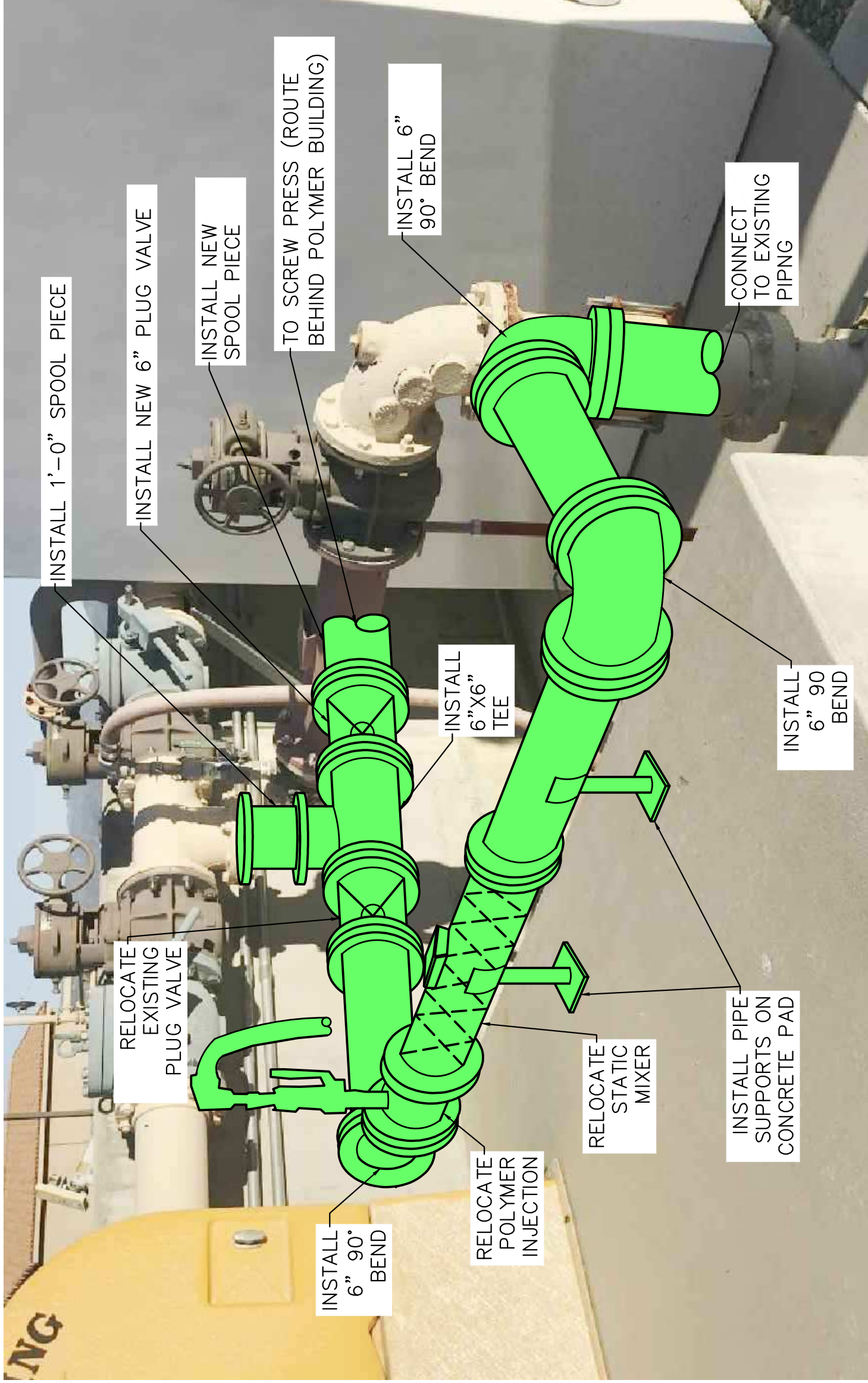
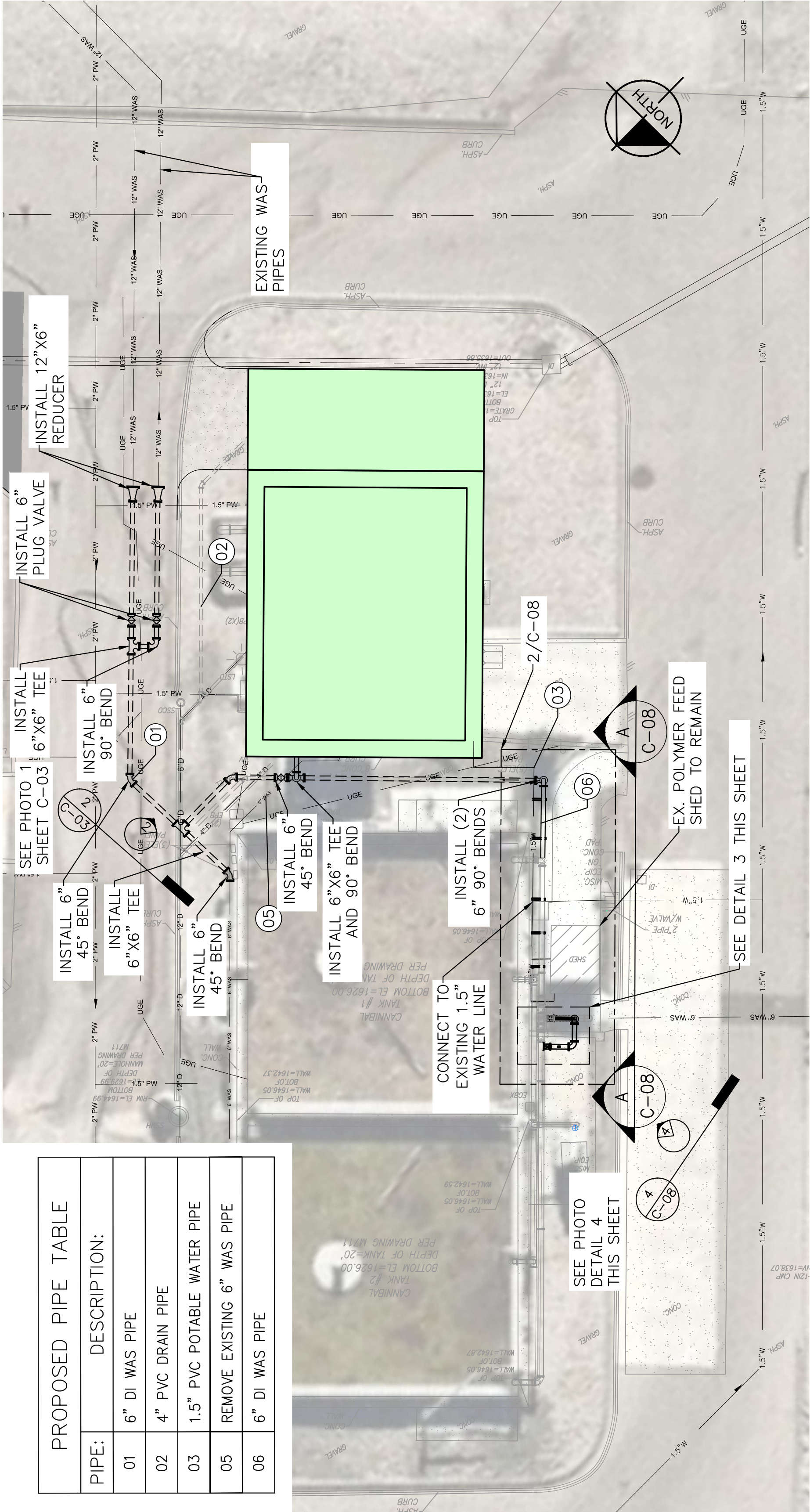


PROPOSED PIPE TABLE	
PIPE:	DESCRIPTION:
01	6" DI WAS PIPE
02	4" PVC DRAIN PIPE
03	1.5" PVC POTABLE WATER PIPE
05	REMOVE EXISTING 6" WAS PIPE
06	6" DI WAS PIPE



NOTE: CONTRACTOR SHALL FIELD DETERMINE PIPE LENGTHS

4

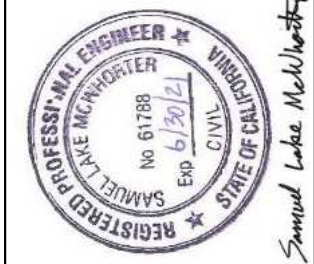
C-06

PROPOSED PIPING

SCALE: NOT TO SCALE

REV.	DATE	DESCRIPTION	APP.	CHECKED

COUNTY OF SAN BERNARDINO
DEPARTMENT OF PUBLIC WORKS - SPECIAL DISTRICTS
222 WEST HOSPITALITY LANE, 2ND FLOOR
SAN BERNARDINO, CA 92415-0450
909-386-8800



APPROVED	2/28/2021
PROJECT ENGINEER	DATE
RECOMMENDED	DATE

LYTLE CREEK NORTH CSA 70 GH SCREW PRESS SLUDGE DEWATERING		DRAWING NUMBER	C-08
PROPOSED WAS PIPING		SHEET	13 of 26
		SCALE:	DATE: DEC 2020

