ft	User Input Column Moment

Governing LC: 3D - 6 - LC 6: IBC 21/ASCE ASD 3 (a)

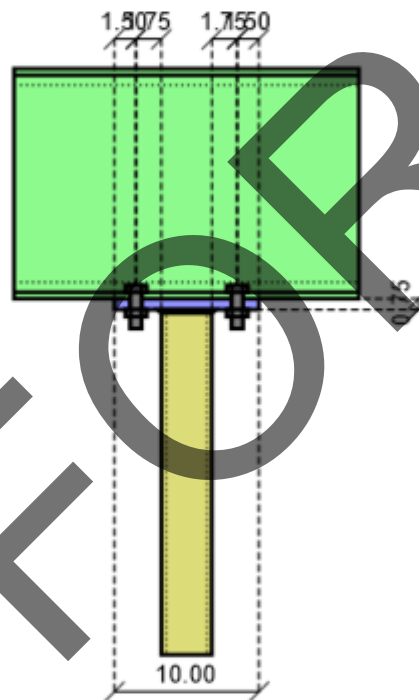
Note: Unless specified, all code references are from AISC 360-10

Limit State	Required	Available	Unity Check	Result
HSS Punching Shear				PASS
Geometry Restrictions at Beam				PASS
Column Weld Limitations				PASS
Rotational Ductility, Erection Stability				PASS
Beam Shear Yield	5739.66 lbs	183150.00 lbs	0.03	PASS
Plate Shear Yield	5739.66 lbs	105000.00 lbs	0.05	PASS
Beam Shear Rupture	5739.66 lbs	144787.50 lbs	0.04	PASS
Plate Shear Rupture at Beam	5739.66 lbs	76781.25 lbs	0.07	PASS
Beam Axial Yield	436.36 lbs	625748.50 lbs	0.00	PASS
Plate Axial Yield	436.36 lbs	157185.63 lbs	0.00	PASS
Beam Tension Rupture	436.36 lbs	249046.35 lbs	0.00	PASS
Plate Tension Rupture at Beam	436.36 lbs	127968.75 lbs	0.00	PASS
Beam Block Shear	5739.66 lbs	155636.72 lbs	0.04	PASS
Plate Block Shear	5739.66 lbs	80742.19 lbs	0.07	PASS
Beam Tearout	436.36 lbs	123069.38 lbs	0.00	PASS
Plate Tearout on Plate at Beam	436.36 lbs	100195.31 lbs	0.00	PASS
Lateral Stability / Stabilizer Plates	5756.22 lbs	520818.63 lbs	0.01	PASS
Plate Flexural Yield			0.00	PASS
Plate Flexural Rupture			0.01	PASS
Plate Flexural Buckling	5739.66 lbs	183383.23 lbs	0.03	PASS
Bolt Bearing on Beam	5756.22 lbs	47712.94 lbs	0.12	PASS
Bolt Bearing on Plate at Beam	5756.22 lbs	47712.94 lbs	0.12	PASS
Bolt Shear at Beam	5756.22 lbs	39437.13 lbs	0.15	PASS
Bolt Group Eccentricity		0.83		
Weld at Column	6729.04 lbs/ft	89088.00 lbs/ft	0.08	PASS
HSS Transverse Plastification	436.36 lbs	53564.84 lbs	0.01	PASS

Column Cap: 2D Views Report

Front view

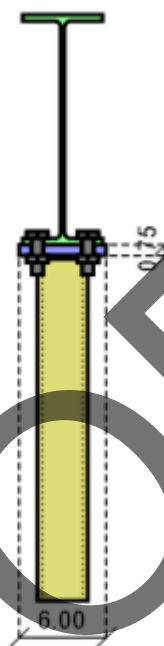
Column/Beam Continuous Beam over
Column Connection



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Column Cap: 2D Views Report (continued):

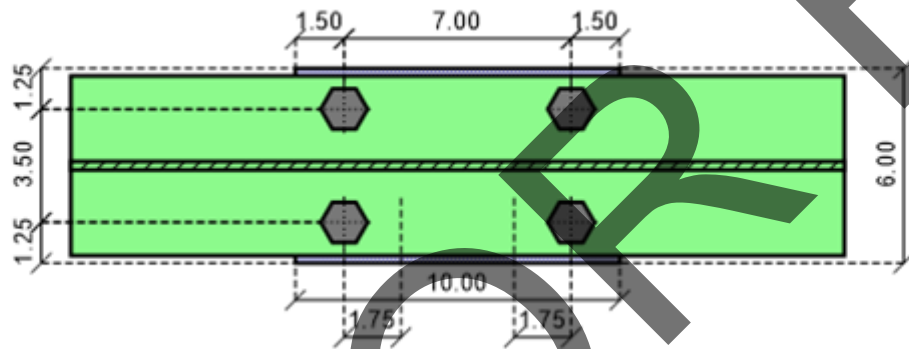
Side view



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Column Cap: 2D Views Report (continued):

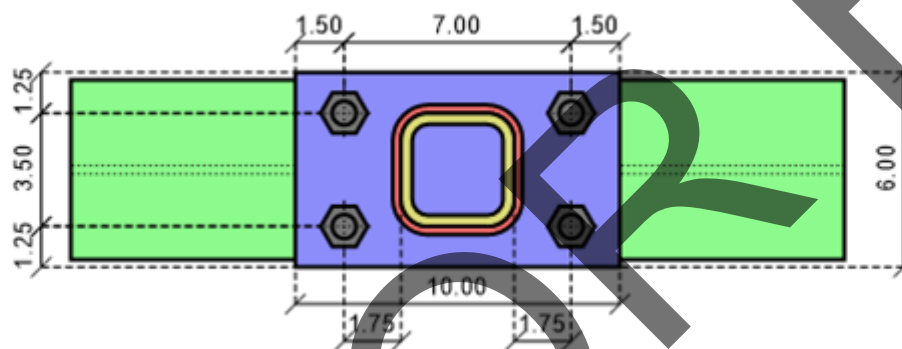
Top view



continued on next page...

Column Cap: 2D Views Report (continued):

Bottom view



Column Cap: LRFD Results Report

LRFD

Column/Beam Continuous Beam over
Column Connection



Material Properties:

Column	HSS3.5X3.5X6	A500 Gr.C Rect	$F_y = 50.00$ ksi	$F_u = 62.00$ ksi
Beam	W16X31	A36	$F_y = 36.00$ ksi	$F_u = 58.00$ ksi
Plate	P0.75x6.00x10.00	A572 Gr.50	$F_y = 50.00$ ksi	$F_u = 65.00$ ksi

Input Data:

Column Force	22.00 kips	User Input Column Force
Moment	0.00 kips-ft	User Input Moment
Strong Axis Shear	0.00 kips	User Input Strong Axis Shear
Weak Axis Shear	0.00 kips	User Input Weak Axis Shear
Puf_c	11.00 kips	Required Flange Force (compression)
Puf_t	0.00 kips	Required Tension Force per Bolt

Note: Unless specified, all code references are from AISC 360-16

Limit State	Required	Available	Unity Check	Result
Load Distribution				n/a
Geometry Restrictions at Beam				PASS
Column Weld Limitations				PASS
Bolt Shear Strength	0.00 kips	71.57 kips	0.00	PASS
Bolt Bearing on Beam	0.00 kips	71.57 kips	0.00	PASS
Bolt Bearing on Plate at Beam	0.00 kips	71.57 kips	0.00	PASS
Column Flange Weld Strength	0.00 kips/in	5.57 kips/in	0.00	PASS
Column Web Weld Strength	0.00 kips/in	5.57 kips/in	0.00	PASS
Beam Web Yielding	11.00 kips	82.26 kips	0.13	PASS
Beam Web Buckling	11.00 kips	32.29 kips	0.34	PASS
Beam Web Crippling	11.00 kips	81.05 kips	0.14	PASS
Column Wall Local Yielding	11.00 kips	97.28 kips	0.11	PASS
Column Wall Local Crippling	11.00 kips	129.44 kips	0.08	PASS

EXHIBITS

NOT FOR BID

A5.602
CALGreen VERIFICATION GUIDELINES
MANDATORY MEASURES CHECKLIST
2022 EDITION

APPLICATION: THIS CHECKLIST SHALL BE USED FOR NONRESIDENTIAL PROJECTS THAT MEET ONE OF THE FOLLOWING: NEW CONSTRUCTION, BUILDING ADDITIONS OF 1,000 SQUARE FEET OR GREATER, OR BUILDING ALTERATIONS WITH A PERMIT VALUATION OF \$50,000 OR MORE PURSUANT TO SECTION 301.3 AND NOT TRIGGER A TIER 1 OR TIER 2 REQUIREMENT.

Y = YES (SECTION HAS BEEN SELECTED AND/OR INCULCATED)
N/A = NOT APPLICABLE (CODE SECTIONS DOES NOT APPLY TO THE PROJECT - MAINLY USED FOR ADDITIONS AND ALTERATIONS)
O = OTHER (PERMIT SPECIFIC)
IN = NEW CONSTRUCTION PURSUANT TO SECTION 301.3
AL = ADDITIONAL AND/OR ALTERATIONS PURSUANT TO SECTION 301.3

CHAPTER 5 DIVISIONS							
DIVISION 5.1 PLANNING AND DESIGN							
REQUIREMENT	SECTION TITLE	CODE SECTION	Y	N	N/A	O	PLAN SHEET, SPEC. OR ATTACH REFERENCE
MANDATORY	STORM WATER POLLUTION PREVENTION FOR PROJECTS THAT COVER LESS THAN 1 ACRE OF LAND	5.106.1 THROUGH 5.106.2					
MANDATORY	SHORT-TERM BICYCLE PARKING (WITH EXCEPTION)	5.106.4.1					
MANDATORY	LONG-TERM BICYCLE PARKING	5.106.4.2 THROUGH 5.106.5.3					
MANDATORY	ELECTRIC VEHICLE (EV) CHARGING (IN (WITH EXCEPTIONS))	5.106.5.3					
MANDATORY	EV CAPABLE SPACES (IN)	5.106.5.3.1					
MANDATORY	ELECTRIC VEHICLE CHARGING STATIONS (EVCS)	5.106.5.3.2					
MANDATORY	USE OF AUTOMATIC LOAD MANAGEMENT SYSTEMS (ALMS)	5.106.5.3.3					
MANDATORY	ACCESSIBLE EVCS	5.106.5.3.4					
MANDATORY	ELECTRIC VEHICLE CHARGING, MEDIUM-DUTY AND HEAVY-DUTY (WITH EXCEPTIONS)	5.106.5.4					
MANDATORY	ELECTRIC VEHICLE CHARGING READINESS REQUIREMENTS FOR WAREHOUSES, GROCERY STORES AND RETAIL STORES WITH PLANNED OFF-STREET LOADING SPACES (IN)	5.106.5.4.1					
MANDATORY	SOFT FLOORS FOR REDUCTION (IN) (WITH EXCEPTIONS)	5.106.8					
MANDATORY	PACING - BACKLIGHT (WITH EXCEPTIONS)	5.106.8.1					
MANDATORY	PACING - GLARE	5.106.8.2					
MANDATORY	GRADING AND PAVING (WITH EXCEPTIONS)	5.106.10					
DIVISION 5.2 ENERGY EFFICIENCY							
REQUIREMENT	SECTION TITLE	CODE SECTION	Y	N	N/A	O	PLAN SHEET, SPEC. OR ATTACH REFERENCE
MANDATORY	MEET THE MINIMUM ENERGY EFFICIENCY STANDARD	5.201.1					
DIVISION 5.3 WATER EFFICIENCY AND CONSERVATION							
REQUIREMENT	SECTION TITLE	CODE SECTION	Y	N	N/A	O	PLAN SHEET, SPEC. OR ATTACH REFERENCE
MANDATORY	SEPARATE METERS (NEW BUILDINGS OR ADDITIONS > \$5,000 IF THAT CONSUME MORE THAN 100 GALLONS)	5.303.1.1					
MANDATORY	SEPARATE METERS FOR TOWELS IN NEW BUILDINGS OR ADDITIONS THAT CONSUME MORE THAN 1,000 GALLONS	5.303.1.2					
MANDATORY	WATER CLOSETS SHALL NOT EXCEED 1.28 GALLONS PER FLUSH (GPF)	5.303.3.1					
MANDATORY	WALL-MOUNTED URINALS SHALL NOT EXCEED 0.125 GPF	5.303.3.2					
MANDATORY	PRINCLE SHOWERSHALL HAVE MAXIMUM FLOW RATE OF 1.8 GPM (GALLONS PER MINUTE) AT 80 PS	5.303.3.3					
MANDATORY	MULTIPLE SHOWERSHALLS SHOWER SHALL HAVE A COMBINED FLOW RATE OF 1.8 GPM AT 80 PS	5.303.3.3.2					
MANDATORY	NONRESIDENTIAL LAVATORY FAUCETS	5.303.3.4					
MANDATORY	KITCHEN FAUCETS	5.303.3.4.2					
MANDATORY	WASH FOUNTAINS	5.303.3.4.3					
MANDATORY	METERING FAUCETS	5.303.3.4.4					
MANDATORY	METERING FAUCETS FOR WASH FOUNTAINS	5.303.3.4.5					
MANDATORY	PRE-RINSE SPRAY VALVE	5.303.3.4.6					
MANDATORY	FOOD WASTE DISPOSERS	5.303.4.1					
MANDATORY	AREAS OF ADDITION OR ALTERATION	5.303.5					
MANDATORY	STANDARDS FOR PLUMBING FIXTURES AND FITTINGS	5.303.6					
MANDATORY	OUTDOOR POTABLE WATER USE IN LANDSCAPE AREA	5.304.1					
DIVISION 5.4 MATERIAL CONSERVATION AND RESOURCE EFFICIENCY							
REQUIREMENT	SECTION TITLE	CODE SECTION	Y	N	N/A	O	PLAN SHEET, SPEC. OR ATTACH REFERENCE
MANDATORY	WEATHER PROTECTION	5.401.1					
MANDATORY	MOISTURE CONTROL - SPRINKLERS	5.407.2.1					
MANDATORY	MOISTURE CONTROL - EXTERIOR DOOR PROTECTION	5.407.2.2					
MANDATORY	MOISTURE CONTROL - FLASHING	5.407.2.3					
MANDATORY	CONSTRUCTION WASTE MANAGEMENT - COMPLY WITH WASTE REDUCTION 5.408.1.1, 5.408.1.2, 5.408.1.3 OR MORE STRINGENT LOCAL ORDINANCE	5.408.1.1, 5.408.1.2, 5.408.1.3					
MANDATORY	CONSTRUCTION WASTE MANAGEMENT - DOCUMENTATION	5.408.1.4					
MANDATORY	UNPAVED WAREHOUSE	5.408.2					
MANDATORY	BACKFILLED SOIL AND LAND CLEARING DEBRIS FOR REUSE OR RECYCLE	5.408.3					
MANDATORY	RECYCLING BY OCCUPANTS (WITH EXCEPTION)	5.410.1					
MANDATORY	RECYCLING BY OCCUPANTS (WITH EXCEPTION)	5.410.1.1					
MANDATORY	RECYCLING BY OCCUPANTS (WITH EXCEPTION)	5.410.1.2					
MANDATORY	COMPOSTING (WITH EXCEPTIONS, 10,000 SQ FT IN)	5.410.2					
MANDATORY	OWNER'S OR OWNER'S AGENT'S PROJECT REQUIREMENTS (IN)	5.410.2.1					
MANDATORY	BASE OF CONCRETE FLOOR (IN)	5.410.2.2					
MANDATORY	CONCRETE FLOOR (IN)	5.410.2.3					
MANDATORY	INTERIOR THERMOCHROMATIC FILMS (IN)	5.410.2.4					
MANDATORY	GLASS AND THERMOCHROMATIC FILMS (IN)	5.410.2.5					
MANDATORY	INTERIOR THERMOCHROMATIC FILMS (IN)	5.410.2.5.1					
MANDATORY	THERMOCHROMATIC FILMS (IN)	5.410.2.5.2					
MANDATORY	COOLING REPORT (IN)	5.410.2.6					
MANDATORY	TESTING AND ADJUSTING FOR NEW BUILDINGS, 10,000 SQ FT OR NEW SYSTEMS THAT HAVE ADDITIONS OR ALTERATIONS	5.410.4					
MANDATORY	SYSTEM TESTING PLAN FOR RENEWABLE ENERGY, LANDSCAPE IRRIGATION AND WATER REUSE (IN)	5.410.4.2					
MANDATORY	PROCEDURES FOR TESTING AND ADJUSTING	5.410.4.3					
MANDATORY	PROCEDURES FOR HVAC BALANCING	5.410.4.3.1					
MANDATORY	TESTING AND ADJUSTING	5.410.4.3.2					
MANDATORY	OPERATION AND MAINTENANCE (O&M) MANUAL	5.410.4.3.3					
MANDATORY	INDEPENDENT AND BIDDING	5.410.4.3.4					