A

C-08

PROPOSED 6-INCH WAS PIPING SECTION

SCALE: NOT TO SCALE

EX POLYMER FEED SHED NOT SHOWN FOR CLARITY

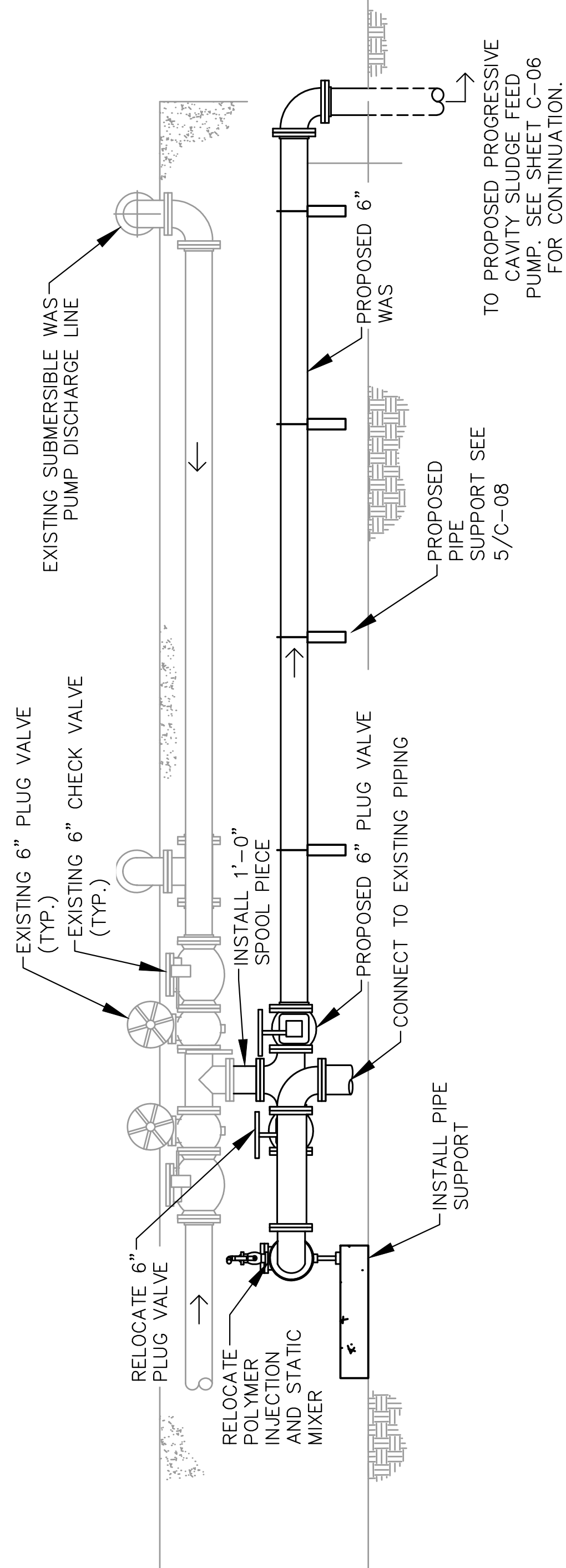
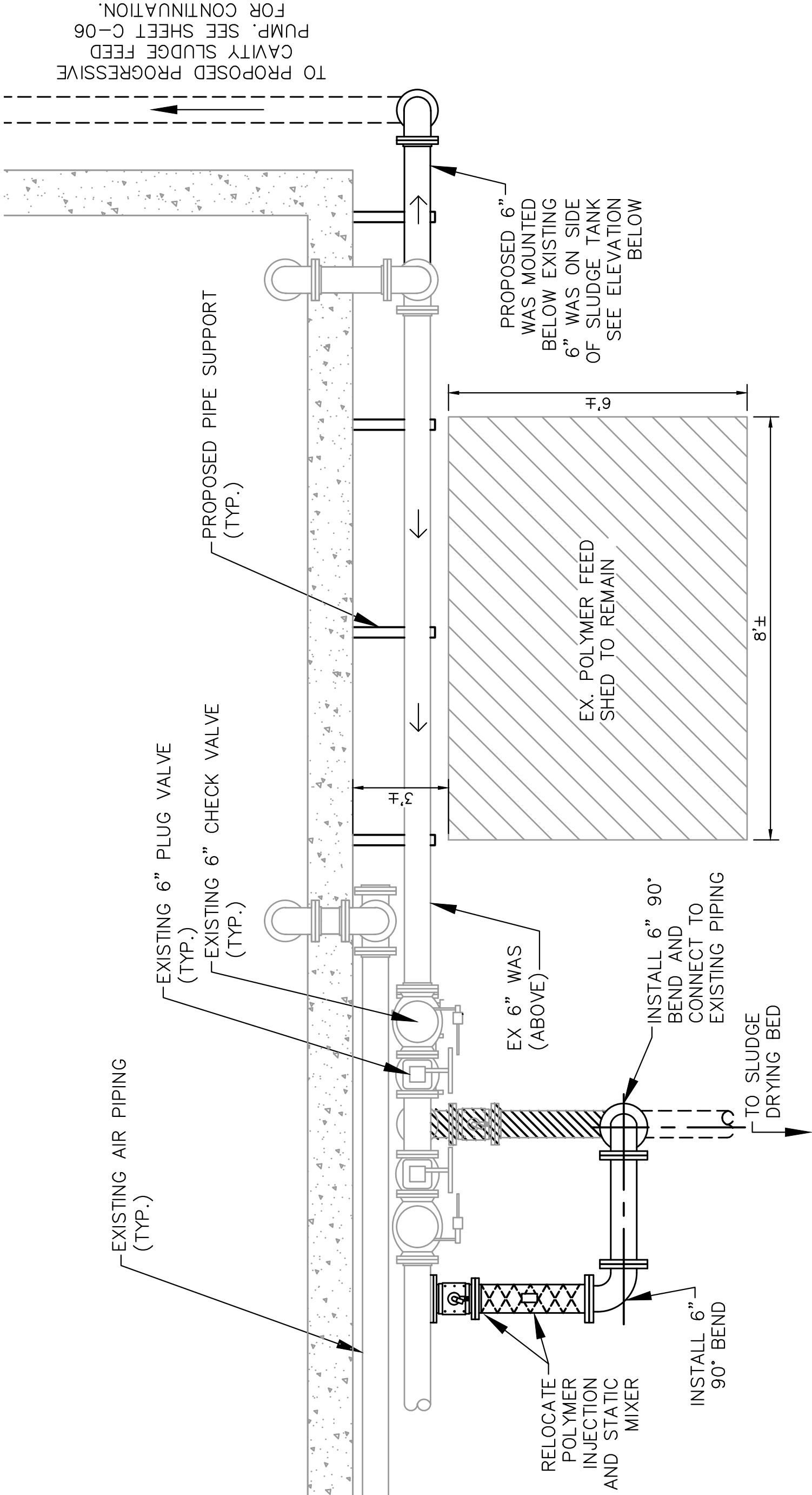
"NOT FOR BID"

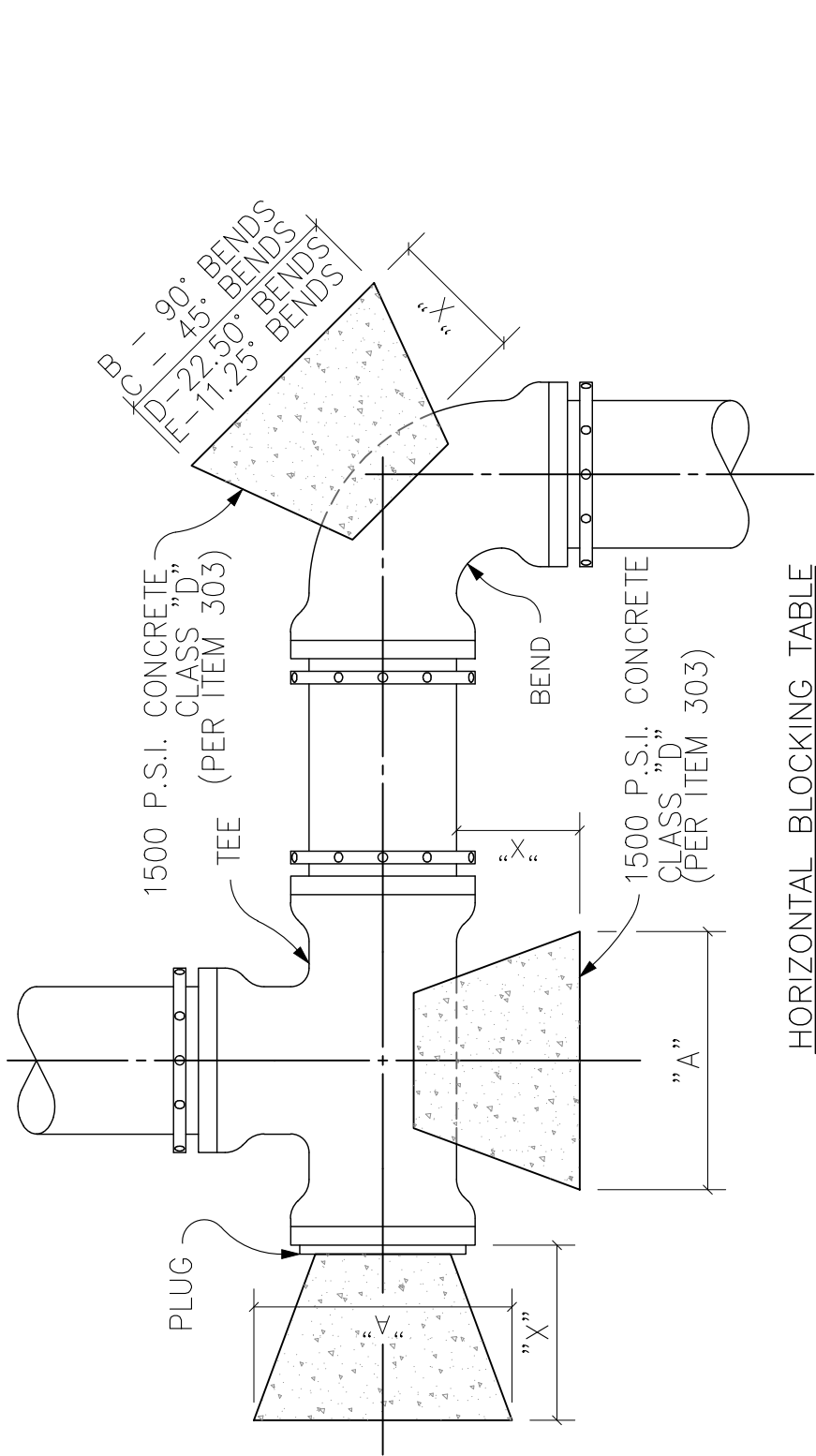
2

C-08

PROPOSED 6-INCH WAS PIPING PLAN VIEW

SCALE: NOT TO SCALE



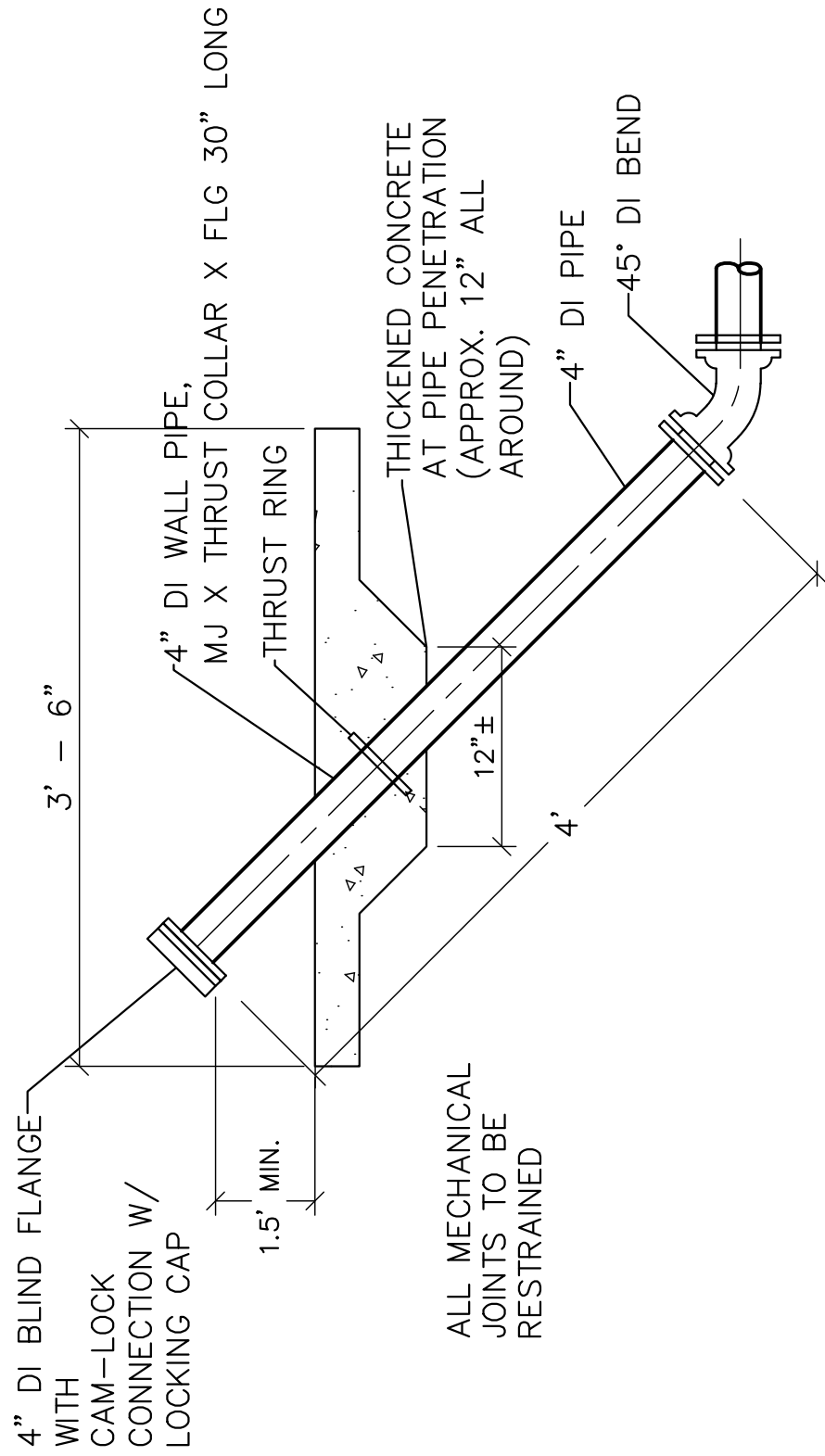


HORIZONTAL BLOCKING TABLE											
DIMENSION "X" TO BE A MINIMUM OF (1) FOOT, BUT IS TO BE INCREASED WHERE NECESSARY TO PROVIDE BEARING AGAINST UNDISTURBED TRENCH WALL.											
PIPE SIZE	"N" DIM.	PLUGS & TEES		90° BENDS		45° BENDS		22.50° BENDS		11.25° BENDS	
		"A"	"B"	"C"	"D"	"E"	"F"	"G"	"H"	"I"	"J"
6"	1'-6"	1'-0"	1'-2"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"	.83
8"	1'-6"	1'-3"	1'-6"	1'-3"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"	.83
10"	1'-6"	1'-9"	2'-0"	1'-6"	1'-3"	1'-3"	1'-3"	1'-3"	1'-0"	1'-0"	1.15
12"	1'-6"	2'-0"	2'-3"	2'-0"	1'-6"	1'-6"	1'-6"	1'-6"	1'-3"	1'-0"	1.66
16"	2'-0"	2'-7"	3'-0"	3'-0"	2'-3"	2'-3"	2'-3"	2'-3"	1'-8"	1'-2"	2.96
20"	2'-0"	3'-3"	3'-10"	3'-10"	3'-3"	3'-3"	3'-3"	3'-3"	1'-10"	1'-2"	4.62

- NOTES:
- BEARING AREAS SHOWN ARE BASED ON 150 PSI TEST PRESSURE AND 4000 PSF ALLOWABLE SOIL BEARING PRESSURE.
 - WRAP ALL BELOW GROUND IRON ASSEMBLIES IN POLYETHYLENE ACCORDING TO AWWA C105.
 - ALL TEES, BENDS, PLUGS, ETC. SHALL BE MECHANICALLY RESTRAINED BY MEGALUG OR APPROVED EQUAL.

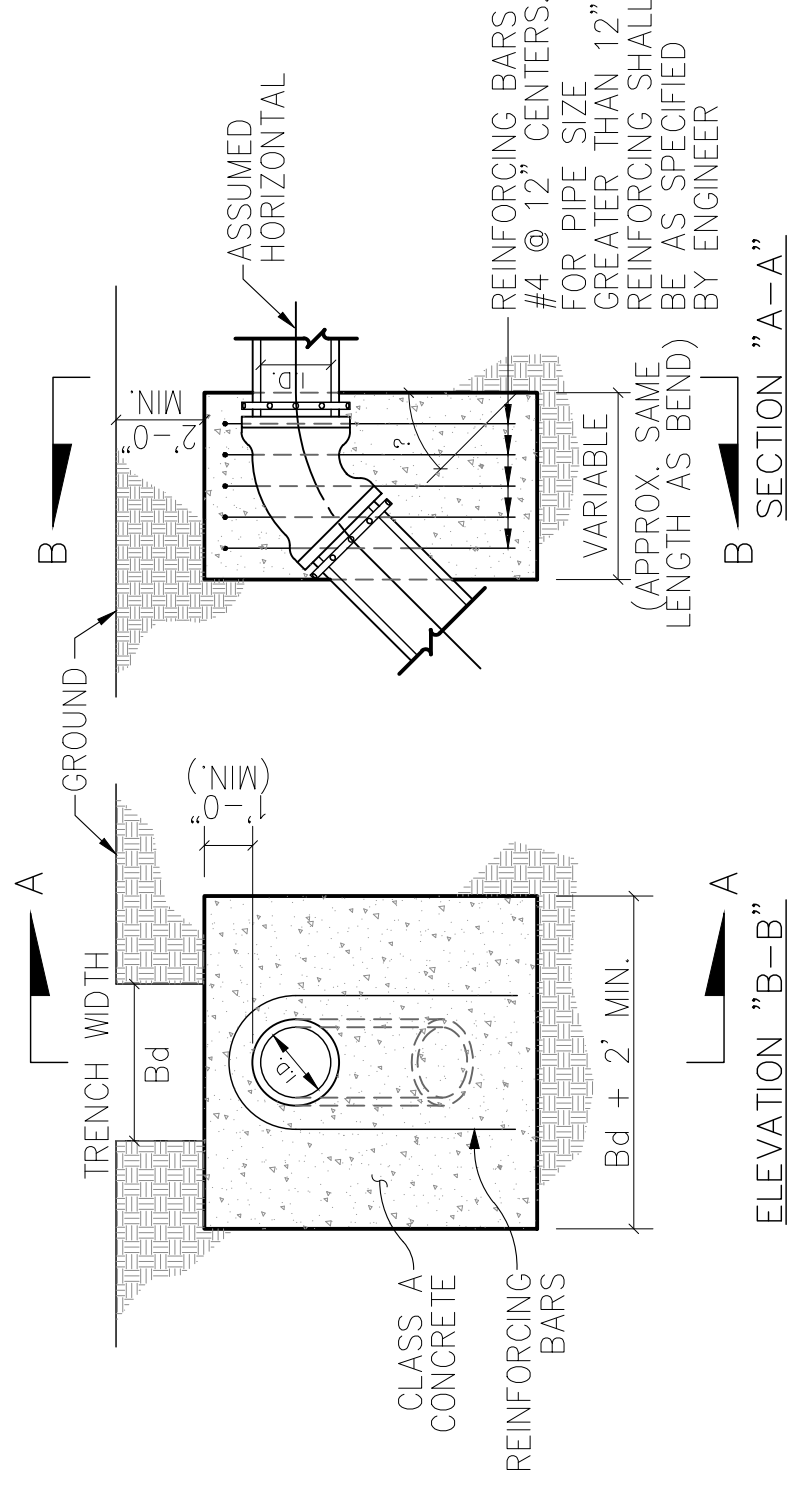
1 HORIZONTAL BLOCKING

SCALE: NOT TO SCALE



4 WAS CLEANOUT SECTION VIEW

SCALE: NOT TO SCALE

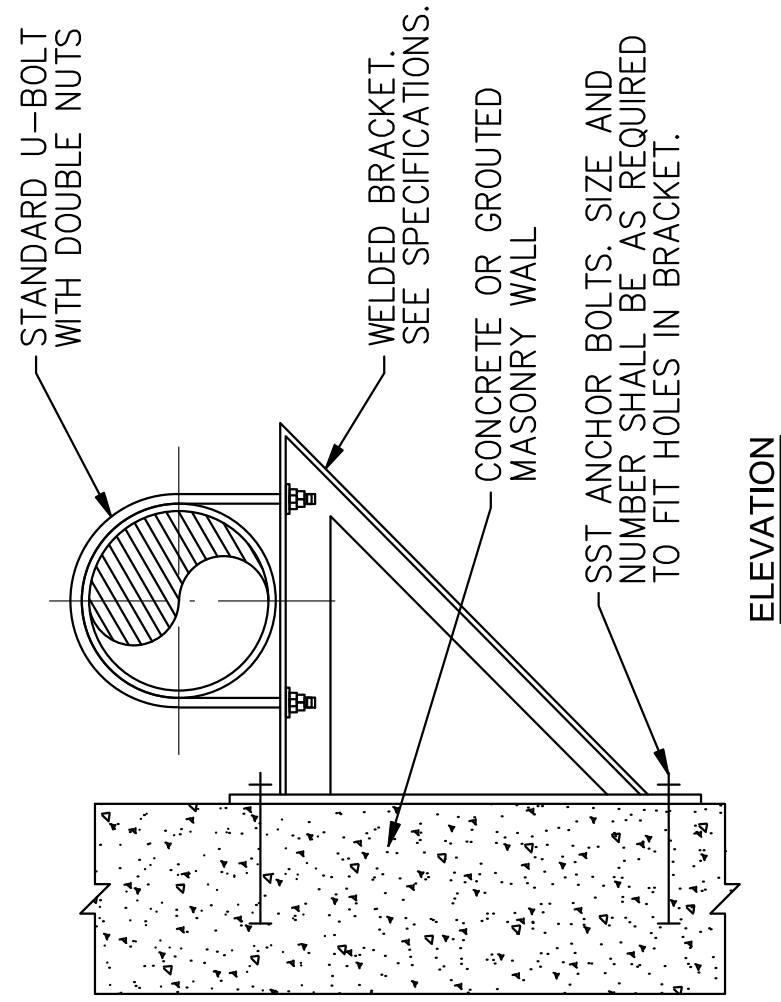
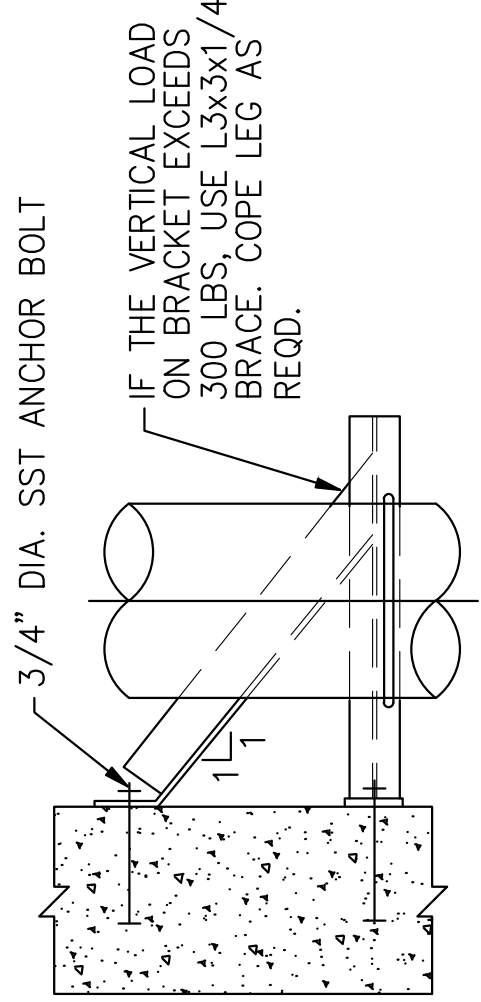