DIVISION 5.5 ENVIRONMENTAL QUALITY							
REQUIREMENT	SECTION TITLE	CODE SECTION	Y	N	N/A	O	PLAN SHEET, SPEC. OR ATTACH REFERENCE
MANDATORY	REFRIGERANTS	5.501.1					
MANDATORY	MECHANICAL EQUIPMENT VENTILATION	5.504.1					
MANDATORY	COVERING OF DUCT OPENINGS AND PROTECTION OF MECHANICAL EQUIPMENT DURING CONSTRUCTION	5.504.2					
MANDATORY	ADHESIVES, SEALANTS, AND CAULKES	5.504.3					
MANDATORY	PAINTS AND COATINGS	5.504.5					
MANDATORY	AEROSOL PAINTS AND COATINGS	5.504.5.1					
MANDATORY	ADHESIVES, SEALANTS, AND COATINGS, VERIFICATION	5.504.5.2					
MANDATORY	CARPET SYSTEMS	5.504.6.1					
MANDATORY	CARPET FLOORING	5.504.6.2					
MANDATORY	COMPOSITE WOOD PRODUCTS 5.504.6.3	5.504.6.3					
MANDATORY	COMPOSITE WOOD PRODUCTS, DOCUMENTATION	5.504.6.3.1					
MANDATORY	RESILIENT FLOORING	5.504.6.4					
MANDATORY	RESILIENT FLOORING, VERIFICATION OF COMPLIANCE	5.504.6.4.1					
MANDATORY	THERMAL INSULATION	5.504.7					
MANDATORY	THERMAL INSULATION, VERIFICATION OF COMPLIANCE	5.504.7.1					
MANDATORY	ACOUSTICAL CEILING AND WALL PANELS	5.504.8					
MANDATORY	ACOUSTICAL CEILING AND WALL PANELS, VERIFICATION OF COMPLIANCE	5.504.8.1					
MANDATORY	WATER LEAKAGE	5.504.9					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.1					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.2					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.3					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.4					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.5					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.6					
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MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.147					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.148					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.149					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.150					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.151					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.152					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.153					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.154					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.155					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.156					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.157					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.158					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.159					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.160					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.161					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.162					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.163					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.164					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.165					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.166					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.167					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.168					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.169					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.170					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.171					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.172					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.173					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.174					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.175					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.176					
MANDATORY	WATER LEAKAGE, VERIFICATION OF COMPLIANCE	5.504.9.177					
MANDATORY	WATER						

CODE ANALYSIS PLAN LEGEND

IMAGE	KEY	DESCRIPTION	REMARKS / COMMENTS
	EXIT SIGN		
	(E) WALL TO REMAIN		
	(E) 1 HR RATED WALL TO REMAIN		
	(E) FINISHED WALL		
	DR - CARD HOLDER		REFER TO ELECTRICAL FOR ADDITIONAL INFORMATION
	RECEIVING AREA		REFER TO XXX
	EXIT ACCESS DOOR		
	EXIT OCCUPANT LOAD		
	CLEAR FLOOR SPACE		

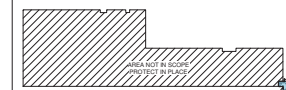
CODE GENERAL NOTES

- PER SECTION 903.1.2 - SMALL ASSEMBLY SPACES: A ROOM OR SPACE USED FOR ASSEMBLY PURPOSES WITH AN OCCUPANT LOAD OF LESS THAN 50 PERSONS AND ACCESSORY TO ANOTHER OCCUPANCY SHALL BE CLASSIFIED AS A GROUP B OCCUPANCY OR AS PART OF THAT OCCUPANCY.
- SQUARE FOOTAGES SHOWN ON SCHEDULE ARE USABLE AREA.
- BUILDINGS AND SITE SHALL BE ACCESSIBLE PER TCM COR. SIGNAGE SHALL BE PROVIDED USING THE INTERNATIONAL SYMBOL OF ACCESSIBILITY (IF 1000 IN FEDERAL STANDARD 500) AS REQUIRED ALONG ACCESSIBLE ROUTES TO ACCESSIBLE ENTRANCES. SIGNAGE SHALL ALSO BE PROVIDED AT EACH ACCESSIBLE ENTRANCE. REFER TO G102 FOR ADDITIONAL INFORMATION.
- FIRE EXTINGUISHERS SHALL BE PLACED AS REQUIRED BY THE FIRE MARSHALL. FIRE EXTINGUISHERS SHALL BE PLACED IN FIRE EXTINGUISHER CABINETS. INSTALL PORTABLE FIRE EXTINGUISHERS PER NFPA PAMPHLET 802, BUT NOT LESS THAN 21002 IN RATING. CABINETS SHALL BE SURFACE MOUNTED OR SEMI-RECESSED PER PLAN.
- FOR BUILDING OCCUPANT LOAD CALCULATIONS REFER TO 0800.
- FOR ALL BUILDING ACCESSIBILITY DETAILS REFER TO 4000 AND 4501.
- FOR EMERGENCY LIGHTING LOCATIONS, REFER TO ELECTRICAL SHEETS FOR ADDITIONAL INFORMATION.

EXIT ACCESS TRAVEL DISTANCE

ROUTE NAME	ROUTE LENGTH
ROUTE A	127'-0"
ROUTE B	131'-0"

KEY PLAN



SAN BERNARDINO COUNTY
SHERIFF'S CRIME LAB
200 SOUTH LENA RD.
SAN BERNARDINO, CA

PROJECT NO. - SBC-001

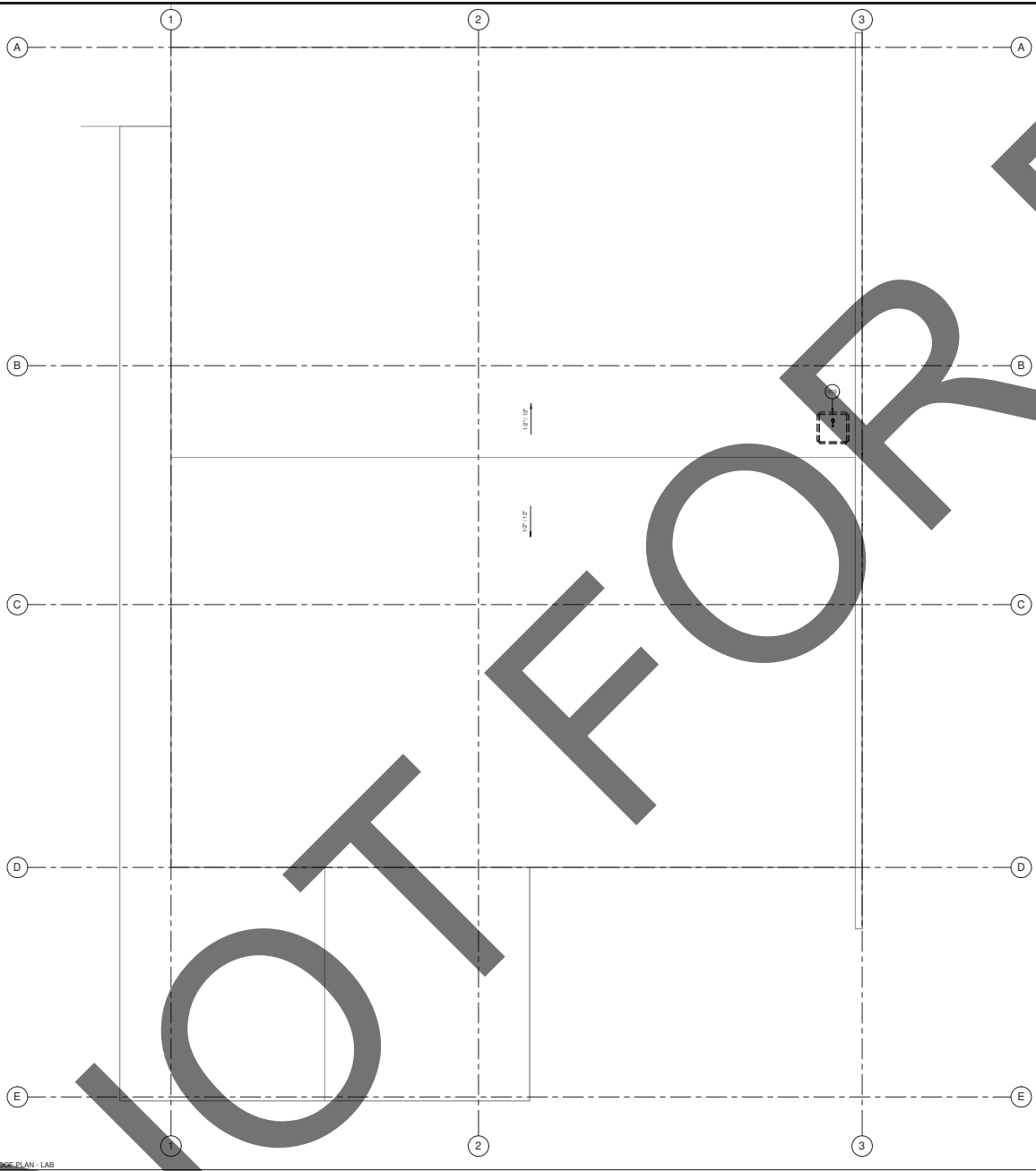
CODE ANALYSIS
PLAN - LAB

G101

CODE ANALYSIS PLAN - LAB
1/4" = 1'-0"

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DEMOLITION ROOF PLAN - LAB
1/4" = 1'-0"



DEMOLITION PLAN LEGEND

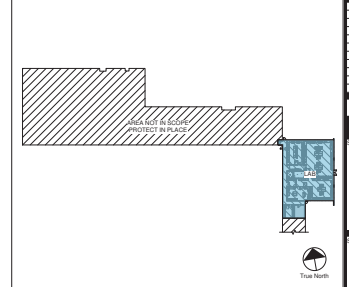
IMAGE	KEY	DESCRIPTION	REMARKS / COMMENTS

DEMOLITION GENERAL NOTES

- EXISTING ROOFING TO REMAIN UNLESS NOTED OTHERWISE.
- EXISTING AREAS SCHEDULED TO REMAIN THAT ARE DAMAGED DURING CONSTRUCTION SHALL BE PATCHED, REPAIRED AND FINISHED TO MATCH ADJACENT SURFACES AT NO COST TO THE OWNER.
- REFER TO MECHANICAL, ELECTRICAL AND PLUMBING FOR ADDITIONAL DEMOLITION INFORMATION.