VERTICAL THRUST BLOCK TABLE											
Δ	D (IN.)	THRUST (TONS)	VOL. (C.Y.)	THRUST (TONS)	VOL. (C.Y.)	THRUST (TONS)	VOL. (C.Y.)	THRUST (TONS)	VOL. (C.Y.)	THRUST (TONS)	VOL. (C.Y.)
11.25'	11.25'	11.25'	11.25'	11.25'	11.25'	11.25'	11.25'	11.25'	11.25'	11.25'	11.25'
22.50'	22.50'	22.50'	22.50'	22.50'	22.50'	22.50'	22.50'	22.50'	22.50'	22.50'	22.50'
30.00'	30.00'	30.00'	30.00'	30.00'	30.00'	30.00'	30.00'	30.00'	30.00'	30.00'	30.00'
45.00'	45.00'	45.00'	45.00'	45.00'	45.00'	45.00'	45.00'	45.00'	45.00'	45.00'	45.00'
67.50'	67.50'	67.50'	67.50'	67.50'	67.50'	67.50'	67.50'	67.50'	67.50'	67.50'	67.50'
90.00'	90.00'	90.00'	90.00'	90.00'	90.00'	90.00'	90.00'	90.00'	90.00'	90.00'	90.00'

- NOTES:
- WRAP ALL BELOW GROUND IRON ASSEMBLIES IN POLYETHYLENE ACCORDING TO AWWA C105.
 - ALL TEES, BENDS, PLUGS, ETC. SHALL BE MECHANICALLY RESTRAINED BY MEGALUG OR APPROVED EQUAL.

2 VERTICAL BLOCKING

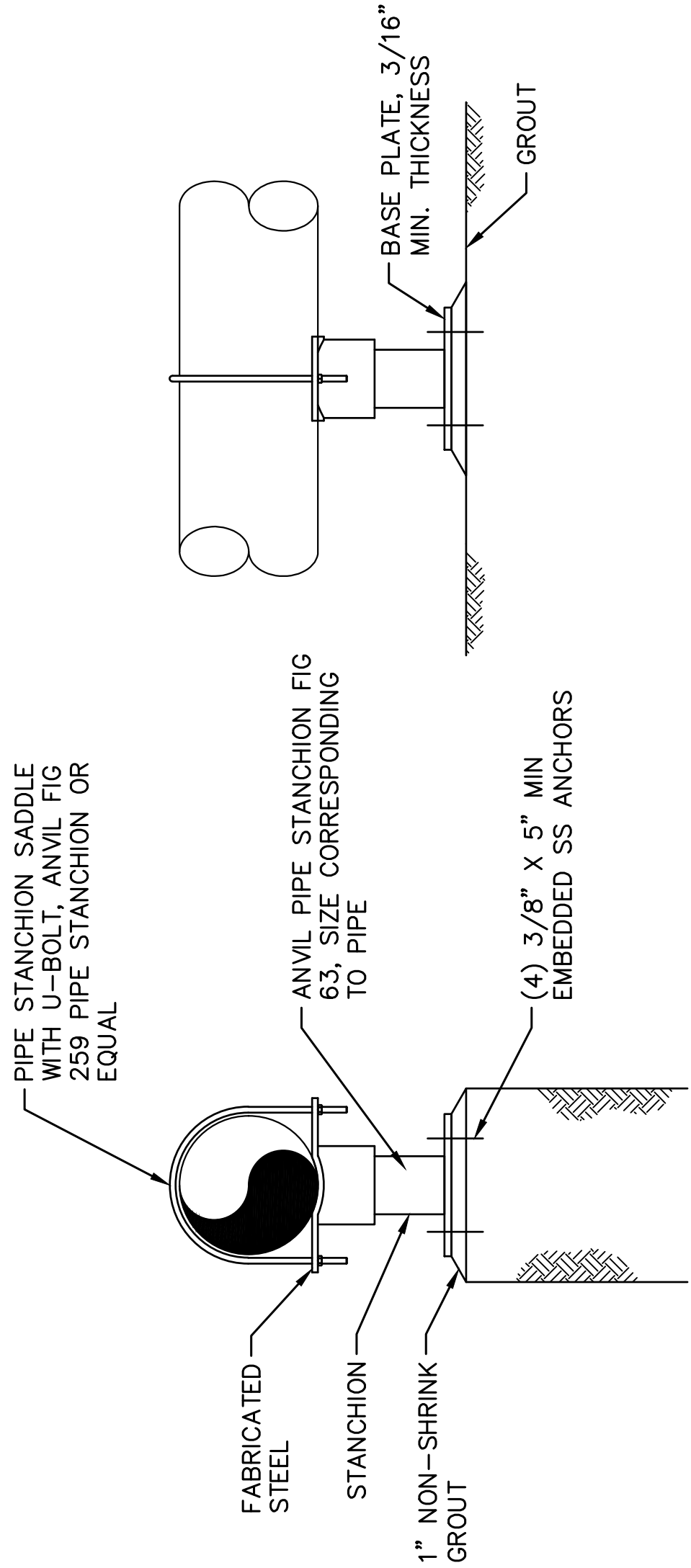
SCALE: NOT TO SCALE



- NOTES:
- HOT-DIP GALVANIZED SUPPORT AFTER FABRICATION.

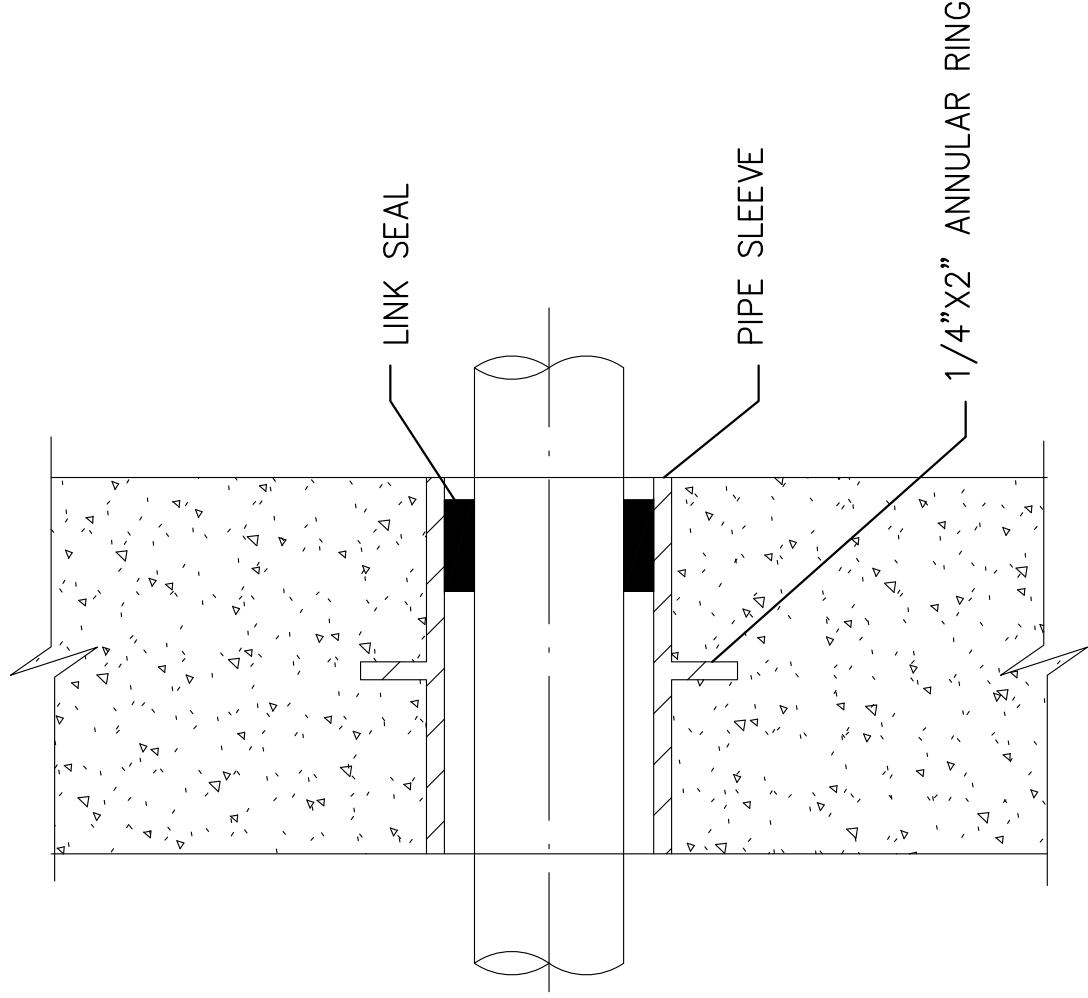
5 PIPE SUPPORT-WALL BRACKET BELOW

SCALE: NOT TO SCALE



3 PIPE STANCHION DETAIL

SCALE: NOT TO SCALE



- NOTES:
- 6" SLEEVES AND SMALLER SHALL BE SCH 40 STEEL PIPE.
 - SLEEVES LARGER THAN 6" SHALL BE 1/4" THICK STEEL PIPE.
 - IN WALLS THICKER THAN 12", LINK SEAL SHALL BE INSTALLED AT BOTH ENDS OF WALL SLEEVE. SLEEVE DIAMETER SHALL BE PER LINK SEAL MANUFACTURER'S RECOMMENDATION.
 - SLEEVE SHALL BE HOT-DIP GALVANIZED AFTER FABRICATION.

6 WALL/FLOOR PENETRATION

SCALE: NOT TO SCALE

REV.	DATE	DESCRIPTION

COUNTY OF SAN BERNARDINO
DEPARTMENT OF PUBLIC WORKS - SPECIAL DISTRICTS
222 WEST HOSPITALITY LANE, 2ND FLOOR
SAN BERNARDINO, CA 92415-0450
909-386-8800



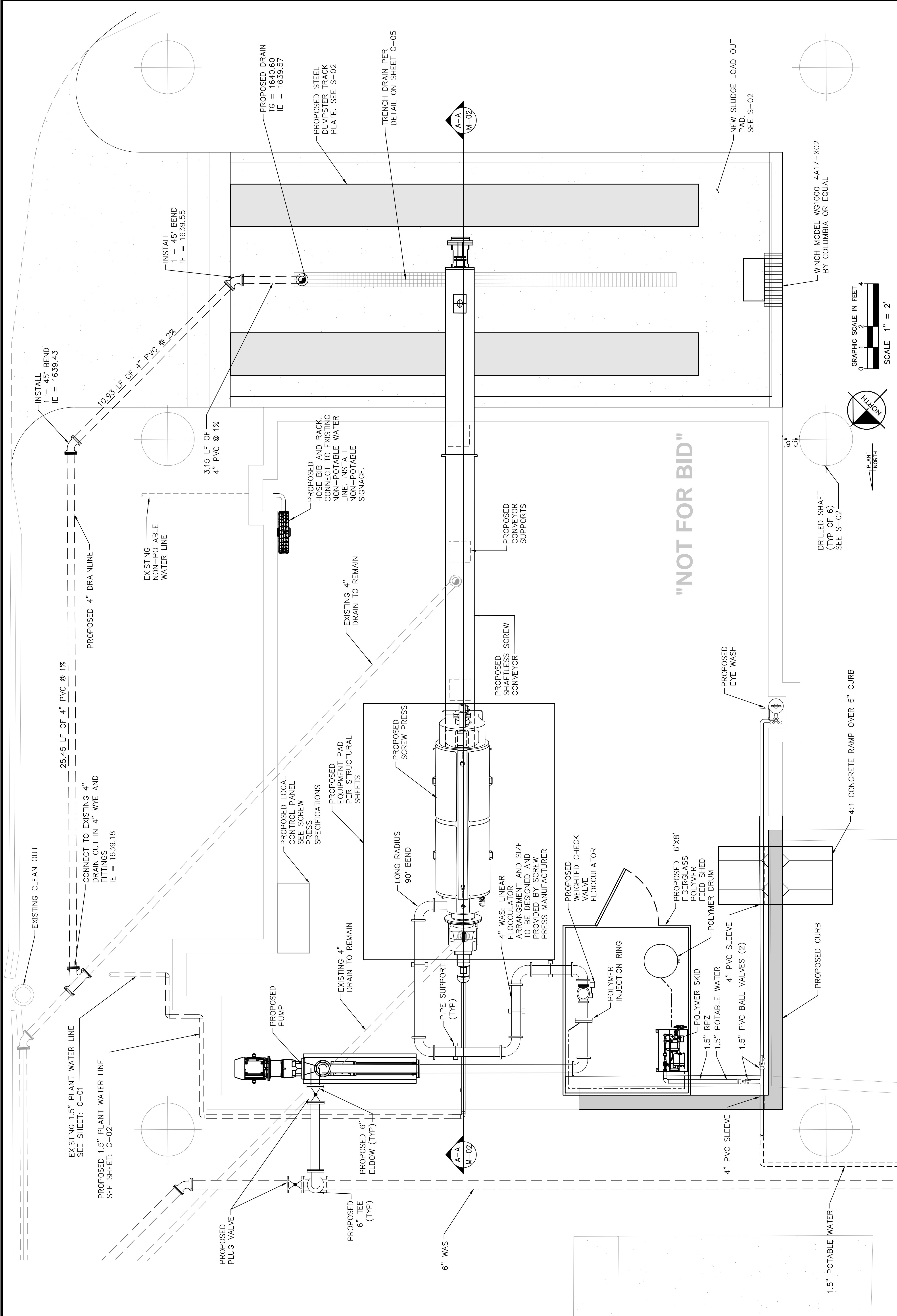
APPROVED *David Lee McElroy* 2/28/2021 DATE
PROJECT ENGINEER *Michael* 12/28/20 DATE
RECOMMENDED _____ DATE

LYTLE CREEK NORTH
CSA 70 GH SCREW PRESS
SLUDGE DEWATERING

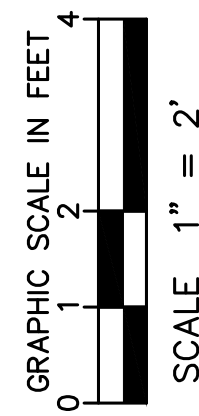
YARD PIPING DETAILS

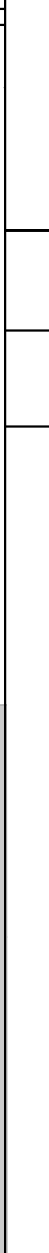
NOTE: ALL EXPOSED
PIPING SHALL BE INSULATED
AND HEAT TRACED PER
SPECIFICATION 15050

AERATED SLUDGE
HOLDING TANK #2



"NOT FOR BID"



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A. GENERAL

1. ALL STRUCTURES ARE DESIGNED TO ACT AS A STRUCTURAL UNIT UPON COMPLETION. CONTRACTOR SHALL DESIGN AND PROVIDE NECESSARY BRACING, TEMPORARY SUPPORTS, AND SHORING TO RESIST FORCES ON THE STRUCTURE DURING CONSTRUCTION.
2. VERIFY ALL DIMENSIONS AND ELEVATIONS PRIOR TO STARTING WORK. NOTIFY THE ENGINEER OF ANY DISCREPANCIES.
3. VERIFY LOCATIONS OF ALL EXISTING UTILITIES WITHIN 20 FEET OF CANOPY DRIP LINE, SCUM PUMP STATION, AND RAS/WAS PLATFORM PRIOR TO STARTING WORK.
4. COORDINATE WITH MECHANICAL, ELECTRICAL, PLUMBING, ARCHITECTURAL, AND SIGN DRAWINGS AND SUPPLIERS FOR ATTACHMENT OF ANY ELEMENTS TO THE STRUCTURAL MEMBERS OF THE FACILITIES. SHOW INSERTS ON ALL SHOP DRAWINGS. DRILLED CONNECTIONS INTO OTHER CONCRETE MEMBERS MUST BE LOCATED TO MISS REINFORCEMENT. ALL CONNECTIONS TO STRUCTURAL ELEMENTS MUST BE APPROVED BY THE ENGINEER.

5. CONTRACTOR SHALL EXERCISE EXTREME CARE DURING THE EXCAVATION AND CONSTRUCTION FOR NEW STRUCTURE TO AVOID DAMAGE TO EXISTING STRUCTURES AND PIPELINES. CONTRACTOR IS RESPONSIBLE FOR PROVIDING ALL MEANS AND METHODS REQUIRED TO FACILITATE CONSTRUCTION OF THE WORK AND ENSURING THE SAFETY, STABILITY AND INTEGRITY OF ADJACENT STRUCTURES AND FACILITIES.
6. ALL MATERIALS AND INSTALLATION COST FOR SCREW PRESS FOUNDATIONS SHALL BE INCLUDED IN THE COST FOR THE SCREW PRESS CANOPY. SCREW PRESS CANOPY SHOP DRAWINGS AND CORRESPONDING STRUCTURAL CALCULATIONS SHALL BE SUBMITTED FOR REVIEW AND APPROVAL PER NOTE "J. SUBMITTALS" ON THIS SHEET. THE SCREW PRESS CANOPY FOUNDATIONS SHALL BE FINALIZED ONCE THE ENGINEER OF RECORD HAS APPROVED THE SCREW PRESS CANOPY SHOP DRAWINGS. BID FORMS TO INCLUDE AN OWNER-CONTROLLED CONTINGENCY TO COVER COST DIFFERENCE BETWEEN DESIGN DETAILS PROPOSED HEREIN AND ANY OWNER-APPROVED CHANGES TO ACCOMMODATE FINAL DEFERRED DESIGN.

B. DESIGN CRITERIA

1. BUILDING CODE
a. DESIGN 2016 CALIFORNIA BUILDING CODE WITH SAN BERNARDINO COUNTY AMENDMENTS
b. CONSTRUCTION PROJECT SPECIFICATIONS
2. DESIGN LOADS
a. LIVE LOAD
i. (300 LB CONCENTRATED LOAD PER 2016 CBC TABLE 1607.1)
- b. ROOF LIVE LOAD 20 PSF
- B. WIND LOAD
a. BASIC WIND SPEED 100 MPH
b. IMPORTANCE FACTOR 1.00
c. EXPOSURE CATEGORY C
d. INTERNAL PRESSURE COEFFICIENT VARIES (PER ASCE 7, FIGURE 6-18A)
- C. SEISMIC LOAD
a. OCCUPANCY CATEGORY II
b. SPECTRAL RESPONSE (S_{DS}) 1.647g
c. SPECTRAL RESPONSE (S_{NI}) 1.132g
d. SEISMIC DESIGN CATEGORY D
e. SITE CLASS E
f. IMPORTANCE FACTOR 1.0
g. SEISMIC FORCE RESISTING SYSTEM: CANTILEVERED COLUMN SYSTEMS DETAILED TO CONFORM TO THE REQUIREMENTS FOR: "SPECIAL STEEL MOMENT FRAMES"
h. RESPONSE MODIFICATION FACTOR 1.5
i. DEFLECTION AMPLIFICATION FACTOR 1.5
j. ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE
- D. DESIGN DEAD LOADS
a. SELF WEIGHT OF MATERIAL
- E. EARTH LOADS: EARTH LOADS PROVIDED BY PROJECT GEOTECHNICAL INVESTIGATION REPORT, SCREW PRESS AT THE LYTLE CREEK NORTH WATER RECYCLING FACILITY, CITY OF SAN BERNARDINO, CALIFORNIA, PREPARED BY CONVERSE CONSULTANTS, PROJECT NO. 18-81-228-01, DATED SEPTEMBER 19, 2018.
a. BEARING CAPACITY 2,200 PSF
b. ACTIVE EARTH PRESSURE 40 PSF
c. AT-REST EARTH PRESSURE 60 PSF
d. PASSIVE EARTH PRESSURE 220 PSF (2,200 PSF MAX)
e. COEFFICIENT OF FRICTION 0.35
- F. PIER FOUNDATION (DRILLED SHAFT)
a. OD 32"
b. DEPTH 15'-0"
c. EXTRA DEPTH 180
d. ALLOWABLE SKIN FRICTION 180 PSF (6-INCH DIA DRILLED PIER)
PER THE GEOTECHNICAL REPORT THE ALLOWABLE SKIN FRICTION VALUE WILL BE INCREASED WITH THE INCREMENT OF DRILLED PIER DIAMETER. THIS VALUE MAY BE INCREASED BY 33 PERCENT FOR TRANSIENT WIND AND SEISMIC FORCES. FOR PIER DESIGN IN TENSION, 50 PERCENT OF THE RECOMMENDED ALLOWABLE SKIN FRICTION VALUES IN COMPRESSION MAY BE USED. FOR DESIGN PURPOSE, THE UPPER 2 FEET OF THE SOILS SHOULD BE NEGLECTED IN DETERMINING SKIN FRICTION. THE EQUIVALENT LATERAL EARTH PRESSURE EQUAL TO 220 PSF PER FOOT OF DEPTH MAY BE USED FOR THE DESIGN.

C. CONCRETE

1. ALL CONCRETE FOR STRUCTURES SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4,000 POUNDS PER SQUARE INCH AT 28 DAYS EXCEPT AS NOTED. CEMENT FOR CONCRETE SHALL BE TYPE II.
2. REINFORCING BARS SHALL BE NEW BILLET STEEL CONFORMING TO ASTM DESIGNATION A615, GRADE 60, DEFORMED. REINFORCING BARS SHOWN ON THE PLANS TO BE WELDED SHALL BE NEW BILLET STEEL CONFORMING TO ASTM DESIGNATION A706, GRADE 60, DEFORMED.
3. CONCRETE DESIGN IS IN CONFORMANCE WITH "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318-05). AND 2007 CBC.