DEMOLITION KEYNOTES

KEY PLAN



HOLT
ARCHITECTURE
1800 100th ST. STE. 100
PALM DESERT, CA 92411
PH: 760.338.0300

WWW.HOLTARCHITECTURE.COM

DATE: 1/1/2021

STAGE:

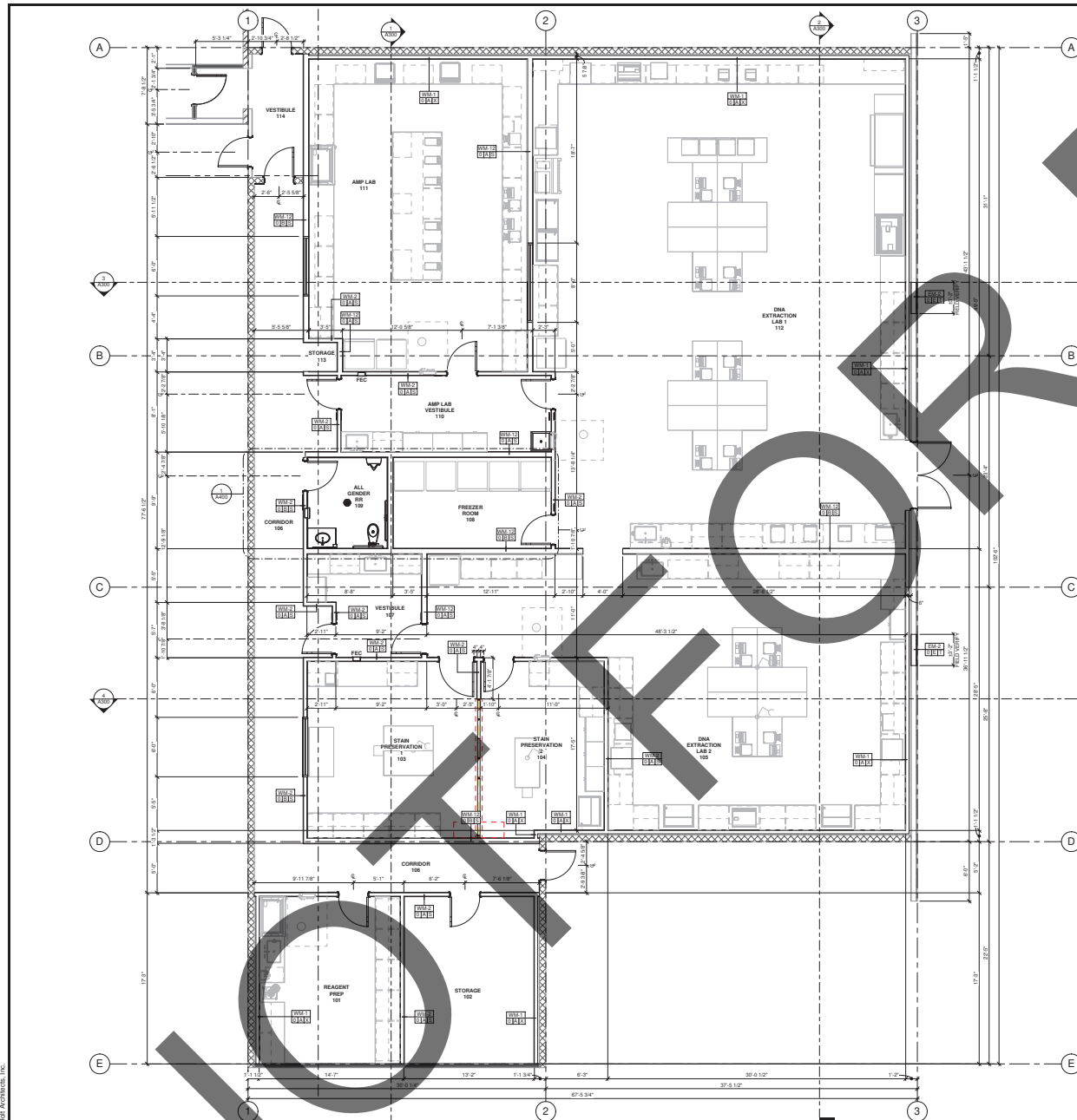
**SAN BERNARDINO COUNTY
SHERIFF'S CRIME LAB**
200 SOUTH LENA RD.
SAN BERNARDINO, CA

DATE	DESCRIPTION

PROJECT NO.: SBC-001

SHEET TITLE:
**DEMOLITION
ROOF PLAN - LAB**

SHEET NO.:
AD140



FLOOR PLAN LEGEND

IMAGE	KEY	DESCRIPTION	REMARKS / COMMENTS
	(E) WALL TO REMAIN		
	(E) 1 HR RATED WALL TO REMAIN		
	(D) DOOR AND FRAME TO REMAIN		
	(F) FRAMED WALL WALL TYPE MARKER		REFER TO A601 WALL SCHEDULE
	(D) DOOR AND FRAME		REFER TO A600 DOOR SCHEDULE
	(W) WINDOW SYSTEM		REFER TO A600 WINDOW SCHEDULE
	CR - CARD READER		REFER TO ELECTRICAL FOR ADDITIONAL INFORMATION
	RECESSED FIRE EXTINGUISHER CABINET		REFER TO XXX

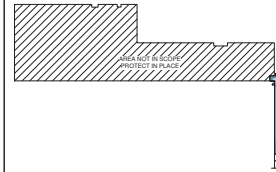
FLOOR PLAN GENERAL NOTES

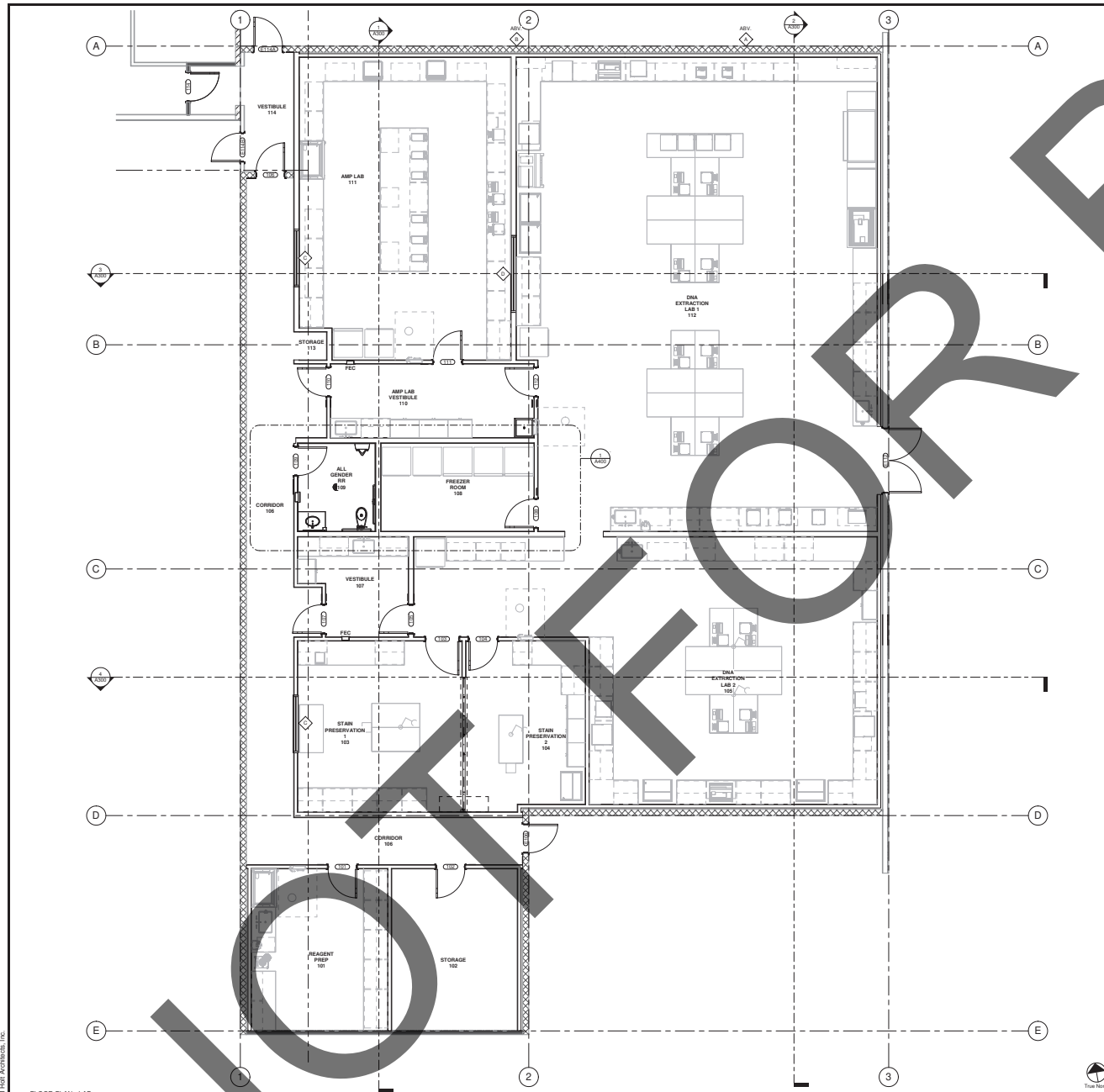
- ALL DIMENSIONS ARE TO FACE OF EXISTING WALLS, AND FACE OF NEW METAL STUD FRAMING UNLESS NOTED OTHERWISE ON PLANS.
- CONTRACTOR TO PROVIDE BACKING REINFORCEMENT FOR FURNITURE & EQUIPMENT IN WALLS & CEILING LOCATIONS TYPICAL.
- CONTRACTOR TO BRACE PARTITION WALLS TO UNDERLIE OF DECK. CONTRACTOR SHALL NOT BRACE AT STRUCTURAL BEAMS. REFER TO DETAIL XXX FOR TYPICAL PARTIAL HEIGHT PARTITION WALL BRACING INFORMATION.
- ALL EXISTING CONDITIONS TO REMAIN UNLESS NOTED OTHERWISE.
- CONTRACTOR TO VERIFY ALL EXISTING CONDITIONS IN FIELD.
- GENERAL CONTRACTOR SHALL COORDINATE WITH DRIPWALL CONTRACTOR TO MAKE SURE THEY REFER TO ELECTRICAL, MECHANICAL, & PLUMBING PLANS FOR ANY ADDITIONAL DRIPWALL PATCH REQUIRED DUE TO NEW INFRASTRUCTURE IN EXISTING WALLS TO REMAIN AS INDICATED ON THE FLOOR PLAN. GENERAL CONTRACTOR IS RESPONSIBLE TO ACQUAINT THEMSELVES WITH THE EXISTING CONDITIONS PRIOR TO BID. COUNTY WILL NOT BE RESPONSIBLE FOR ANY NEGLIGENCE BY THE GENERAL CONTRACTOR & OR SUB CONTRACTORS.
- ALL VOIDS, CRACKS, & JOINTS AROUND DOOR FRAMES, JAMBES, DUCTS, & OUTLETS MUST BE FILLED WITH INSULATION AND SEALED WITH SPOUNING AND/OR CAULKING AND PAINTED.
- PAIN ALL EXPOSED PLUMBING, MECHANICAL, & OTHER ITEMS WITHOUT FACTORY FINISH SUCH AS CONDUITS, PIPES, ACCESS PANELS AND ITEMS OF A SIMILAR NATURE TO MATCH ADJACENT SURFACE FINISH UNLESS NOTED OTHERWISE.
- FIRE EXTINGUISHERS SHALL BE PLACED AS REQUIRED BY THE FIRE MARSHALL. INSTALL PORTABLE FIRE EXTINGUISHERS PER WPA PARAPART 4.6, BUT NOT LESS THAN 2106 IN WRITING. REFER TO LAB EQUIPMENT PLANS FOR COORDINATION AND ADDITIONAL INFORMATION WITH CASEWORK / EQUIPMENT.
- ALL DOORS ARE TO BE LOCATED 4" FROM FACE OF ADJACENT WALL STUD UNLESS DIMENSIONED OTHERWISE ON PLANS.

FLOOR PLAN KEYNOTES

NOT USED

KEY PLAN





FLOOR PLAN LEGEND

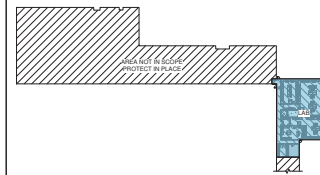
IMAGE	KEY	DESCRIPTION	REMARKS / COMMENTS
	(E) WALL TO REMAIN		
	(E) 1 HR RATED WALL TO REMAIN		
	RECESSED FIRE EXTINGUISHER CABINET		
	(N) FINISHED WALL		REFER TO A601 WALL SCHEDULE
	(N) DOOR AND FRAME		REFER TO A605 DOOR SCHEDULE
	(N) WINDOW SYSTEM		REFER TO A605 WINDOW SCHEDULE
	OR - CARD READER		REFER TO ELECTRICAL FOR ADDITIONAL INFORMATION
	RECESSED FIRE EXTINGUISHER CABINET		REFER TO XXX

FLOOR PLAN GENERAL NOTES

- CONTRACTOR TO PROVIDE BACKING REINFORCEMENT FOR FURNITURE & EQUIPMENT IN WALLS & CEILING LOCATIONS TYPICAL.
- CONTRACTOR TO BRACE PARTITION WALLS TO UNDERSIDE OF DECK. CONTRACTOR SHALL NOT BRACE AT STRUCTURAL BEAMS. REFER TO DETAIL XXX FOR TYPICAL PARTIAL HEIGHT PARTITION WALL BRACING INFORMATION.
- ALL EXISTING CONDITIONS TO REMAIN UNLESS NOTED OTHERWISE.
- CONTRACTOR TO VERIFY ALL EXISTING CONDITIONS IN FIELD.
- GENERAL CONTRACTOR SHALL COORDINATE WITH SUPPLIER CONTRACTOR TO MAKE SURE THEY REFER TO ELECTRICAL, MECHANICAL & PLUMBING PLANS FOR ANY ADDITIONAL DRYWALL PATCH REQUIRED DUE TO NEW INFRASTRUCTURE. REPAIRS AND WALLS TO REMAIN ARE LOCATED ON THE FLOOR PLAN. GENERAL CONTRACTOR IS RESPONSIBLE TO ACCURATELY THEMSELVES WITH THE EXISTING CONDITIONS PRIOR TO BID. COUNTY WILL NOT BE RESPONSIBLE FOR ANY NEGLIGENCE BY THE GENERAL CONTRACTOR & OR SUB CONTRACTORS.
- ALL VOIDS, CRACKS & JOINTS AROUND DOOR FRAMES, JAMBIE DUCTS, & OUTLETS MUST BE FILLED WITH INSULATION AND SEALED WITH SPACKLING AND/OR CAULKING AND PAINTED.
- PAIN ALL EXPOSED PLUMBING, MECHANICAL & OTHER ITEMS WITHOUT FACTORY FINISH SUCH AS CONDENS, PIPES, ACCESS PANELS AND TENDS OF A BREAKAWAY NATURE TO MATCH ADJACENT SURFACE FINISH UNLESS NOTED OTHERWISE.
- FIRE EXTINGUISHERS SHALL BE PLACED AS REQUIRED BY THE FIRE MARSHALL. INSTALL PORTABLE FIRE EXTINGUISHERS PER NFPA PAMPHLET 60, BUT NOT LESS THAN 2100C IN PLATING. REFER TO LAB EQUIPMENT PLANS FOR COORDINATION AND ADDITIONAL INFORMATION WITH CASEWORK / EQUIPMENT.

FLOOR PLAN KEYNOTES

KEY PLAN





RCP LEGEND		
IMAGE	KEY	DESCRIPTION
	(E) WALL TO REMAIN	
	(E) 1 HR RATED WALL TO REMAIN	
	(E) SUSPENDED CEILING TO REMAIN, PAINT TO BE IN PLACE	
	(E) FIRE SPRINKLER HEAD TO REMAIN, PROTECT IN PLACE	
	ACoustical CEILING TILE & GRID	REFER TO INTERIOR MATERIAL SCHEDULE FOR ADDITIONAL INFORMATION
	HARD LID CEILING	PAINT FINISH, REFER TO INTERIOR MATERIAL SCHEDULE FOR COLOR
	24" X 24" ACCESS PANEL	REFER TO XXX
	HVAC DIFFUSER	REFER TO MECHANICAL FOR ADDITIONAL INFORMATION
	ARTICULATING TASK ARM LIGHT	REFER TO MECHANICAL FOR ADDITIONAL INFORMATION
	LIGHT FIXTURE	REFER TO ELECTRICAL FOR ADDITIONAL INFORMATION
	OVERHEAD CHORD REEL (OCHR)	REFER TO ELECTRICAL FOR ADDITIONAL INFORMATION

RCP GENERAL NOTES

1. FIRE SPRINKLER HEAD PLACEMENT SHALL BE LOCATED IN THE CENTER OF 2'-0" BY 4'-0" ACoustical CEILING TILE TYPICAL THROUGHOUT PROJECT. ANY PLACEMENT OF HEAD THAT CAN NOT MEET REQUIREMENTS SHALL BE INDENTIFIED AND MUST GET APPROVAL FROM ARCHITECT PRIOR TO INSTALLATION.

RCP KEYNOTES

KEY PLAN

HOLT ARCHITECTURE
18051 COOK ST. SUITE 100
PALM DESERT, CA 92211
PH: 760.335.0300
WWW.HOLTARCHITECTURE.COM

SAN BERNARDINO COUNTY SHERIFF'S CRIME LAB
200 SOUTH LENA RD.
SAN BERNARDINO, CA

PROJECT

NO.	DATE	DESCRIPTION

PROJECT NO. - SBC-001

SHEET TITLE

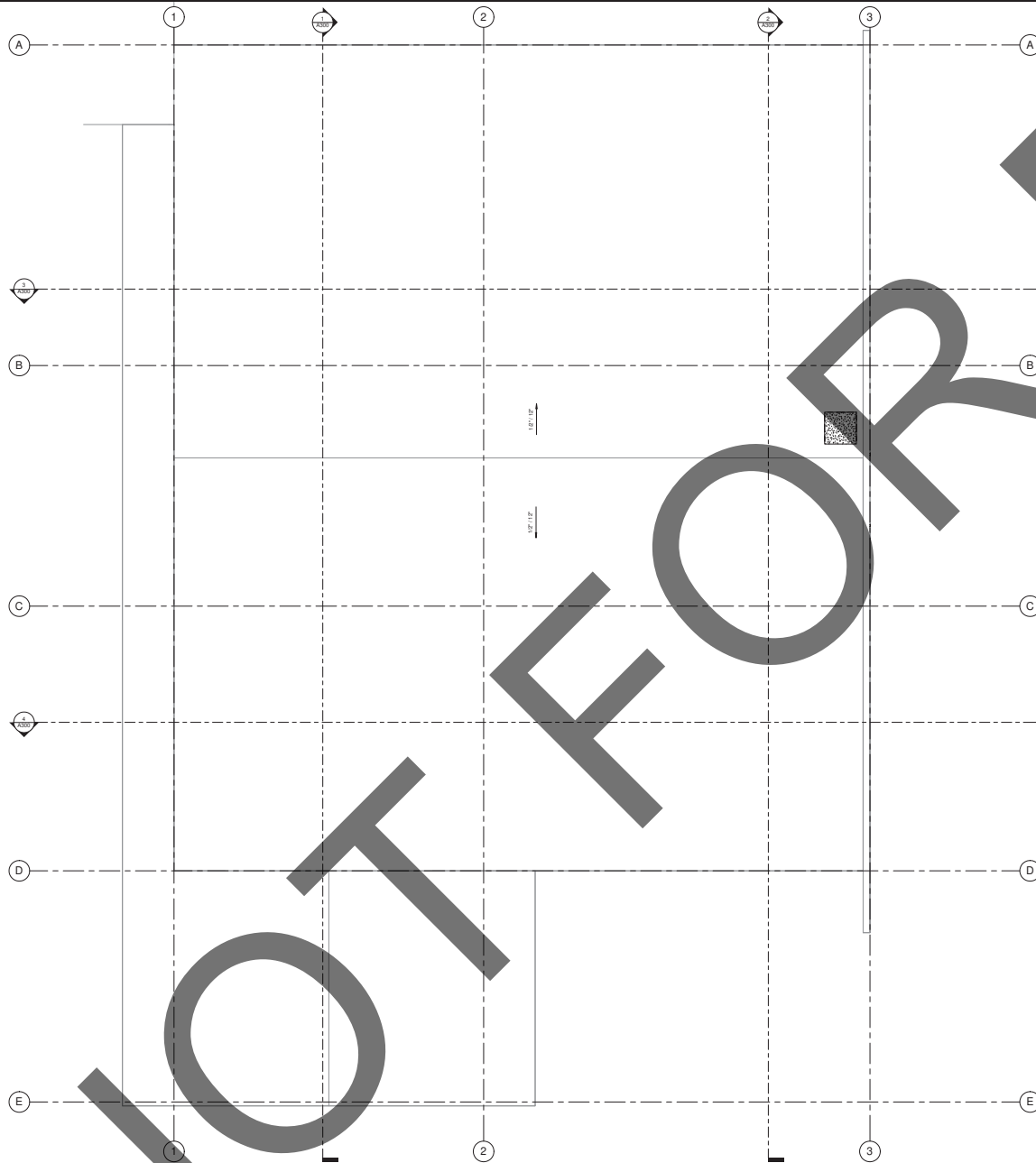
REFLECTED CEILING PLAN - LAB

SHEET NO.

A120

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ROOF PLAN - LAB
1/4" = 1'-0"



ROOF PLAN LEGEND

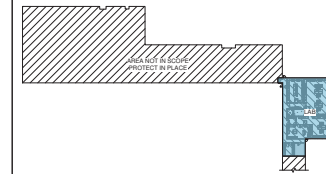
IMAGE	KEY	DESCRIPTION	REMARKS / COMMENTS
		ROOF PATCH	REFER TO 2001 SPEC FOR ADDITIONAL INFORMATION

ROOF PLAN GENERAL NOTES

1. PATCH AND REPAIR ROOF TO MATCH EXISTING ROOF WARRANTY. TYPICAL.

ROOF PLAN KEYNOTES

KEY PLAN



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CONTACT: (951) 261-1111

STAGE:

SAN BERNARDINO COUNTY
SHERIFF'S CRIME LAB
200 SOUTH LENA RD.
SAN BERNARDINO, CA

PROJECT

CURT

REVISION	DATE	DESCRIPTION

PROJECT NO. - SBC-001

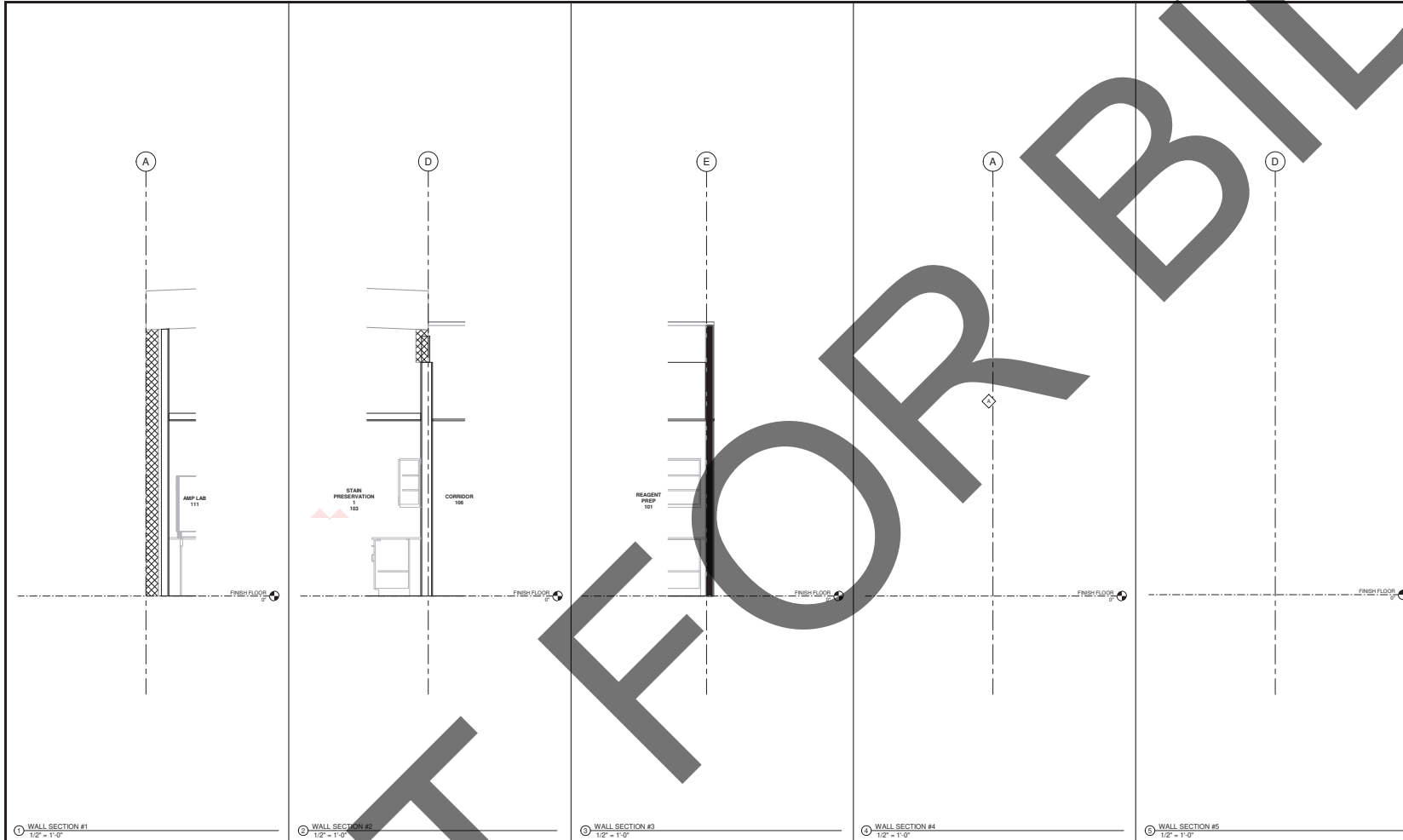
SHEET TITLE
ROOF PLAN - LAB

SHEET NO.