4. DETAIL, FABRICATE AND ERECT REINFORCEMENT BARS, INCLUDING BAR SUPPORTS, SPACERS, ETC. IN ACCORDANCE WITH "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT" (ACI SP-66(04)) OR APPROVED BY BUILDING MANUFACTURER.
5. CONCRETE COVER FOR REINFORCEMENT BARS SHALL CONFORM TO THE FOLLOWING, UNLESS INDICATED OTHERWISE ON THE DRAWINGS.
a. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH . . . 3 INCHES
b. CONCRETE EXPOSED TO EARTH OR WEATHER 2 INCHES
c. PEDESTALS 1½ INCHES
NOTE: CLEARANCES MAY BE REDUCED ½ INCH FOR STIRRUPS AND TIES.
6. CHAMFER EXPOSED CONCRETE EDGES ¾ INCH X ¾ INCH UNLESS NOTED OTHERWISE. CONCRETE PEDESTALS SHALL NOT RECEIVE CHAMFERS.
7. CONCRETE JOINT LOCATIONS NOT SHOWN ON STRUCTURAL DRAWINGS SHALL BE SUBMITTED FOR REVIEW BY THE ENGINEER PRIOR TO START OF WORK.
8. UNLESS NOTED ON THE DRAWINGS, CONCRETE FORMED AND UNFORMED SURFACES SHALL RECEIVE FINISH NOTED IN SPECIFICATIONS.

D. EARTHWORK AND FOUNDATION PREPARATION

1. FOUNDATION PREPARATION SHALL BE PREPARED AS STATED IN THE PROJECT GEOTECHNICAL INVESTIGATION REPORT:
- PRIOR TO THE START OF CONSTRUCTION, ALL EXISTING UNDERGROUND UTILITIES AND APPURTENANCES, IF PRESENT, SHOULD BE LOCATED AT THE PROJECT SITE. SUCH UTILITIES SHOULD EITHER BE PROTECTED IN-PLACE OR REMOVED AND REPLACED DURING CONSTRUCTION AS REQUIRED BY THE PROJECT SPECIFICATIONS. ALL EXCAVATIONS SHOULD BE CONDUCTED IN SUCH A MANNER AS NOT TO CAUSE LOSS OF BEARING AND/OR LATERAL SUPPORT OF EXISTING STRUCTURES OR UTILITIES.
- THE FINAL BOTTOM OF SURFACES OF ALL EXCAVATIONS SHOULD BE OBSERVED AND APPROVED BY THE PROJECT GEOTECHNICAL CONSULTANT PRIOR TO PLACING ANY FILL.

TRUCK LOADOUT PAD SHALL BE OVEREXCAVATED, SCARIFIED AND RECOMPACTED A MINIMUM OF 18 INCHES BELOW BOTTOM PAD. THE OVEREXCAVATION SHOULD EXTEND AT LEAST 2 FEET BEYOND THE FOOTPRINT OF THE TRUCK LOADOUT PAD. COMPACTED FILL SHALL BE PREPARED PER THE GEOTECHNICAL REPORT REQUIREMENTS IN SECTION 9.4.

DRILLED PIERS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION SECTION 305-PILE DRIVING AND TIMBER CONSTRUCTION (GREENBOOK, 2015). BORING PIERS SHOULD BE REASONABLY CLEAN AND FREE OF LOOSE SOIL PRIOR TO INSTALLING THE CONCRETE PIERS. THE ANNULAR SPACE AROUND THE PIERS BASE SHOULD BE FILLED WITH GROUT.

CASING SHOULD BE USED AT THE DISCRETION OF THE CONTRACTOR. CASING SHALL BE ADVANCED AS DRILLING PROCEEDS BY DRILLING WITH A FLIGHT OR BUCKET AUGER SMALLER IN DIAMETER THAN THE INSIDE OF THE CASING. OCCASIONAL HAMMERING MAY BE REQUIRED TO ADVANCE THE CASING WITHIN THE EXCAVATION. THE CASING SHALL NOT BE LEFT IN PLACE AS THE PIERS DESIGNS ARE BASED ON SKIN FRICTION ONLY. CASING SHALL BE PULLED AS THE CONCRETE IS BEING POURED. WHILE ALWAYS MAINTAINING A HEAD OF CONCRETE INSIDE THE CASING, THE CONTRACTOR SHALL HAVE EQUIPMENT ON-SITE WITH SUFFICIENT PULLING CAPACITY TO PULL THE CASING. THE CASING SHALL HAVE INSIDE DIAMETER NOT LESS THAN THE SPECIFIED DIAMETER OF THE PIER.

DRILLED PIER INSTALLATION SHALL BE PERFORMED UNDER THE CONTINUOUS OBSERVATION OF THE GEOTECHNICAL ENGINEER CONSULTANT TO CONFIRM THAT THE SUBSURFACE SOILS ARE SIMILAR TO THE SOILS ENCOUNTERED DURING FIELD INVESTIGATION, WHICH HAVE FORMED THE BASIS OF PIER DESIGN RECOMMENDATIONS. THE CONTRACTOR SHALL PROVIDE ACCESS AND NECESSARY FACILITIES, INCLUDING DROPLIGHTS, AT CONTRACTORS EXPENSE, TO ACCOMMODATE PIER OBSERVATIONS.

E. STRUCTURAL STEEL

1. STRUCTURAL STEEL WIDE FLANGE BEAMS, COLUMNS, AND MISCELLANEOUS FRAMING MEMBERS SHALL CONFORM TO ASTM A992 GRADE 50.
2. PLATES, BARS, AND ANGLES SHALL CONFORM TO ASTM A36 UNLESS NOTED OTHERWISE.
3. HSS RECTANGULAR MEMBERS SHALL CONFORM TO ASTM A500, GRADE B (46 KSI).
4. DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE LATEST AISC CODES AND SPECIFICATIONS, INCLUDING THE AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS.
5. EXPOSED OR PARTIALLY EXPOSED STRUCTURAL STEEL SHALL BE FABRICATED AND INSTALLED PER THE "ARCHITECTURALLY EXPOSED STRUCTURAL STEEL" SECTION OF THE AISC MANUAL AND THE AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS. STEEL NOT CONFORMING TO THESE TOLERANCES WILL BE REJECTED AND REFABRICATED AT NO EXPENSE TO THE OWNER. SEE DRAWINGS FOR LOCATIONS OF ARCHITECTURALLY EXPOSED STEEL.
6. SHOP CONNECTIONS SHALL BE MADE WITH HIGH-STRENGTH BOLTS OR BY WELDING. FIELD CONNECTIONS SHALL BE MADE WITH HIGH-STRENGTH BOLTS, EXCEPT WHERE WELDING IS INDICATED ON THE DRAWINGS. HIGH-STRENGTH BOLTS SHALL CONFORM TO ASTM DESIGNATION A325 BEARING TYPE CONNECTION WITH THREADS INCLUDED IN SHEAR PLANE UNLESS NOTED OTHERWISE. ALL BOLTS SHALL BE ¾ INCH DIAMETER UNLESS NOTED OTHERWISE.
7. ALL WELD SIZES, NOT INDICATED SHALL COMPLY WITH THE LATEST AWS D1.1 BUT IN NO CASE SHALL WELD SIZE BE LESS THAN ¾ INCH.
8. NATURAL MILL CAMBER OF BEAMS SHALL BE PLACED UP.
9. THE STRUCTURAL BEAMS ARE NOT DESIGNED FOR USE AS LIFT BEAMS.
10. ADEQUATE TEMPORARY BRACING SHALL BE PROVIDED DURING CONSTRUCTION.
11. USE NON-SHRINK, NON-METALLIC GROUT UNDER BASE PLATES AS REQUIRED.
12. MINIMUM THICKNESS OF STRUCTURAL STEEL SHALL BE ¼ INCH UNLESS NOTED OTHERWISE.

F. METAL ROOF DECK

1. METAL ROOF DECK SHALL CONFORM TO THE LATEST S.D.I. "STEEL DECK DESIGN MANUAL".
2. UNLESS NOTED OTHERWISE METAL ROOF DECK SHALL BE PAINTED (SEE SPECIFICATIONS).
3. LIGHT GAGE FRAMING AND MECHANICAL ELECTRICAL OR OTHER EQUIPMENT SHALL NOT BE ATTACHED TO ANY METAL ROOF DECKING EXCEPT AS SHOWN ON THE CANOPY MANUFACTURER'S DRAWINGS.

4. PROVIDE ADDITIONAL REINFORCEMENT AND CLOSURE PIECES AT LOCATIONS OF PROPOSED ROOF OPENINGS PER REQUIREMENTS DEFINED IN THE CANOPY MANUFACTURER'S DESIGN CALCULATIONS AND SHOP DRAWINGS.
5. CONNECTIONS TO STEEL SUPPORTS SHALL BE FUSION TYPE WELDS AT ALL EDGES AND BY MECHANICAL FASTENERS AT SIDE LAPS. MINIMUM FASTENERS AT ALL SUPPORTS SHALL BE ¾" PUDDLE WELDS AT EACH FLUTE.
6. METAL ROOF DECK SHALL BE 3½" CLOSED RIB ROOF DECK WITH THE FOLLOWING MINIMUM SECTION PROPERTIES OR APPROVED EQUAL:
a. I = 1.75 IN
b. SP = 0.66 IN³
c. SN = 0.61 IN³
d. MIN THICKNESS = 20 GAUGE
SEE PROJECT SPECIFICATIONS FOR ADDITIONAL INFORMATION.

G. SCREW PRESS CANOPY FINISHES NOTES

1. CANOPY SHALL BE GALVANIZED.
2. SEE PROJECT SPECIFICATIONS FOR PAINTING AND FINISH REQUIREMENTS ON EXPOSED STRUCTURAL STEEL, STEEL ROOF JOISTS AND METAL ROOF DECK.
3. STANDING SEAM ROOF SHALL HAVE STANDING SEAM PROFILE HEIGHT = 1.5" OR APPROVED EQUAL.