A140

**SAN BERNARDINO COUNTY
SHERIFF'S CRIME LAB**
200 SOUTH LENA RD.
SAN BERNARDINO, CA

NO.	DATE	DESCRIPTION



① WALL SECTION #1
1/2" = 1'-0"

② WALL SECTION #2
1/2" = 1'-0"

③ WALL SECTION #3
1/2" = 1'-0"

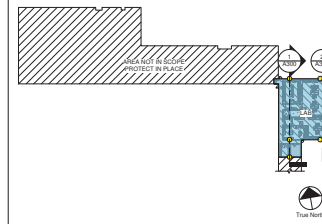
④ WALL SECTION #4
1/2" = 1'-0"

⑤ WALL SECTION #5
1/2" = 1'-0"

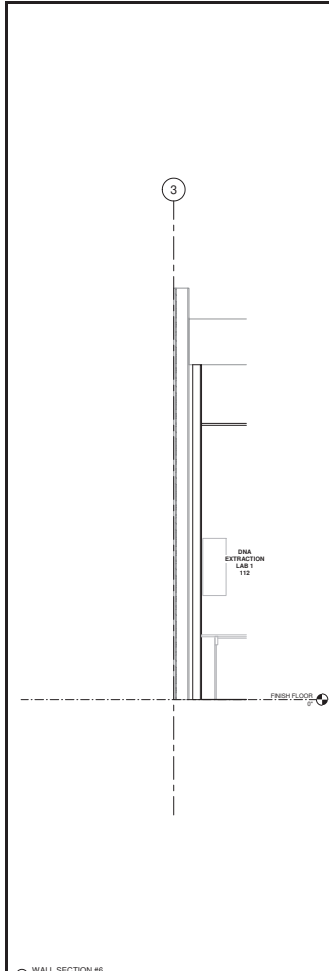
SECTION GENERAL NOTES

SECTION KEYNOTES

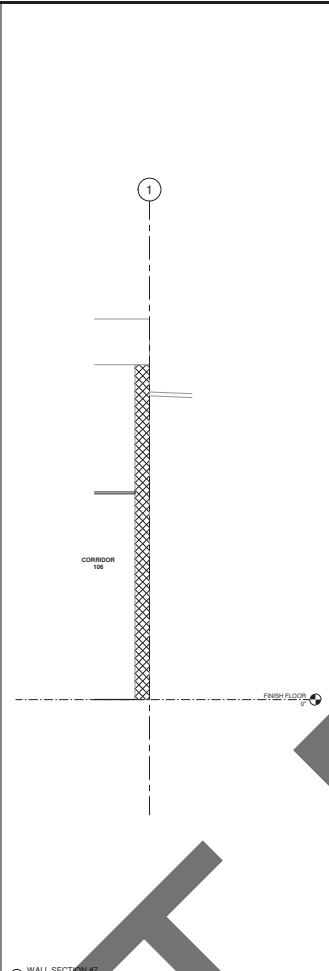
KEY PLAN



NOT USED



⑥ WALL SECTION #6
1/2" = 1'-0"

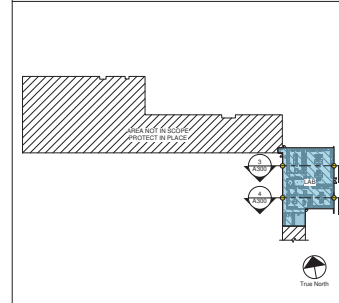


⑦ WALL SECTION #7
1/2" = 1'-0"

SECTION GENERAL NOTES

SECTION KEYNOTES

KEY PLAN



200 SOUTH LENA RD.
SAN BERNARDINO, CA

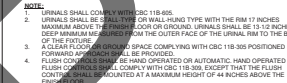
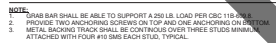
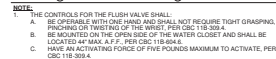
A400



① ENLARGED PLAN - FREEZER ROOM 108 & ALL GENDER RR 109
1/2" = 1'-0"



Town of Newbury



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STAMPS

1

[illegible]

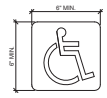
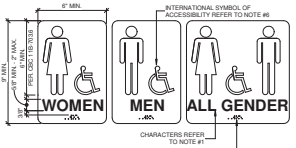
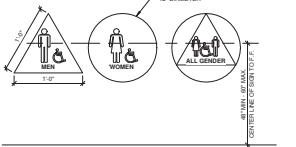
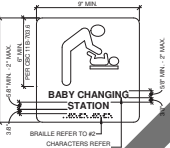

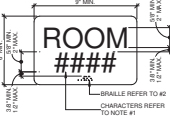
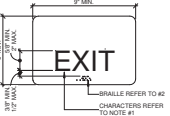
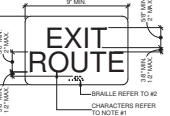

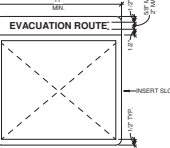
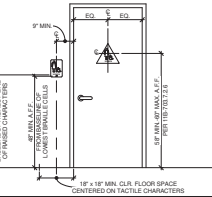
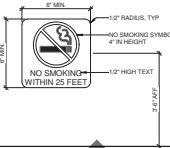
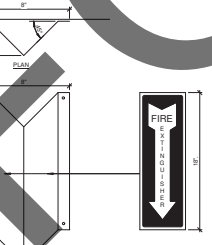


PROJECT NO.: SBC.002


SHEET TITLE:

**TYPICAL
ACCESSIBILITY
DETAILS**

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A500

 <p>NOTES:</p> <ol style="list-style-type: none"> 1. SIGN SHALL COMPLY WITH CBC 11B-703.2.1. 2. EDGES OF SIGN SHALL COMPLY WITH CBC 11B-703.2.6.4. 	 <p>NOTES:</p> <ol style="list-style-type: none"> 1. RAISED CHARACTERS SHALL COMPLY WITH CBC 11B-703.2 AND SHALL BE DUPLICATED IN BRaille COMPLYING WITH CBC 11B-703.3. 2. BRaille SHALL BE CONTRACTED (GRADE 3) AND SHALL COMPLY WITH CBC 11B-703.3. 3. SIGNS WITH TACTILE CHARACTERS SHALL BE INSTALLED IN ACCORDANCE WITH CBC 11B-703.4. 4. ALL VISUAL CHARACTERS SHALL COMPLY WITH CBC 11B-703.5. 5. CHARACTERS AND THEIR BACKGROUND SHALL HAVE A NON-GLARE FINISH, AND SHALL CONTRAST WITH THEIR BACKGROUND PER CBC 11B-703.5.1. 6. EDGES OF SIGN SHALL COMPLY WITH CBC 11B-703.2.6.4. 7. ALL SINGLE-USER RESTROOMS SHALL BE IDENTIFIED AS "ALL GENDER" PER CALIFORNIA HEALTH & SAFETY CODE SECTION 118600. 	 <p>NOTES:</p> <ol style="list-style-type: none"> 1. MEN'S SIGN: AN EQUILATERAL TRIANGLE, 1/4" THICK WITH EDGES 12" LONG. THE TRIANGLE SYMBOL SHALL CONTRAST WITH THE DOOR PER CBC 11B-703.2.6.1. 2. WOMEN'S SIGN: A CIRCLE, 1/4" THICK AND 12" IN DIAMETER. THE CIRCLE SYMBOL SHALL CONTRAST WITH THE DOOR PER CBC 11B-703.2.6.2. 3. ALL GENDER SIGN: A CIRCLE, 1/4" THICK AND 12" IN DIAMETER WITH A 1/4" THICK TRIANGLE SUPERIMPOSED AND GEOMETRICALLY INSCRIBED WITHIN THE CIRCLE AND WITHIN THE 12" DIAMETER. THE TRIANGLE SYMBOL SHALL CONTRAST WITH THE CIRCLE SYMBOL. THE CIRCLE SYMBOL SHALL CONTRAST WITH THE DOOR PER CBC 11B-703.2.6.3. 4. EDGES SHALL BE EASED OR ROUNDED AT 1/16" MIN. OR CHAMFERED AT 1/8" MAX. VERTICES SHALL BE ROUNDED BETWEEN 1/4" MIN. AND 1/4" MAX. 11B-703.2.6.4. 5. ALL SINGLE-USER RESTROOMS SHALL BE IDENTIFIED AS "ALL GENDER" PER CALIFORNIA HEALTH & SAFETY CODE SECTION 118600. 	 <p>NOTES:</p> <ol style="list-style-type: none"> 1. RAISED CHARACTERS SHALL COMPLY WITH CBC 11B-703.2 AND SHALL BE DUPLICATED IN BRaille COMPLYING WITH CBC 11B-703.3. 2. BRaille SHALL BE CONTRACTED (GRADE 3) AND SHALL COMPLY WITH CBC 11B-703.3. 3. SIGNS WITH TACTILE CHARACTERS SHALL BE INSTALLED IN ACCORDANCE WITH CBC 11B-703.4. 4. ALL VISUAL CHARACTERS SHALL COMPLY WITH CBC 11B-703.5. 5. CHARACTERS AND THEIR BACKGROUND SHALL HAVE A NON-GLARE FINISH, AND SHALL CONTRAST WITH THEIR BACKGROUND PER CBC 11B-703.5.1. 6. EDGES OF SIGN SHALL COMPLY WITH CBC 11B-703.2.6.4. 	 <p>NOTES:</p> <ol style="list-style-type: none"> 1. ALL VISUAL CHARACTERS SHALL COMPLY WITH CBC 11B-703.5. 2. CHARACTERS AND THEIR BACKGROUND SHALL HAVE A NON-GLARE FINISH, AND SHALL CONTRAST WITH THEIR BACKGROUND PER CBC 11B-703.5.1. 3. EDGES OF SIGN SHALL COMPLY WITH CBC 11B-703.2.6.4.
<p>INTERNATIONAL SYMBOL OF ACCESSIBILITY 3" x 1'-0"</p>  <p>NOTES:</p> <ol style="list-style-type: none"> 1. RAISED CHARACTERS SHALL COMPLY WITH CBC 11B-703.2 AND SHALL BE DUPLICATED IN BRaille COMPLYING WITH CBC 11B-703.3. 2. BRaille SHALL BE CONTRACTED (GRADE 3) AND SHALL COMPLY WITH CBC 11B-703.3. 3. SIGNS WITH TACTILE CHARACTERS SHALL BE INSTALLED IN ACCORDANCE WITH CBC 11B-703.4. 4. ALL VISUAL CHARACTERS SHALL COMPLY WITH CBC 11B-703.5. 5. CHARACTERS AND THEIR BACKGROUND SHALL HAVE A NON-GLARE FINISH, AND SHALL CONTRAST WITH THEIR BACKGROUND PER CBC 11B-703.5.1. 6. EDGES OF SIGN SHALL COMPLY WITH CBC 11B-703.2.6.4. 7. ALL SINGLE-USER RESTROOMS SHALL BE IDENTIFIED AS "ALL GENDER" PER CALIFORNIA HEALTH & SAFETY CODE SECTION 118600. 	<p>RESTROOM SIGNAGE - WALL MOUNTED 3" x 1'-0"</p>  <p>NOTES:</p> <ol style="list-style-type: none"> 1. RAISED CHARACTERS SHALL COMPLY WITH CBC 11B-703.2 AND SHALL BE DUPLICATED IN BRaille COMPLYING WITH CBC 11B-703.3. 2. BRaille SHALL BE CONTRACTED (GRADE 3) AND SHALL COMPLY WITH CBC 11B-703.3. 3. SIGNS WITH TACTILE CHARACTERS SHALL BE INSTALLED IN ACCORDANCE WITH CBC 11B-703.4. 4. ALL VISUAL CHARACTERS SHALL COMPLY WITH CBC 11B-703.5. 5. CHARACTERS AND THEIR BACKGROUND SHALL HAVE A NON-GLARE FINISH, AND SHALL CONTRAST WITH THEIR BACKGROUND PER CBC 11B-703.5.1. 6. EDGES OF SIGN SHALL COMPLY WITH CBC 11B-703.2.6.4. 	<p>RESTROOM SIGNAGE - DOOR MOUNTED 1 1/2" x 1'-0"</p>  <p>NOTES:</p> <ol style="list-style-type: none"> 1. RAISED CHARACTERS SHALL COMPLY WITH CBC 11B-703.2 AND SHALL BE DUPLICATED IN BRaille COMPLYING WITH CBC 11B-703.3. 2. BRaille SHALL BE CONTRACTED (GRADE 3) AND SHALL COMPLY WITH CBC 11B-703.3. 3. SIGNS WITH TACTILE CHARACTERS SHALL BE INSTALLED IN ACCORDANCE WITH CBC 11B-703.4. 4. ALL VISUAL CHARACTERS SHALL COMPLY WITH CBC 11B-703.5. 5. CHARACTERS AND THEIR BACKGROUND SHALL HAVE A NON-GLARE FINISH, AND SHALL CONTRAST WITH THEIR BACKGROUND PER CBC 11B-703.5.1. 6. EDGES OF SIGN SHALL COMPLY WITH CBC 11B-703.2.6.4. 	<p>BABY CHANGING STATION SIGN 3" x 1'-0"</p>  <p>NOTES:</p> <ol style="list-style-type: none"> 1. RAISED CHARACTERS SHALL COMPLY WITH CBC 11B-703.2 AND SHALL BE DUPLICATED IN BRaille COMPLYING WITH CBC 11B-703.3. 2. BRaille SHALL BE CONTRACTED (GRADE 3) AND SHALL COMPLY WITH CBC 11B-703.3. 3. SIGNS WITH TACTILE CHARACTERS SHALL BE INSTALLED IN ACCORDANCE WITH CBC 11B-703.4. 4. ALL VISUAL CHARACTERS SHALL COMPLY WITH CBC 11B-703.5. 5. CHARACTERS AND THEIR BACKGROUND SHALL HAVE A NON-GLARE FINISH, AND SHALL CONTRAST WITH THEIR BACKGROUND PER CBC 11B-703.5.1. 6. EDGES OF SIGN SHALL COMPLY WITH CBC 11B-703.2.6.4. 	<p>OCCUPANCY MAXIMUM 3" x 1'-0"</p>  <p>NOTES:</p> <ol style="list-style-type: none"> 1. ALL VISUAL CHARACTERS SHALL COMPLY WITH CBC 11B-703.5. 2. CHARACTERS AND THEIR BACKGROUND SHALL HAVE A NON-GLARE FINISH, AND SHALL CONTRAST WITH THEIR BACKGROUND PER CBC 11B-703.5.1 (BLACK OR RED). 3. EDGES OF SIGN SHALL COMPLY WITH CBC 11B-703.2.6.4. 4. SIGN FRAME TO HOLD A 8 1/2" x 11" INSERT MAP. (INSERT PROVIDED BY OWNER). 5. SIGN FRAME TO HAVE A TRANSPARENT CENTER TO VIEW INSERT. 6. CONTRACTOR SHALL BE RESPONSIBLE FOR DEVELOPING ALL EVACUATION PLAN INSERTS. TYP. AND SUBMIT TO ADR FOR REVIEW AND APPROVAL.
<p>TACTILE ROOM IDENTIFICATION SIGN 3" x 1'-0"</p>  <p>NOTES:</p> <ol style="list-style-type: none"> 1. ALL TACTILE SIGNAGE SHALL COMPLY WITH CBC 11B-703.4.2. 2. AT DOUBLE DOORS WITH ONE ACTIVE LEAF, THE SIGN SHALL BE LOCATED ON THE ACTIVE LEAF PER CBC 11B-703.4.2. 3. AT DOUBLE DOORS WITH TWO ACTIVE LEAFS, THE SIGN SHALL BE LOCATED ON THE RIGHT OF THE RIGHT HAND DOOR PER CBC 11B-703.4.2. 4. WHERE THERE IS NO WALL SPACE AT THE LATCH SIDE OF A SINGLE DOOR OR AT THE RIGHT OF DOUBLE DOORS, SIGN SHALL BE LOCATED ON THE NEAREST ADJACENT WALL PER CBC 11B-703.4.2. 5. SIGNS IDENTIFYING PERMANENT ROOMS AND SPACES SHALL BE LOCATED AT THE ENTRANCE TO, AND OUTSIDE OF THE ROOM OR SPACE PER CBC 11B-703.4.2. 6. SIGNS IDENTIFYING EXITS SHALL BE LOCATED AT THE EXIT DOOR WHEN APPROACHED IN THE DIRECTION OF EGRESS TRAVEL. 11B-703.4.2. 	<p>TACTILE EXIT SIGN 3" x 1'-0"</p>  <p>NOTES:</p> <ol style="list-style-type: none"> 1. ALL VISUAL CHARACTERS SHALL COMPLY WITH CBC 11B-703.5. 2. CHARACTERS AND THEIR BACKGROUND SHALL HAVE A NON-GLARE FINISH, AND SHALL CONTRAST WITH THEIR BACKGROUND PER CBC 11B-703.5.1. 3. EDGES OF SIGN SHALL COMPLY WITH CBC 11B-703.2.6.4. 	<p>TACTILE EXIT SIGN - EXIT ROUTE 3" x 1'-0"</p>  <p>NOTES:</p> <ol style="list-style-type: none"> 1. SIGN SHALL INCLUDE WORKING THAT STATES "ASSISTIVE LISTENING SYSTEM AVAILABLE" AND SHALL BE POSTED IN A PROMINENT PLACE AT OR NEAR THE ASSEMBLY AREA ENTRANCE PER SECTION 11B-703.10. 2. ASSISTIVE LISTENING SIGNS SHALL COMPLY WITH SECTION 11B-703.10 AND SHALL INCLUDE THE INTERNATIONAL SYMBOL OF ACCESS FOR HEARING LOSS COMPLYING WITH SECTION 11B-703.2.4. 3. PICTOGRAMS SHALL BE ACCOMPANIED BY THE EQUIVALENT VERBAL DESCRIPTION WITH SECTION 11B-703.2.4. 4. PICTOGRAMS SHALL BE LOCATED DIRECTLY BELOW THE PICTOGRAM. 5. THE BORDER DIMENSION OF THE PICTOGRAM SHALL BE 6" MINIMUM IN HEIGHT. 6. PICTOGRAMS AND THEIR FIELD SHALL HAVE A NON-GLARE FINISH. PICTOGRAMS SHALL CONTRAST WITH THEIR FIELD WITH EITHER A LIGHT PICTOGRAM ON A DARK FIELD OR A DARK PICTOGRAM ON A LIGHT FIELD PER SECTION 11B-703.5.1. 7. ASSISTIVE LISTENING SYSTEM SHALL COMPLY WITH CBC SECTION 11B-703.10. 	<p>TACTILE EXIT SIGN - TO EXIT 3" x 1'-0"</p>  <p>NOTES:</p> <ol style="list-style-type: none"> 1. SIGN SHALL INCLUDE WORKING THAT STATES "ASSISTIVE LISTENING SYSTEM AVAILABLE" AND SHALL BE POSTED IN A PROMINENT PLACE AT OR NEAR THE ASSEMBLY AREA ENTRANCE PER SECTION 11B-703.10. 2. ASSISTIVE LISTENING SIGNS SHALL COMPLY WITH SECTION 11B-703.10 AND SHALL INCLUDE THE INTERNATIONAL SYMBOL OF ACCESS FOR HEARING LOSS COMPLYING WITH SECTION 11B-703.2.4. 3. PICTOGRAMS SHALL BE ACCOMPANIED BY THE EQUIVALENT VERBAL DESCRIPTION WITH SECTION 11B-703.2.4. 4. PICTOGRAMS SHALL BE LOCATED DIRECTLY BELOW THE PICTOGRAM. 5. THE BORDER DIMENSION OF THE PICTOGRAM SHALL BE 6" MINIMUM IN HEIGHT. 6. PICTOGRAMS AND THEIR FIELD SHALL HAVE A NON-GLARE FINISH. PICTOGRAMS SHALL CONTRAST WITH THEIR FIELD WITH EITHER A LIGHT PICTOGRAM ON A DARK FIELD OR A DARK PICTOGRAM ON A LIGHT FIELD PER SECTION 11B-703.5.1. 7. ASSISTIVE LISTENING SYSTEM SHALL COMPLY WITH CBC SECTION 11B-703.10. 	<p>EVACUATION MAP FRAME / SIGN 3" x 1'-0"</p>  <p>NOTES:</p> <ol style="list-style-type: none"> 1. PROVIDE 1" HIGH LETTERING IN CONTRASTING COLOR.
<p>SIGNAGE MOUNTING AND LOCATION 1 1/2" x 1'-0"</p> <p>NOT USED</p>	<p>NO SMOKING SIGN 3" x 1'-0"</p> <p>NOT USED</p>	<p>FIRE EXTINGUISHER - THREE WAY SIGN 3" x 1'-0"</p> <p>NOT USED</p>	<p>ASSISTIVE LISTENING SYSTEM (ALS) SIGN 3" x 1'-0"</p> <p>NOT USED</p>	<p>MAIN EXIT SIGN 3" x 1'-0"</p> <p>NOT USED</p>



HOLT ARCHITECTURE
1000 COOK ST. STE. 1000 SAN BERNARDINO, CA 92401
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DATE: 1/21/21

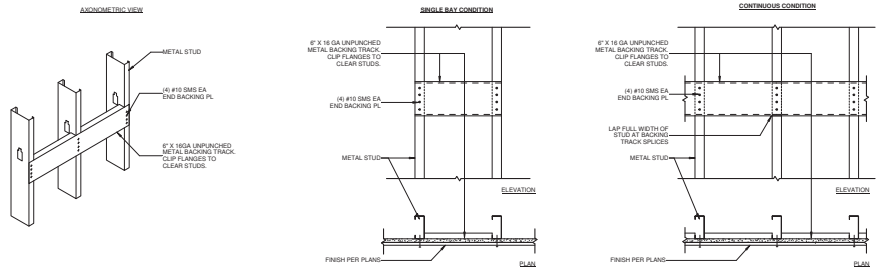
PROJECT: SAN BERNARDINO COUNTY SHERIFF'S CRIME LAB

200 SOUTH LEONA RD.
SAN BERNARDINO, CA

PROJECT NO.: SBC-002

SHEET TITLE: TYPICAL SIGNAGE DETAILS

SHEET NO.: A501



NOTE:
IN USE OF TRACK, A 6" X 16GA FLAT STRAP MAY BE USED TO SUPPORT LOADS UP TO 20 LB/FT WITH A MAXIMUM CONCENTRATED LOAD OF 50 LBS. STRAP SHALL BE CONTINUOUS OVER THREE STUDS MINIMUM. ATTACH AS NOTED ABOVE.

1 TYPICAL METAL BACKING DETAIL
1 1/2" = 1'-0"

PLACEHOLDER

1 TYPICAL WALL FURF OUT
1 1/2" = 1'-0"

PLACEHOLDER

2 FLOOR DRAIN AT DEPRESSED SLAB
1 1/2" = 1'-0"

PLACEHOLDER

3 FIRE EXTINGUISHER CABINET
1 1/2" = 1'-0"

PLACEHOLDER

7 CEILING ACCESS PANEL
1 1/2" = 1'-0"

PLACEHOLDER

8 ROOF ACCESS LADDER
1 1/2" = 1'-0"

PLACEHOLDER

9 HSS COLUMN IN WALL
1 1/2" = 1'-0"

PLACEHOLDER

10 FLOOR DRAIN AT NON-DEPRESSED SLAB
1 1/2" = 1'-0"

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PALM DESERT, CA 92011
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PROJECT NAME:

STAGE:

**SAN BERNARDINO COUNTY
SHERIFF'S CRIME LAB**
200 SOUTH LENA RD.
SAN BERNARDINO, CA

PROJECT:

CURT:

REVISION	DATE	DESCRIPTION

PROJECT NO.: SRC-001

SHEET TITLE:
**TYPICAL INTERIOR
DETAILS**

SHEET NO.:
A510

PLACEHOLDER
① TYPICAL EXTERIOR DOOR - HEAD
1 1/2" x 1'-0"

PLACEHOLDER
② TYPICAL EXTERIOR DOOR - JAMB
1 1/2" x 1'-0"

PLACEHOLDER
③ TYPICAL EXTERIOR DOOR - THRESHOLD
1 1/2" x 1'-0"

PLACEHOLDER
④ DOOR ACCESS CONTROL DIAGRAM
1 1/2" x 1'-0"

PLACEHOLDER
⑤ TYPICAL INTERIOR DOOR - HEAD
1 1/2" x 1'-0"

PLACEHOLDER
⑥ TYPICAL INTERIOR DOOR - JAMB
1 1/2" x 1'-0"

PLACEHOLDER
⑦ TYPICAL INTERIOR DOOR - THRESHOLD
1 1/2" x 1'-0"

PLACEHOLDER
⑧ TYPICAL EXTERIOR WINDOW - HEAD
1 1/2" x 1'-0"

PLACEHOLDER
⑨ TYPICAL EXTERIOR WINDOW - JAMB
1 1/2" x 1'-0"

PLACEHOLDER
⑩ TYPICAL EXTERIOR WINDOW - SILL
1 1/2" x 1'-0"

PLACEHOLDER
⑪ TYPICAL INTERIOR WINDOW - HEAD
1 1/2" x 1'-0"

PLACEHOLDER
⑫ TYPICAL INTERIOR WINDOW - JAMB
1 1/2" x 1'-0"

PLACEHOLDER
⑬ TYPICAL INTERIOR WINDOW - SILL
1 1/2" x 1'-0"






8. CEILING GRID MEMBER ATTACHED TO WALL AT TWO ADJACENT WALLS ONLY. REFER TO CEILING GENERAL NOTES #10, #11, #14 AND DETAIL 6 / A509.
9. CEILING GRID MEMBER NOT ATTACHED AT OPPOSITE WALLS. REFER TO ACT CEILING GENERAL NOTES #10, #11, #14 AND DETAIL 6 / A509.
10. HANGER WIRE AND BRACING WIRE ATTACHMENT TO STRUCTURE. REFER TO ACT CEILING GENERAL NOTE #3, #4 AND DETAIL 11 / A509.
11. SAME CRITERIA AS NOTED ABOVE ARE APPLICABLE AT 24" x 24" GRID.