H. SPECIAL INSPECTION

1. SPECIAL INSPECTION IS REQUIRED OF MATERIALS, INSTALLATION, FABRICATION, ERECTION OR PLACEMENT OF COMPONENTS AND CONNECTIONS REQUIRING SPECIAL EXPERTISE TO ENSURE COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS.
2. COORDINATE SCHEDULES WITH AGENCY PERFORMING SPECIAL INSPECTION TO INSURE AMPLE TIME IS AVAILABLE TO PERFORMED REQUIRED TASKS.
3. THE FOLLOWING ITEMS REQUIRE SPECIAL INSPECTION:

1. INSPECTION OF CONCRETE CONSTRUCTION (2016 CBC TABLE 1704.4)			
INSPECTION ITEMS	INSPECTION TASK	FREQUENCY OF INSPECTION	COMMENTS
STRUCTURAL CAST-IN-PLACE CONCRETE	REINFORCING STEEL INCLUDING PLACEMENT	PERIODIC	(1) (3) (4)
	VERIFICATION OF MIX DESIGN	PERIODIC	(1) (3)
	SAMPLES TAKEN FOR STRENGTH, SLUMP, AIR CONTENT, AND CONCRETE TEMPERATURE	CONTINUOUS	(2) (3)
	INSPECTION OF CONCRETE PLACEMENT	CONTINUOUS	(2) (3)
	MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES	PERIODIC	(1) (3)
	FORMWORK FOR SHAPE, LOCATION, AND DIMENSIONS	PERIODIC	(1) (3)
	INSPECT BOLTS TO BE INSTALLED IN CONCRETE PRIOR TO AND DURING PLACEMENT OF CONCRETE	CONTINUOUS	(2) (3)

2. INSPECTION OF STEEL CONSTRUCTION (2016 CBC TABLE 1704.3)			
INSPECTION ITEMS	INSPECTION TASK	FREQUENCY OF INSPECTION	COMMENTS
HIGH-STRENGTH BOLTS, NUTS, AND WASHERS	MATERIAL IDENTIFICATION MARKINGS	PERIODIC	(1) (3)
	MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED	PERIODIC	(1) (3)
	BEARING-TYPE CONNECTIONS	PERIODIC	(1) (3)
	SLIP-CRITICAL CONNECTIONS	PERIODIC	(1) (3)
	COMPLETE AND PARTIAL PENETRATION GROOVE WELDS	CONTINUOUS	(2) (3)
STRUCTURAL WELDING	MULTIPASS FILLET WELDS	CONTINUOUS	(2) (3)
	SINGLE-PASS FILLET WELDS > ¾"	CONTINUOUS	(2) (3)
	SINGLE-PASS FILLET WELDS < OR EQUAL TO ¾"	PERIODIC	(1) (3)
	FLOOR AND ROOF DECK WELDS	PERIODIC	(1) (3)
STRUCTURAL WELDING OF REINFORCING STEEL	-----	-----	(4)
FABRICATOR	INSPECT IN-PLANT FABRICATION PRACTICES	PERIODIC	(1)
	REVIEW QUALITY CONTROL PROCEDURES	PERIODIC	(1) (5)

COMMENTS

1. PERIODIC SPECIAL INSPECTION: THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF WORK. (2016 CBC 1702).
2. CONTINUOUS SPECIAL INSPECTION: THE FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED. (2016 CBC 1702.1).
3. ITEMS NOT SHOWN MAY REQUIRE CONTINUOUS OR PERIODIC SPECIAL STRUCTURAL INSPECTION AT THE DISCRETION OF THE ENGINEER. ITEMS LISTED MAY REQUIRE ALTERNATE FREQUENCIES OF INSPECTION OTHER THAN SHOWN UNDER DIRECTION OF THE ENGINEER.
4. WELDING OF REINFORCING STEEL NOT ACCEPTABLE UNLESS NOTED OTHERWISE ON THE PLANS OR AS DIRECTED BY THE ENGINEER.
5. FABRICATOR SHALL SUBMIT CERTIFICATE OF COMPLIANCE STATING WORK PERFORMED WAS IN ACCORDANCE WITH APPROVED CONSTRUCTION DOCUMENTS.