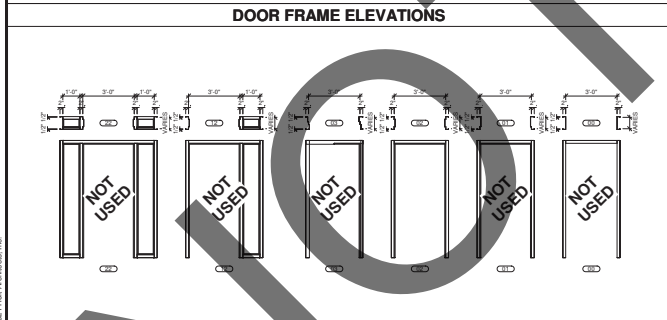
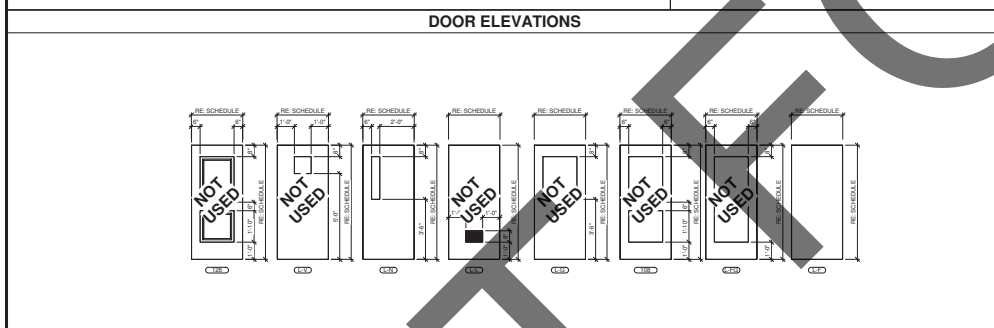
13 TYPICAL SADDLE TIE DETAIL



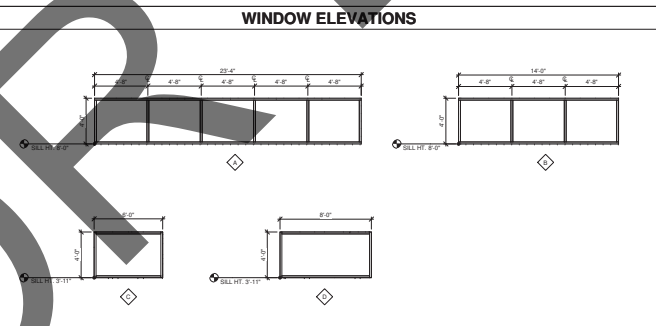
<p>PLACEHOLDER</p> <p>① TYPICAL EXISTING GYM WALL INFILL AT DOOR 1/16" = 1'-0"</p>	<p>PLACEHOLDER</p> <p>② TYPICAL EXISTING GYM WALL INFILL AT WINDOW 1/16" = 1'-0"</p>	<p>PLACEHOLDER</p> <p>③ TYPICAL EXISTING ROOF INFILL PATCH 1/16" = 1'-0"</p>	<p>PLACEHOLDER</p> <p>④ TYPICAL DIRECT PENETRATION THROUGH ROOF 1/16" = 1'-0"</p>	<p>PLACEHOLDER</p> <p>⑤ FLASHING AT DIRECT PENETRATION 1/16" = 1'-0"</p>					
<p>PLACEHOLDER</p> <p>⑥ TYPICAL SINGLE PIPE ROOF PENETRATION 1/16" = 1'-0"</p>	<p>PLACEHOLDER</p> <p>⑦ TYPICAL DOUBLE PIPE ROOF PENETRATION 1/16" = 1'-0"</p>	<p>PLACEHOLDER</p> <p>⑧ ROOF ACCESS HATCH 1/16" = 1'-0"</p>	<p>PLACEHOLDER</p> <p>⑨ FLASHING AT ROOF ACCESS HATCH 1/16" = 1'-0"</p>						
<table><tr><td data-bbox="1347 1812 1469 1969"><p>HOLT ARCHITECTURE 1000 WEST 10TH AVENUE SUITE 100 DENVER, CO 80202 303.733.1000 WWW.HOLT-ARCHITECTURE.COM</p></td><td data-bbox="1250 1812 1347 1969"><p>DATE: _____</p></td><td data-bbox="722 1812 1250 1969"><p>PROJECT: SAN BERNARDINO COUNTY SHERIFF'S CRIME LAB</p><p>200 SOUTH LENA RD. SAN BERNARDINO, CA</p></td><td data-bbox="462 1812 722 1969"><p>CLIENT: _____</p></td><td data-bbox="162 1812 462 1969"><p>SHEET TITLE: TYPICAL EXTERIOR DETAILS</p><p>PROJECT NO.: A520</p></td></tr></table>					 <p>HOLT ARCHITECTURE 1000 WEST 10TH AVENUE SUITE 100 DENVER, CO 80202 303.733.1000 WWW.HOLT-ARCHITECTURE.COM</p>	<p>DATE: _____</p>	<p>PROJECT: SAN BERNARDINO COUNTY SHERIFF'S CRIME LAB</p> <p>200 SOUTH LENA RD. SAN BERNARDINO, CA</p>	<p>CLIENT: _____</p>	<p>SHEET TITLE: TYPICAL EXTERIOR DETAILS</p> <p>PROJECT NO.: A520</p>
 <p>HOLT ARCHITECTURE 1000 WEST 10TH AVENUE SUITE 100 DENVER, CO 80202 303.733.1000 WWW.HOLT-ARCHITECTURE.COM</p>	<p>DATE: _____</p>	<p>PROJECT: SAN BERNARDINO COUNTY SHERIFF'S CRIME LAB</p> <p>200 SOUTH LENA RD. SAN BERNARDINO, CA</p>	<p>CLIENT: _____</p>	<p>SHEET TITLE: TYPICAL EXTERIOR DETAILS</p> <p>PROJECT NO.: A520</p>					

DOOR SCHEDULE														
LOCATION			LEAF INFORMATION					FRAME INFORMATION						
MARK	FROM	TO	LEAF TYPE	LEAF-1 TYPE	LEAF-1 WIDTH	LEAF-1 HEIGHT	THICKNESS	MATERIAL	FINISH	TYPE	MATERIAL	FINISH	HW SET	ELECTRIFIED
101	CORRIDOR	REAGENT PREP	L-N		2'-0"	8'-0"	1 3/4"	HM	PTFN-02	P-02	HM	PTFN-02		0
102	CORRIDOR	STORAGE	L-N		2'-0"	8'-0"	1 3/4"	HM	PTFN-02	P-02	HM	PTFN-02		0
103	OSM EXTRACTION LAB 2	STAIN PRESERVATION 2	L-N		2'-0"	8'-0"	1 3/4"	HM	PTFN-02	P-02	HM	PTFN-02		0
104	OSM EXTRACTION LAB 2	STAIN PRESERVATION 2	L-N		2'-0"	8'-0"	1 3/4"	HM	PTFN-02	P-02	HM	PTFN-02		0
105	OSM EXTRACTION LAB 2	VESTIBULE	L-N		2'-0"	8'-0"	1 3/4"	HM	PTFN-02	P-02	HM	PTFN-02		0
106	CORRIDOR	VESTIBULE	L-N		2'-0"	8'-0"	1 3/4"	HM	PTFN-02	P-02	HM	PTFN-02		0
107	VESTIBULE	CORRIDOR	L-N		2'-0"	8'-0"	1 3/4"	HM	PTFN-02	P-02	HM	PTFN-02		0
108	OSM EXTRACTION LAB 1	PREPZAR ROOM	L-N		2'-0"	8'-0"	1 3/4"	HM	PTFN-02	P-02	HM	PTFN-02		0
109	CORRIDOR	ALL GENDER FR	L-F		2'-0"	8'-0"	1 3/4"	HM	PTFN-02	P-02	HM	PTFN-02		0
110	CORRIDOR	ALL GENDER FR	L-F		2'-0"	8'-0"	1 3/4"	HM	PTFN-02	P-02	HM	PTFN-02		0
111	AMP LAB VESTIBULE	AMP LAB	L-N		2'-0"	8'-0"	1 3/4"	HM	PTFN-02	P-02	HM	PTFN-02		0
112	OSM EXTRACTION LAB 1	AMP LAB VESTIBULE	L-N		2'-0"	8'-0"	1 3/4"	HM	PTFN-02	P-02	HM	PTFN-02		0
113	STORAGE	CORRIDOR	L-F		2'-0"	8'-0"	1 3/4"	HM	PTFN-02	P-02	HM	PTFN-02		0
114	ADMIN OFFICES		L-N		2'-0"	8'-0"	1 3/4"	HM	PTFN-02	P-02	HM	PTFN-02		0
115	CORRIDOR		L-F		2'-0"	8'-0"	1 3/4"	HM	PTFN-02	P-02	HM	PTFN-02		0
117	OSM EXTRACTION LAB 1		L-F		2'-0"	8'-0"	1 3/4"	HM	PTFN-02	P-02	HM	PTFN-02		0
117A	VESTIBULE		L-N		2'-0"	8'-0"	1 3/4"	HM	PTFN-02	P-02	HM	PTFN-02		0
118	VESTIBULE		L-F		2'-0"	8'-0"	1 3/4"	HM	PTFN-02	P-02	HM	PTFN-02		0

ABBREVIATIONS	
GLAZING	EXTERIOR GLAZING - 1" INSULATED, TINTED, TEMPERED
G-1	INTERIOR GLAZING - 1/4" CLEAR, TEMPERED
G-2	INTERIOR GLAZING - 1/4" CLEAR, TEMPERED - FIRE RATED
G-3	INTERIOR GLAZING - 1/4" CLEAR, TEMPERED - SECURITY LAMINATED
G-4	INTERIOR GLAZING - 5/16" CLEAR, TEMPERED - SECURITY LAMINATED
MATERIAL	
HM	HOLLOW METAL
AL	ALUMINUM
SCW	SOLID CORE WOOD
SCW	SOLID CORE WOOD
SS	STAINLESS STEEL
GLV	GALVANIZED
FINISH	
CLAN	CLEAR ANODIZED
BLAN	BLACK ANODIZED
DMAN	DARK ANILINE ANODIZED
PTFN	PAINT FINISH
STAN	STAIN FINISH
PLAM	PLASTIC LAMINATE FINISH



WINDOW SCHEDULE									
MARK	WIDTH	HEIGHT	SILL HEIGHT	FRAME MATERIAL	FRAME FINISH	GLAZING TYPE	FINISH TREATMENT	OPERATING	COUNT
A	24'-1 1/4"	4'-0" @ 2'-0"	4'-0" @ 2'-0"						1
B	1'-0"	4'-0" @ 2'-0"	4'-0" @ 2'-0"						1
C	8'-0"	4'-0" @ 2'-0"	4'-0" @ 2'-0"						1
D	8'-0"	4'-0" @ 2'-0"	4'-0" @ 2'-0"						1