J. SUBMITTALS

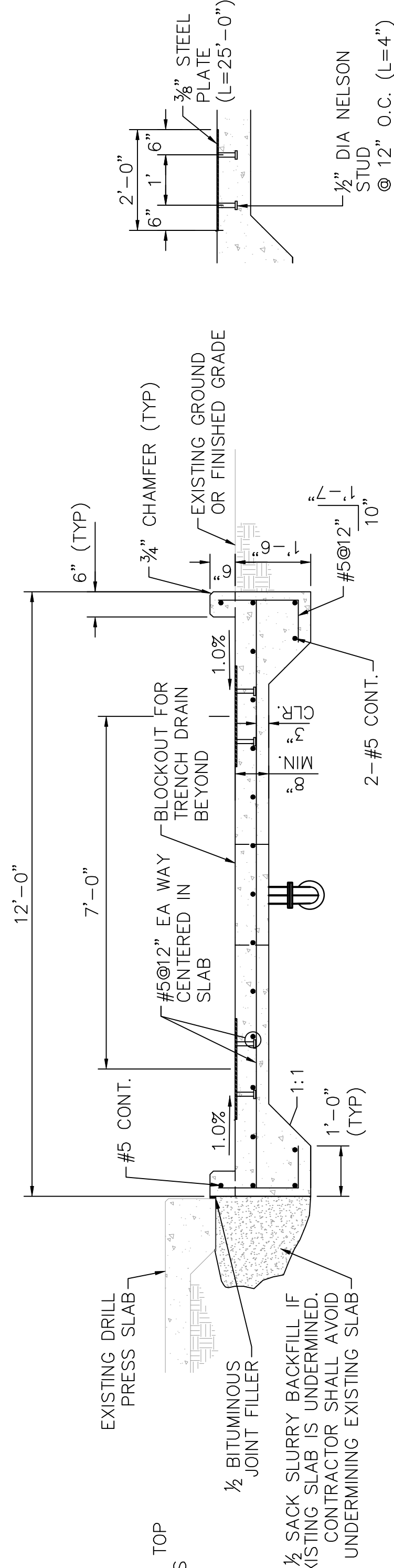
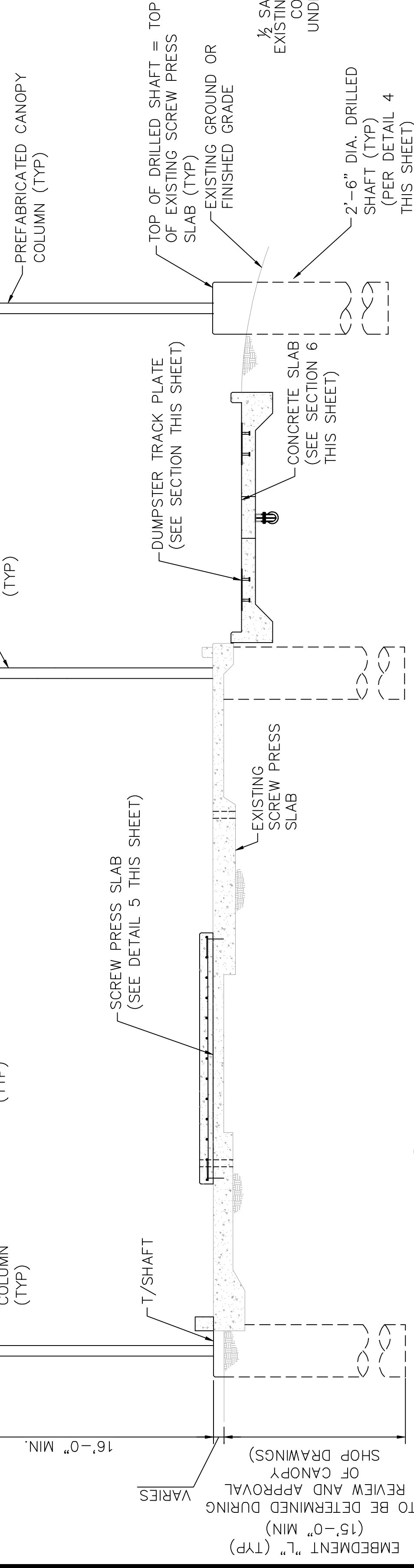
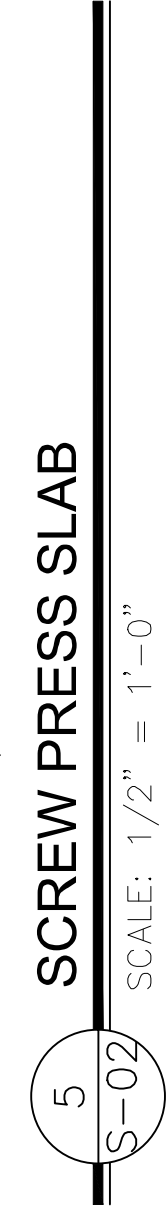
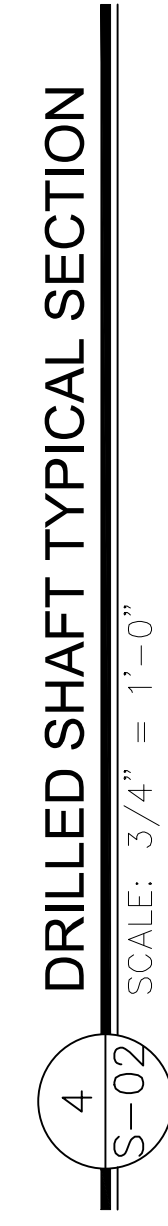
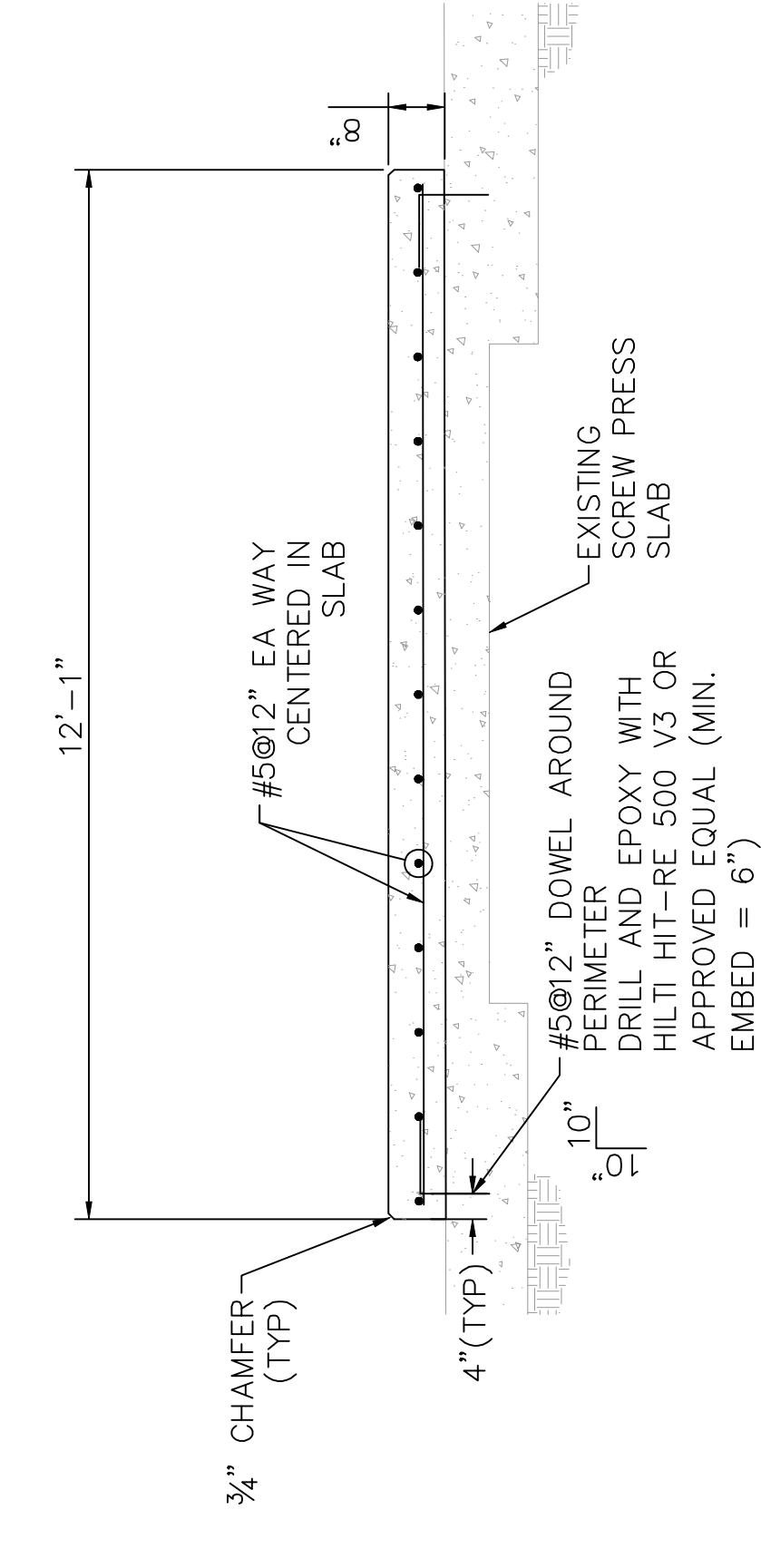
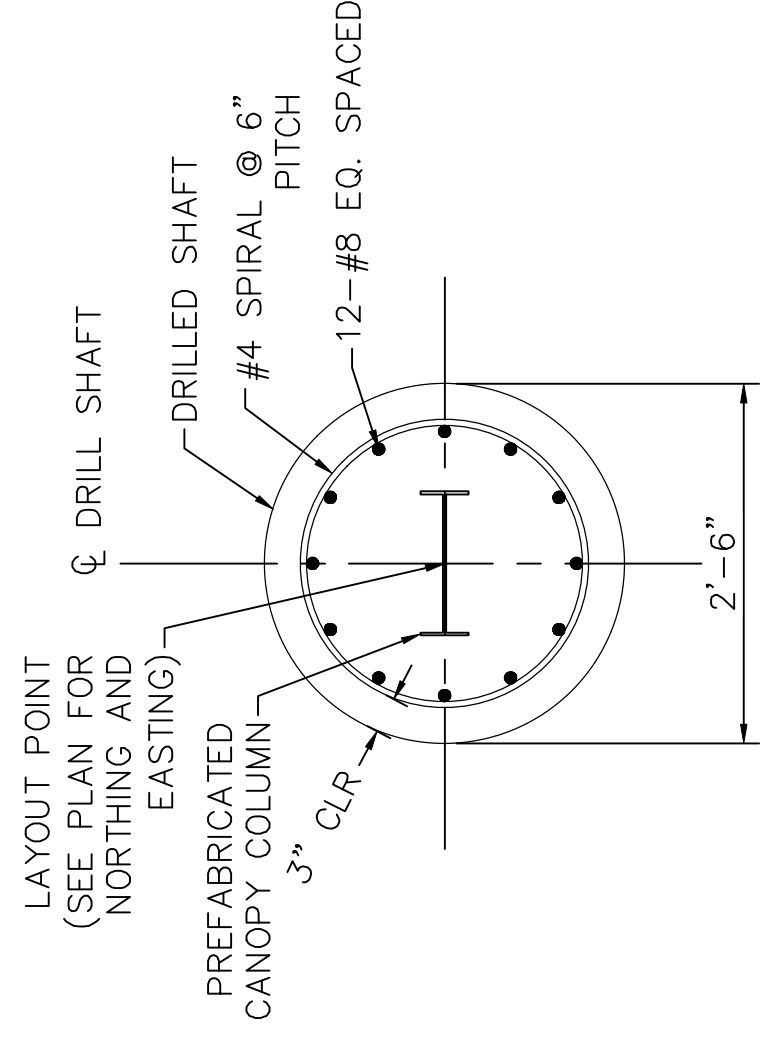
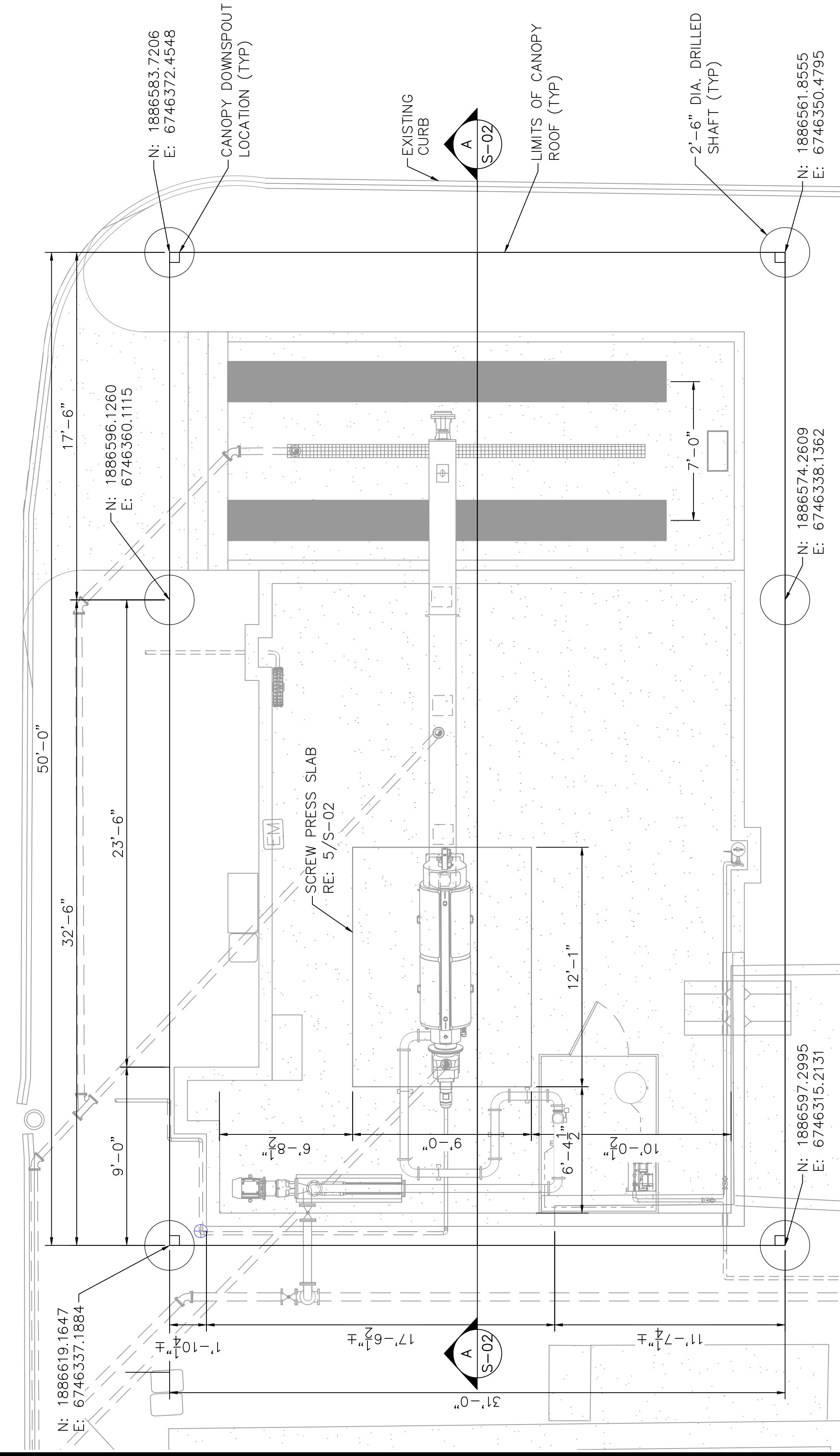
1. SCREW PRESS CANOPY STRUCTURE SHALL BE A DEFERRED SUBMITTAL. CONTRACTOR SHALL SUBMIT STRUCTURAL CALCULATIONS AND SHOP DRAWINGS FOR CANOPY TO ENGINEER FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION. SCREW PRESS CANOPY SHALL BE PROVIDED BY METAL BUILDING CORP., WHITE'S STEEL INC., ALLIED CORPORATION, OR APPROVED EQUAL. THE CALCULATIONS AND SHOP DRAWINGS MUST BE STAMPED BY A CURRENTLY LICENSED STRUCTURAL ENGINEER IN THE STATE OF CALIFORNIA. DO NOT ERECT CANOPY PRIOR TO SUBMITTAL APPROVAL BY CITY OR COUNTY INSPECTOR. MAIN STRUCTURE AND ROOF FRAMING SHALL CONSIST OF GALVANIZED STRUCTURAL STEEL. WOOD FRAMING IS NOT ACCEPTABLE.

CANOPY INFORMATION	
OCCUPANCY	F-2
CANOPY AREA	1288 SF
TYPE OF CONSTRUCTION	IIB
OCCUPANCY CONTENT	6 OCCUPANT
REQUIRED EXIT UNITS	1
FIRE SPRINKLERS	0
STANDPIPES	N/A
FIRE ALARMS	NO
EMERGENCY LIGHTING	N/A
AGE OF BUILDING	N/A

K. SCREW PRESS CANOPY FOUNDATIONS

1. SCREW PRESS CANOPY FOUNDATIONS SHALL HAVE THE MINIMUM SIZE, DEPTH, AND REINFORCING AS SHOWN ON CANOPY PLAN. SCREW PRESS CANOPY FOUNDATION SIZES TO BE FINALIZED BY ENGINEER/ OWNER REPRESENTATIVE WITH REVIEW AND APPROVAL OF SCREW PRESS CANOPY SHOP DRAWINGS AND CANOPY FOUNDATION LOADS.

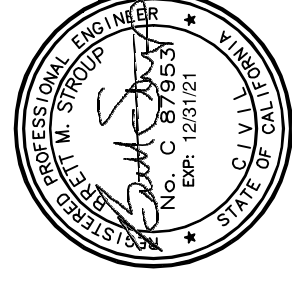
"NOT FOR BID"



Kimley»»Horn

				KPK DESIGNED
				RGR DRAWN
				KPD CHECKED
	REV.	DATE	DESCRIPTION	APP.

COUNTY OF SAN BERNARDINO
DEPARTMENT OF PUBLIC WORKS - SPECIAL DISTRICTS
222 WEST HOSPITALITY LANE, 2ND FLOOR
SAN BERNARDINO, CA 92415-0450
909-386-8800



APPROVED	2/28/2021
PROJECT ENGINEER	DATE 12/28/20
RECOMMENDED	DATE

LYTLE CREEK NORTH
CSA 70 GH SCREW PRESS
SLUDGE DEWATERING

LE CREEK NORTH A 70 GH SCREW PRESS LUDGE DEWATERING	DRAWING NUMBER	S-02
	SHEET 18 OF 26	
CANOPY PLAN	SCALE:	
	DATE:	DEC 2020