- ### DOOR GENERAL NOTES
- REFER TO DIMENSIONAL FLOOR PLANS FOR DOOR OPENING LOCATIONS. ALL DOORS ARE TO BE LOCATED 4" FROM FACE OF ADJACENT WALL STUD UNLESS DIMENSIONS OTHERWISE ON PLANS.
 - ALL EXITS TO BE OPERABLE FROM THE INSIDE WITHOUT THE USE OF A KEY OR ANY SPECIAL KNOWLEDGE OR EFFORT PER 2022 CBC SECTION 1010.1.9.
 - ALL DOORS, WINDOWS, AND OTHER JOINTS IN THE EXTERIOR WALLS OF A CLIMATE CONTROLLED SPACE SHALL BE FULLY INSULATED.
 - DOOR REQUIREMENTS: ALL HAND ACTIVATED LATCHING OR LOCKING DOORS SHALL HAVE EGRESS TYPE HARDWARE CENTERED EQUAL TO OR GREATER THAN 34" BUT EQUAL OR LESS THAN 48" FROM THE BOTTOM OF THE DOOR. MAXIMUM EFFORT REQUIRED TO OPEN DOORS SHALL BE EQUAL TO OR LESS THAN 34 LB AT ALL DOORS TO MINIMUM 90 DEGREES. WHERE FIRE DOORS ARE REQUIRED, THE MAXIMUM EFFORT TO OPERATE THE DOOR MAY BE REDUCED TO THE MINIMUM ALLOWABLE BY THE APPROPRIATE ADMINISTRATIVE AUTHORITY (RIVERSIDE COUNTY FIRE DEPARTMENT) NOT TO EXCEED 15 LB. COMPARTMENTAL DEVICES OF AUTOMATIC DOORS MAY BE UTILIZED TO MEET THE ABOVE STANDARDS.
 - DOORS SHALL BE OPERABLE WITH A SINGLE EFFORT.
 - LOWER 18" OF ALL DOORS TO BE SMOOTH AND UNINTERRUPTED TO ALLOW THE DOOR TO BE OPENED BY A WHEELCHAIR FOOTREST WITHOUT CREATING A TRAP OR HAZARDOUS CONDITION. PER CBC SECTION 101-4.04.2.10.
 - REFER TO SHEET G-102 FOR SCHEDULE REQUIREMENTS.
 - REFER TO EXTERIOR MATERIAL SCHEDULE, INTERIOR MATERIAL SCHEDULE, AND MATERIAL SCHEDULE - OPTIONS FOR ADDITIONAL FINISH INFORMATION.
 - DOOR, GATES, AND SIDE LITES ADJACENT TO DOORS OR GATES CONTAINING ONE OR MORE GLAZING PANELS THAT PERMIT VIEWING THROUGH THE PANELS, HAVE THE BOTTOM OF AT LEAST ONE GLAZED PANEL LOCATED 48" MAX. PER FINISH SECTION 101-4.04.2.1.
 - CONTRACTOR SHALL FIELD VERIFY AND COORDINATE FINAL DOOR AND WINDOW FRAME THROAT SIZES PRIOR TO ORDERING.
 - AT DOOR REQUIRED TO HAVE ELECTRIFIED HARDWARE, REFER TO DETAIL XXX FOR ADDITIONAL INFORMATION.

FRAME SCHEDULE				
FRAME TYPE	FRAME WIDTH	DOOR WIDTH	FRAME HEIGHT	DOOR HEIGHT
P-02	12'-10"	12'-0"	12'-0"	12'-0"
P-03	12'-0"	12'-0"	12'-0"	12'-0"
P-04	12'-0"	12'-0"	12'-0"	12'-0"
P-05	12'-0"	12'-0"	12'-0"	12'-0"
P-06	12'-0"	12'-0"	12'-0"	12'-0"
P-07	12'-0"	12'-0"	12'-0"	12'-0"
P-08	12'-0"	12'-0"	12'-0"	12'-0"
P-09	12'-0"	12'-0"	12'-0"	12'-0"
P-10	12'-0"	12'-0"	12'-0"	12'-0"
P-11	12'-0"	12'-0"	12'-0"	12'-0"
P-12	12'-0"	12'-0"	12'-0"	12'-0"
P-13	12'-0"	12'-0"	12'-0"	12'-0"
P-14	12'-0"	12'-0"	12'-0"	12'-0"
P-15	12'-0"	12'-0"	12'-0"	12'-0"
P-16	12'-0"	12'-0"	12'-0"	12'-0"
P-17	12'-0"	12'-0"	12'-0"	12'-0"
P-18	12'-0"	12'-0"	12'-0"	12'-0"
P-19	12'-0"	12'-0"	12'-0"	12'-0"
P-20	12'-0"	12'-0"	12'-0"	12'-0"
P-21	12'-0"	12'-0"	12'-0"	12'-0"
P-22	12'-0"	12'-0"	12'-0"	12'-0"
P-23	12'-0"	12'-0"	12'-0"	12'-0"
P-24	12'-0"	12'-0"	12'-0"	12'-0"
P-25	12'-0"	12'-0"	12'-0"	12'-0"
P-26	12'-0"	12'-0"	12'-0"	12'-0"
P-27	12'-0"	12'-0"	12'-0"	12'-0"
P-28	12'-0"	12'-0"	12'-0"	12'-0"
P-29	12'-0"	12'-0"	12'-0"	12'-0"
P-30	12'-0"	12'-0"	12'-0"	12'-0"
P-31	12'-0"	12'-0"	12'-0"	12'-0"
P-32	12'-0"	12'-0"	12'-0"	12'-0"
P-33	12'-0"	12'-0"	12'-0"	12'-0"
P-34	12'-0"	12'-0"	12'-0"	12'-0"
P-35	12'-0"	12'-0"	12'-0"	12'-0"
P-36	12'-0"	12'-0"	12'-0"	12'-0"
P-37	12'-0"	12'-0"	12'-0"	12'-0"
P-38	12'-0"	12'-0"	12'-0"	12'-0"
P-39	12'-0"	12'-0"	12'-0"	12'-0"
P-40	12'-0"	12'-0"	12'-0"	12'-0"
P-41	12'-0"	12'-0"	12'-0"	12'-0"
P-42	12'-0"	12'-0"	12'-0"	12'-0"
P-43	12'-0"	12'-0"	12'-0"	12'-0"
P-44	12'-0"	12'-0"	12'-0"	12'-0"
P-45	12'-0"	12'-0"	12'-0"	12'-0"
P-46	12'-0"	12'-0"	12'-0"	12'-0"
P-47	12'-0"	12'-0"	12'-0"	12'-0"
P-48	12'-0"	12'-0"	12'-0"	12'-0"
P-49	12'-0"	12'-0"	12'-0"	12'-0"
P-50	12'-0"	12'-0"	12'-0"	12'-0"
P-51	12'-0"	12'-0"	12'-0"	12'-0"
P-52	12'-0"	12'-0"	12'-0"	12'-0"
P-53	12'-0"	12'-0"	12'-0"	12'-0"
P-54	12'-0"	12'-0"	12'-0"	12'-0"
P-55	12'-0"	12'-0"	12'-0"	12'-0"
P-56	12'-0"	12'-0"	12'-0"	12'-0"
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P-60	12'-0"	12'-0"	12'-0"	12'-0"
P-61	12'-0"	12'-0"	12'-0"	12'-0"
P-62	12'-0"	12'-0"	12'-0"	12'-0"
P-63	12'-0"	12'-0"	12'-0"	12'-0"
P-64	12'-0"	12'-0"	12'-0"	12'-0"
P-65	12'-0"	12'-0"	12'-0"	12'-0"
P-66	12'-0"	12'-0"	12'-0"	12'-0"
P-67	12'-0"	12'-0"	12'-0"	12'-0"
P-68	12'-0"	12'-0"	12'-0"	12'-0"
P-69	12'-0"	12'-0"	12'-0"	12'-0"
P-70	12'-0"	12'-0"	12'-0"	12'-0"
P-71	12'-0"	12'-0"	12'-0"	12'-0"
P-72	12'-0"	12'-0"	12'-0"	12'-0"
P-73	12'-0"	12'-0"	12'-0"	12'-0"
P-74	12'-0"	12'-0"	12'-0"	12'-0"
P-75	12'-0"	12'-0"	12'-0"	12'-0"
P-76	12'-0"	12'-0"	12'-0"	12'-0"
P-77	12'-0"	12'-0"	12'-0"	12'-0"
P-78	12'-0"	12'-0"	12'-0"	12'-0"
P-79	12'-0"	12'-0"	12'-0"	12'-0"
P-80	12'-0"	12'-0"	12'-0"	12'-0"
P-81	12'-0"	12'-0"	12'-0"	12'-0"
P-82	12'-0"	12'-0"	12'-0"	12'-0"
P-83	12'-0"	12'-0"	12'-0"	12'-0"
P-84	12'-0"	12'-0"	12'-0"	12'-0"
P-85	12'-0"	12'-0"	12'-0"	12'-0"
P-86	12'-0"	12'-0"	12'-0"	12'-0"
P-87	12'-0"	12'-0"	12'-0"	12'-0"
P-88	12'-0"	12'-0"	12'-0"	12'-0"
P-89	12'-0"	12'-0"	12'-0"	12'-0"
P-90	12'-0"	12'-0"	12'-0"	12'-0"
P-91	12'-0"	12'-0"	12'-0"	12'-0"
P-92	12'-0"	12'-0"	12'-0"	12'-0"
P-93	12'-0"	12'-0"	12'-0"	12'-0"
P-94	12'-0"	12'-0"	12'-0"	12'-0"
P-95	12'-0"	12'-0"	12'-0"	12'-0"
P-96	12'-0"	12'-0"	12'-0"	12'-0"
P-97	12'-0"	12'-0"	12'-0"	12'-0"
P-98	12'-0"	12'-0"	12'-0"	12'-0"
P-99	12'-0"	12'-0"	12'-0"	12'-0"
P-100	12'-0"	12'-0"	12'-0"	12'-0"

200 SOUTH LENA RD.
SAN BERNARDINO, CA

PH: 951.465.1871

PROJECT: 200 SOUTH LENA RD. SAN BERNARDINO, CA

CLIENT: SAN BERNARDINO COUNTY SHERIFF'S CRIME LAB

PROJECT NO.: SBC-002

SHEET TITLE: DOOR & WINDOW SCHEDULES

SHEET NO.: A600

WALL SCHEDULE

TYPE MARK	FINISH SIDE 1	SUBSTRATE SIDE 1	FRAMING MATERIAL	SUBSTRATE SIDE 2	FINISH SIDE 2	FIRE RATING FBI	INSULATION TYPE	ETC RATING	COMMENTS
WM.2	---	---	4" METAL STUD	---	---	0	---	N/A	MATCH EXISTING ADJACENT WALL ASSEMBLY. FIELD VERIFY.
WM.21	---	---	4" JOIST	---	---	0	X	N/A	MATCH EXISTING ADJACENT WALL ASSEMBLY. FIELD VERIFY.
WM.1	5/8" GYPSUM BOARD	---	4" METAL STUD	---	---	0	X	N/A	
WM.2	5/8" GYPSUM BOARD	---	4" METAL STUD	---	5/8" GYPSUM BOARD	0	S	N/A	
WM.12	5/8" GYPSUM BOARD	---	CHIMNEY	---	5/8" GYPSUM BOARD	0	S	N/A	

OPENINGS MATERIAL SCHEDULE

Material: Sorting for Schedule	KEY	PRODUCT	MANUFACTURER	STYLE/SCREEN/FINISH
08 11 16 - ALUMINUM DOORS AND FRAMES				
0	02-N	BLACK ANODIZED - ALUMINUM DOOR/FRAME		
0	02-N	CLEAR ANODIZED - ALUMINUM DOOR/FRAME		
0	02-N	DARK BROWN ANODIZED - ALUMINUM DOOR/FRAME		
08 14 00 - WOOD DOORS				
0	01-W	STAIN FRESH INTERIOR WOOD DOOR		
08 14 23 16 - PLASTIC LAMINATE FRESH WOOD DOORS				
0	PLAM	PLASTIC LAMINATE FRESH INTERIOR DOOR		
08 20 00 - GLAZING				
0	02-1	EXTERIOR GLAZING TYPE 1, INSULATED GLASS UNIT		
0	02-1	INTERIOR GLAZING TYPE 1, INSULATED GLASS UNIT		
0	02-2	EXTERIOR GLAZING TYPE 2, INSULATED GLASS UNIT		
0	02-3	EXTERIOR GLAZING TYPE 3, INSULATED GLASS UNIT		
0	02-4	EXTERIOR GLAZING TYPE 4, INSULATED GLASS UNIT		
08 30 00 - PAINTING				
0	PTPN-1	PAINT FRESH EXTERIOR YELLOW METAL DOOR		
0	PTPN-2	PAINT FRESH EXTERIOR YELLOW METAL FRAME		
0	PTPN-3	PAINT FRESH INTERIOR YELLOW METAL DOOR		
0	PTPN-4	PAINT FRESH INTERIOR YELLOW METAL FRAME		

TOILET ACCESSORIES SCHEDULE

TYPE MARK	MANUFACTURER	DESCRIPTION	MODEL	SIZE	BLOCKING REQUIRED	COUNT
002	BOURCK	Long Leverage Soap, Tissue Dispenser And Waste Receptacle	0-20012			1
003	BOURCK		0-10-2000			1
004	BOURCK	Double Leverage Wall Mounted Liquid Soap Dispenser - 10 2012	0-2012			1
005	BOURCK		2-201		Yes	1
006	BOURCK	Long Leverage Recirculating Sanitary Napkin Dispenser	0-20003	6-1/16"W X 12-1/2"H X 3/4" X 1/2" PROTRUSION		1
007	BOURCK	Hand Sanitizer Dispenser - 100220001	0-20001	8" Grab Bar	Yes	2
008	BOURCK			8" Grab Bar	Yes	1



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